

# **CLECO POWER LLC**

## **DOLET HILLS POWER STATION**



### **5-YEAR PERIODIC REVIEW**

### **STRUCTURAL STABILITY ASSESSMENT**

**ASH BASIN No. 2**

**OCTOBER 2021**

**Providence Engineering and Environmental Group LLC**  
1201 Main Street  
Baton Rouge, LA 70802  
(225) 766-7400  
[www.providenceeng.com](http://www.providenceeng.com)  
Providence Project No: 002-293



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## 1.0 INTRODUCTION

Providence was contracted by Cleco Power LLC (Cleco) to conduct the 5-year periodic review of the structural stability assessment of Ash Basin No. 2 at Cleco's Dolet Hills Power Station.

The Coal Combustion Residual (CCR) regulations at 40 CFR 257.73(d)(1) established requirements for owners and operators to conduct a structural stability assessment by a qualified professional engineer to document whether the design, construction, operation and maintenance is consistent with recognized and generally accepted good engineering practices. This assessment must, at a minimum, document whether the CCR unit has been designed, constructed, operated, and maintained with:

- Stable foundations and abutments.
- Adequate slope protection to protect against surface erosion, wave action, and adverse effects of sudden drawdown.
- Dikes mechanically compacted to a density sufficient to withstand the range of loading conditions in the CCR unit.
- A single spillway or a combination of spillways designed, operated, and maintained to adequately manage flow during a 100-year flood for a low hazard potential CCR surface impoundment.
- Hydraulic structures underlying the base of the CCR unit or passing through the dike of the CCR unit that maintain structural integrity and are free of significant deterioration, deformation, distortion, bedding deficiencies, sedimentation, and debris which may negatively affect the operation of the hydraulic structure.
- For CCR units with downstream slopes which can be inundated by the pool of an adjacent water body, such as a river, stream or lake, downstream slopes must maintain structural stability during low pool of the adjacent water body or sudden drawdown of the adjacent water body.

This structural stability assessment pertains to Ash Basin No. 2 that is utilized for the Unit 1 coal-fired generation unit. The Cleco Dolet Hills Power Station is located approximately 8 miles southeast of Mansfield, DeSoto Parish, LA. A site location map showing the Dolet Hills Power Station is included as **Figure 1**. A site map for Ash Basin No. 2 is included as **Figure 2**.

As part of this 5-year periodic review of the structural stability assessment of Ash Basin No. 2, Providence reviewed the construction drawings and operational plan and reviewed the inspection and maintenance procedures with Cleco for Ash Basin No. 2.

**2.0 STRUCTURAL STABILITY**

**Stable Foundations and Abutments**

In 2016, Providence modeled a short-term slope stability analysis for the pond where the facility allows the pond to fill to the freeboard level for Ash Basin No. 2. This scenario represents the flood/heavy rainfall conditions. The flood/heavy rainfall freeboard elevation was determined using 2.5 feet of freeboard from the lowest levee crown elevation for this pond.

Based on the results of the short-term slope stability analysis, the following minimum factors of safety were obtained:

**Table 1 Short-Term Factors of Safety**

Surface Impoundment	Section Number	Soil Boring No.	Water Elevation (feet NAVD 88)	Analysis	Factor of Safety
Ash Basin No. 2	Section 2	B-5 and B-6	242.5	Spencer Method Circular Failure	2.64

Providence performed a review of the calculated short-term static factor of safety under maximum surcharge pool loading conditions. The current operational status of Ash Basin No.2 has not changed since 2016. After a review of the results of the 2016 short-term static factor of safety, Providence has determined the short-term static factor of safety remains greater than 1.40, therefore the safety factor is adequate.

The normal operating water level in Ash Basin No. 2 ranges from 225 to 240.5 feet NAVD 88. These levels are significantly lower than the modeled flooded/heavy rainfall conditions.

The interior and exterior slopes of the perimeter levees are on a three horizontal to one vertical and were compacted during the construction of the levees.

**Adequate Slope Protection to Protect Against Surface Erosion, Wave Action, and Adverse Effects of Sudden Drawdown**

The levees have adequate slope protection against surface erosion, wave action, and adverse effects of a sudden drawdown. The levees have a minimum three-foot thick layer of clay on the interior, exterior, and crest of the levee. Vegetation is adequate on the top of the levee where it may be exposed to the elements. As part of Cleco’s operational plan, the levees are inspected weekly for any erosion due to weather, animals, or other elements and deficiencies are promptly corrected.

**Dikes Mechanically Compacted to a Density Sufficient to Withstand the Range of Loading Conditions in the CCR Unit**

At the time of construction, the dikes were mechanically compacted to a density sufficient to withstand the range of loading conditions for the daily operation of the unit.

**A Single Spillway or a Combination of Spillways Designed, Operated, and Maintained to Adequately Manage Flow During a 100-Year Flood for a Low Hazard Potential CCR Surface Impoundment**

Ash Basin No. 2 captures and retains rainfall runoff from drainage areas upstream of the basin dike. Ash Basin No. 2 is provided with a weir box and an auxiliary spillway. Normally, runoff from the drainage area of Ash Basin No. 2 is captured in the basin, mixed with sluice water, and drained at a slow rate via the weir box. Because the drainage area of the Ash Basin No. 2 is large, an auxiliary spillway is provided for the basin to protect against overflow of the dikes during a period of high runoff when the basin has ash at the high ash elevation level. The crest elevation of the auxiliary spillway is set so that overflow of the spillway will not occur for runoffs equal to or less than the 50-year, 24-hour runoff. The spillway is designed to discharge excess rainfall due to a 100-year, 24-hour rainfall event occurring with the basin at a maximum operating water level. The elevation of the top of the dike for the Ash Basin No. 2 was selected to provide 2 feet of interior freeboard above the maximum 100-year rainfall event water level.

The clear water that is discharged is pumped back or recirculated to the plant by the ash recirculation pumps and used again to sluice ash.

The Natural Resource Conservation Service (NRCS) Technical Release-55 (TR-55) rain distribution for a 100-year, 24-hour rain event would cause a precipitation depth of 10.5 inches. Based on the operating water levels and the discharge system in the pond, the facility would adequately manage the rainfall for a 100-year flood event.

**Hydraulic Structures Underlying the Base of the CCR Unit or Passing Through the Dike of the CCR Unit That Maintain Structural Integrity and are Free of Significant Deterioration, Deformation, Distortion, Bedding Deficiencies, Sedimentation, And Debris Which May Negatively Affect the Operation of the Hydraulic Structure**

As part of the structural evaluation, Providence was asked to determine the presence of any culverts or pipes buried in the levees of the Ash Basin No. 2. Based on the survey of the pond levees, several site inspections, review of solid waste permit files, and discussions with Cleco personnel, Providence determined that the following culverts/pipes exist within the levees surrounding the Ash Basin No. 2:

- 36” corrugated metal pipe that drains Ash Basin No. 2.
- 4” carbon steel pipe for the high pressure service water line was cut and capped at both ends and left in place in several locations along the western levee of Ash Basin No. 2.
- 42” concrete pipe located between Ash Basin No. 2 and the Secondary Pond. This pipe drains some of the storm water from the wooded area east of the Secondary Pond.
- 12” HDPE Bottom Ash Sump Line was cut and left in place in several locations along the western levee of Ash Basin No. 2 along the crest. The HDPE pipe was capped off and replaced with a steel pipe above ground.

The 42” concrete pipe located between Ash Basin No. 2 and the Secondary Pond intercepts storm water from the wooded area east of the Secondary Pond and routes it to a ditch west of the Ash Basin No. 2 which drains offsite. This pipe is completely embedded in the levee separating Ash Basin No. 2 and the Secondary Pond. This concrete pipe does not traverse under Ash Basin No. 2 or the Secondary Pond. This pipe is in a stable setting of compacted fill so movement or failure is highly unlikely, but if a failure were to occur, it poses minimal risks to Cleco based on the elevation of the pipe in the levee in relation to the elevation of the bottom of Ash Basin No. 2. Providence subcontracted to Compliance EnviroSystems (CES) to place a camera within the total length of the concrete pipe to examine the integrity of the pipe. CES placed the camera in the concrete pipe on September 22, 2021 and the pipe was in satisfactory condition for the entire length of the pipe.

There are no risks for offsite impacts associated with the other pipes mentioned above with the exception of the 42” concrete pipe and those risks are minimal.

**For CCR Units with Downstream Slopes Which Can Be Inundated by the Pool of an Adjacent Water Body, such as a River, Stream, or Lake, Downstream Slopes that Maintain Structural Stability During Low Pool of the Adjacent Water Body or Sudden Drawdown of the Adjacent Water Body**

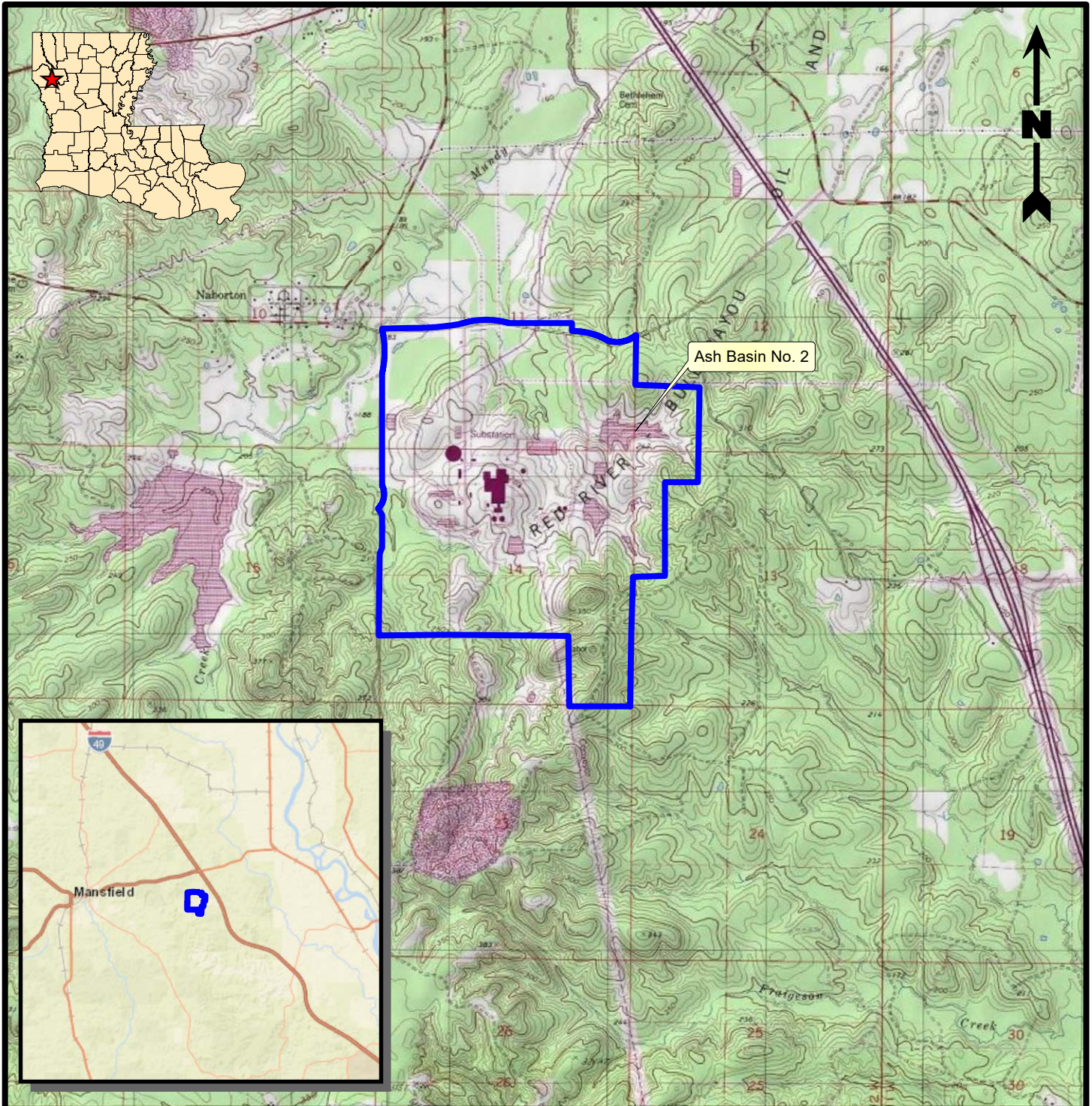
The levees do not get inundated by surface waters from adjacent features.

### **3.0 CONCLUSION**

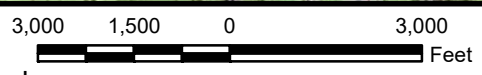
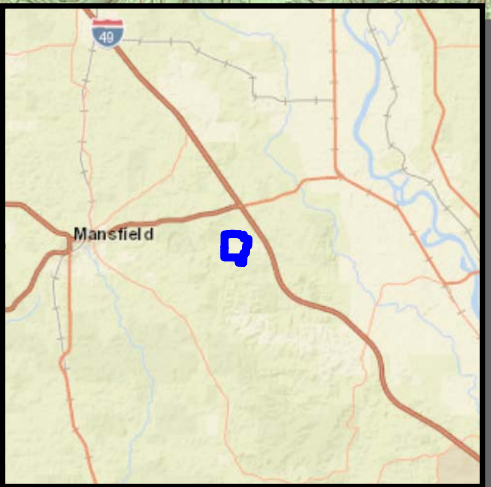
Based on the results from the structural stability assessment, Ash Basin No. 2’s design, construction, operation and maintenance is consistent with recognized and generally accepted good engineering practices. Ash Basin No. 2 meets the requirements at 257.73(d)(1) of the CCR regulations. **Appendix A** contains a P.E. Certification that attests to the 5-year periodic review of the structural stability assessment for Ash Basin No. 2.

**FIGURE 1**  
**SITE LOCATION MAP**






Ash Basin No. 2



**Legend**

 Facility Boundary

**Reference**

Base map comprised of U.S.G.S. 7.5 minute topographic maps, "Lena, LA", "Boyce, LA", "Jericho, LA", and "Gardner, LA".

**Site Location Map**

**5-Year Periodic Review  
Structural Stability Assessment - Ash Basin No. 2  
Mansfield, DeSoto Parish, Louisiana**

**Cleco Power LLC**  
Dolet Hills Power Station



Drawn By	LMH	09/21/21
Checked By	LMH	09/21/21
Approved By	CVH	09/21/21

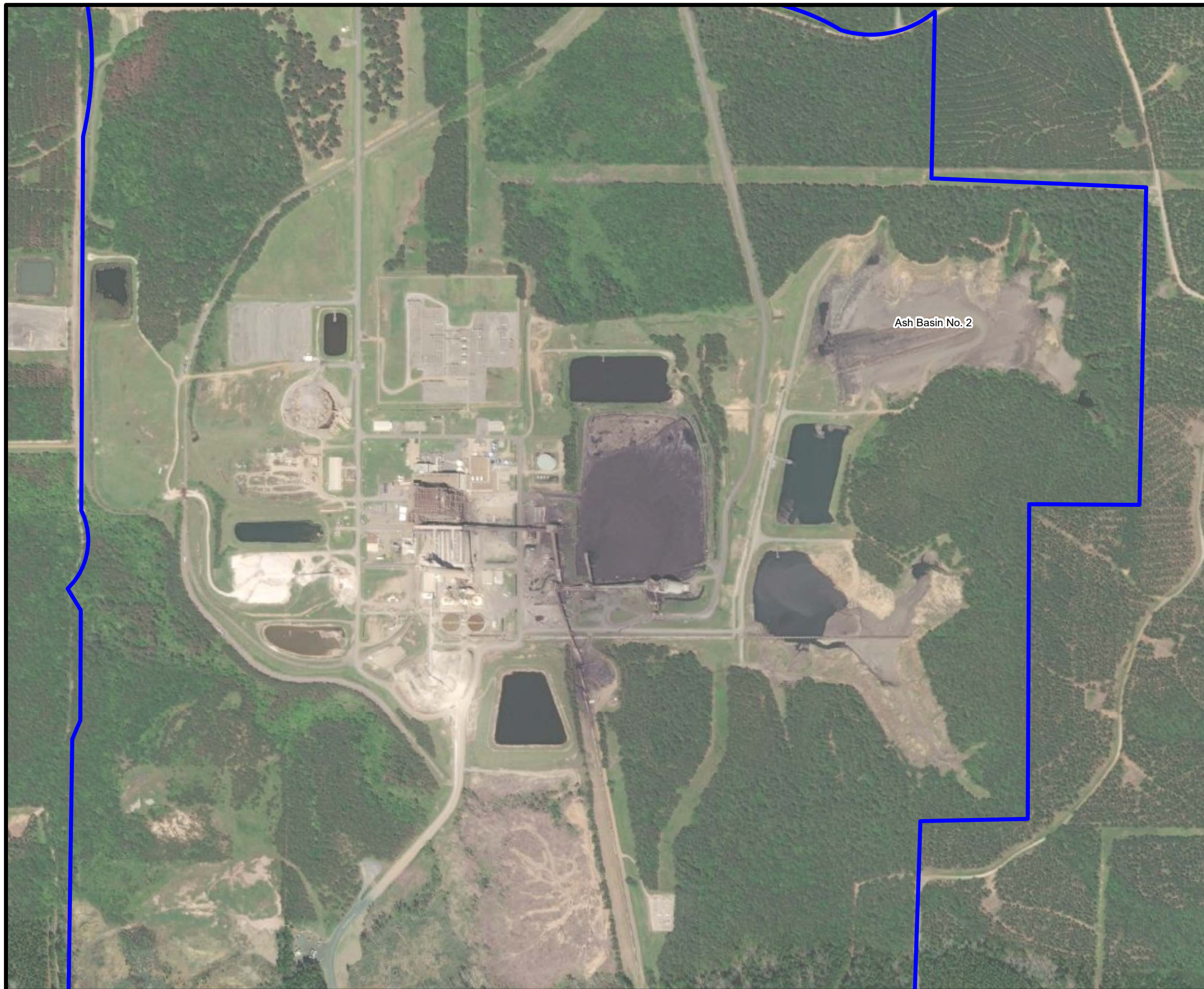
Project Number	002-293
Drawing Number	002-293-A018

**1**  
Figure




**FIGURE 2**  
**SITE MAP**



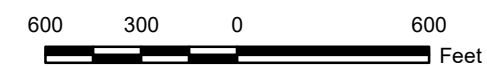


**Legend**

 Facility Boundary

**Reference**

Base map comprised of Bing Maps aerial imagery from (c) 2021 Microsoft Corporation and its data suppliers.



**Site Map**  
**5-Year Periodic Review**  
**Structural Stability Assessment - Ash Basin No. 2**  
 Mansfield, DeSoto Parish, Louisiana

**Cleco Power LLC**  
 Dolet Hills Power Station



Drawn By	LMH	09/21/21
Checked By	LMH	09/21/21
Approved By	CVH	09/21/21
Project Number	002-293	
Drawing Number	002-293-B019	<b>2</b> Figure



**APPENDIX A**  
**P.E. CERTIFICATION**

**CLECO DOLET HILLS POWER STATION  
ASH BASIN NO. 2  
5-YEAR PERIODIC REVIEW - CCR STRUCTURAL STABILITY ASSESSMENT**

**PROFESSIONAL ENGINEER CERTIFICATION**

I hereby certify that I have performed the 5-year periodic review of the structural stability assessment for Cleco's Dolet Hills Power Station Ash Basin No. 2 in accordance with the 40 CFR 257.73(d)(1) CCR requirements. This 5-year periodic review of the structural stability assessment has determined that Ash Basin No. 2's design, construction, operation and maintenance is consistent with recognized and generally accepted good engineering practices. It has been designed, constructed, operated, and maintained with:

- Stable foundations and abutments.
- Adequate slope protection to protect against surface erosion, wave action, and adverse effects of sudden drawdown.
- Dikes mechanically compacted to a density sufficient to withstand the range of loading conditions in the CCR unit.
- A single spillway or a combination of spillways designed, operated, and maintained to adequately manage rainfall during a 100-year flood for a low hazard potential CCR surface impoundment.
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- For CCR units with downstream slopes which can be inundated by the pool of an adjacent water body, such as a river, stream or lake, downstream slopes must maintain structural stability during low pool of the adjacent water body or sudden drawdown of the adjacent water body.

**James C. Van Hoof**

Name

**24630**

**LA**

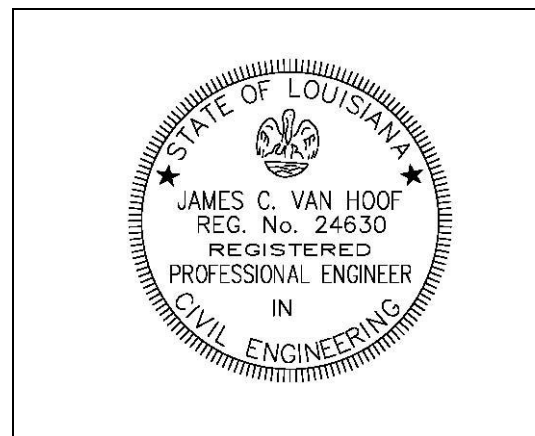
Registration No.

State

Signature

10/14/2021

Date



(Seal)