Periodic Update to the Run-on and Run-off Control System Plan Dolet Hills Fly Ash / Scrubber Sludge Landfill

CLECO Corporation

Dolet Hills Power Station
Project No. 202248

Revision 2
10/13/2026

Periodic Update to the Run-on and Run-off Control System Plan Dolet Hills Fly Ash / Scrubber Sludge Landfill

prepared for

CLECO Corporation
Dolet Hills Power Station
DeSoto Parish, Louisiana

Project No. 202248

Revision 2
10/13/2026

prepared by

Burns & McDonnell Engineering Company, Inc.
Kansas City, Missouri

INDEX AND CERTIFICATION

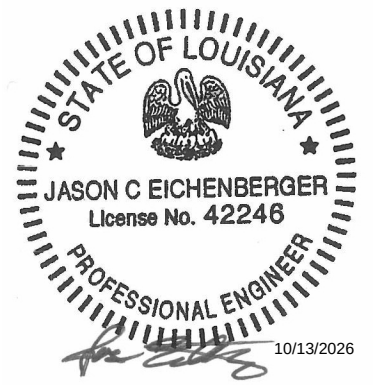
CLECO Corporation
Periodic Update to the Run-on and Run-off Control System Plan
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Certification

I hereby certify, as a Professional Engineer in the state of Louisiana, that the information in this document was assembled under my direct supervisory control. This report is not intended or represented to be suitable for reuse by the CLECO Corporation or others without specific verification or adaptation by the Engineer.



Jason C. Eichenberger, P.E.
Louisiana License #42246

Date: 10/13/2026

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LIST OF ABBREVIATIONS

Abbreviation	Term/Phrase/Name
ac	Acre
CCR	Coal Combustion Residual
CFR	Code of Federal Regulations
cfs	cubic feet per second
CLECO	CLECO Corporation
Dolet Hills	Dolet Hills Power Station
EPA	Environmental Protection Agency
ft	Feet
in	Inch
Landfill	Fly Ash Scrubber Sludge Landfill
LPDES	Louisiana Pollutant Discharge Elimination System
LDEQ	Louisiana Department of Environmental Quality
min	Minute
NAD 83	North American Datum of 1983
NAVD 88	North American Vertical Datum of 1988
PFDS	Precipitation Frequency Data Server
RCRA	Resource Conservations and Recovery Act
SCS	Soil Conservation Service
U.S.C.	United States Code

1.0 INTRODUCTION

On April 17, 2015, the Environmental Protection Agency (EPA) issued the final version of the federal Coal Combustion Residual Rule (CCR Rule) to regulate the disposal of CCR materials generated at coal-fired units. The rule will be administered as part of the Resource Conservation and Recovery Act [RCRA, 42 United States Code (U.S.C.) §6901 et seq.], using the Subtitle D approach.

CLECO Corporation (CLECO) is subject to the CCR Rule and as such must develop a run-on and run-off control system plan for each CCR landfill per 40 Code of Federal Regulations (CFR) §257.81 and prepare periodic plans every five years. This report serves as the second periodic update to the initial run-on and run-off control system plan for the Fly Ash / Scrubber Sludge Landfill (the Landfill) at Dolet Hills Power Station (Dolet Hills).

This run-on and run-off control system plan is in addition to, not in place of, any other applicable site permits, environmental standards, or work safety practices.

2.0 PLAN OBJECTIVES

Per 40 CFR §257.81, the run-on and run-off control system plan must contain documentation (including supporting engineering calculations) that the control system has been designed and constructed to:

- Prevent flow onto the active portion of the CCR unit during peak discharge from a 25-year, 24-hour storm,
- Collect and control at least the water volume resulting from a 25-year, 24-hour storm, and
- Handle run-off from the active portion of the CCR landfill in accordance with the surface water requirements under 40 CFR §257.3-3.

Per 40 CFR §257.81(c)(5), CLECO must obtain certification from a qualified professional engineer that the run-on and run-off control system plan, and subsequent updates to the plan, meet the requirements of 40 CFR §257.81. This sealed document serves as that certification.

3.0 EXISTING CONDITIONS

Dolet Hills is located east of Mansfield in DeSoto Parish, Louisiana. Dolet Hills contains one CCR landfill which received fly ash and scrubber sludge and has now ceased receipt of waste and is undergoing final closure. A site plan is shown in Figure SK-CIVIL-001 in Appendix A.

Run-on and run-off controls were designed by others as part of the Landfill permit application to the Louisiana Department of Environmental Quality (LDEQ). The applicable sections of the permit renewal application prepared by Providence Engineering and Environmental Group LLC. in 2019 are included as Appendix B, along with design drawings for the final closure of the landfill. The run-on and run-off control system plan provided herein is based on review and assessment of the certified permit information, as well as supplementary design documents and operational characteristics provided by CLECO.

3.1 Existing Design Document Review

The CCR Rule requires that peak stormwater flows from the 25-year, 24-hour storm event be handled by run-on and run-off control measures. The EPA defines run-on as any rainwater, leachate, or other liquid that drains over land onto any part of a CCR landfill or lateral expansion of a CCR landfill. The EPA defines run-off as any rainwater, leachate, or other liquid that drains over land from any part of a CCR landfill or lateral expansion of a CCR landfill. The information included in Appendix B indicates compliance with the CCR Rule run-on and run-off control system plan requirements.

3.1.1 Run-on

The Landfill was sited so that it sits in a stream valley surrounded by ridgelines on three sides which prevent run-off from surrounding watersheds from entering the Landfill area. Placement of fill in the Landfill was sequenced from the southern, closed end of the stream valley and proceeded to the north. As the Landfill was developed, perimeter drainage diversion dikes were configured to prevent outside stormwater run-off (non-contact stormwater run-off) from entering the Landfill cell. Perimeter drainage ditches collect this non-contact stormwater run-off and drain it offsite, thus preventing run-on. The collection ditches have a triangular section with an average bottom slope of 3%, minimum 2-foot depth, and 3H:1V side slopes (see Figure 3-1).

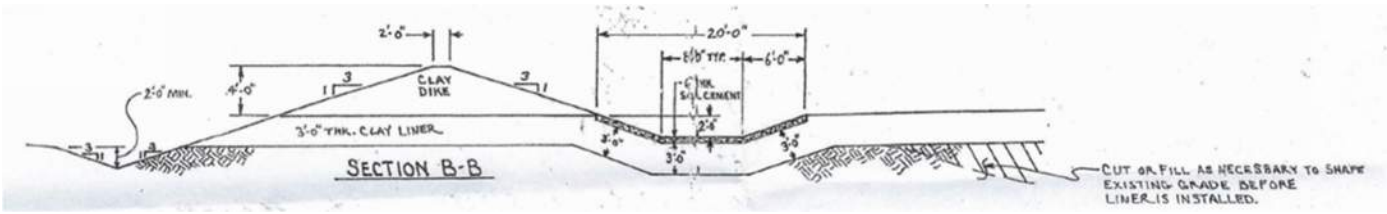


Figure 3-1: Typical run-off collection ditches and diversion dike

Additional diversion dikes were installed to prevent run-off from the closed portion of the Landfill from entering the active area. Run-off from the closed portion of the Landfill is typically drained offsite to existing drainage channels.

3.1.2 Run-off

As the Landfill expanded to the north, levees were reconstructed along the interface of the closed and open portions of the Landfill as well as around the perimeter of the open portion of the Landfill to isolate contact stormwater run-off from non-contact stormwater run-off.

As noted above, run-off from the closed portion of the landfill is collected in perimeter drainage ditches and discharged off-site via drainage channels. A series of perimeter drainage ditches within the levee diverted contact stormwater run-off to a temporary sump. The perimeter ditches had an approximately trapezoidal section with an 8-foot bottom width, 1% bottom slope, 2-foot minimum depth, and 3H:1V side slopes (see Figure 3-1). At the time of this periodic update, the landfill ceased receiving waste and is undergoing final closure. Closed portions will be directed offsite and will not contribute to the sump area. The perimeter ditches conveying non-contact runoff have a trapezoidal section with a 5-ft bottom width, average 5% bottom slope, and 3H:1V side slopes.

The landfill sump area was reconstructed each time the landfill expanded to the north. The Cell 11 sump was reconfigured into a settling basin as part of the Landfill closure design. It will receive leachate from the Landfill, but no runoff will be directed towards it besides what falls over the basin footprint. The settling basin overflows to the Runoff Pond via the existing 48-inch collector pipe. Refer to Appendix B for existing design drawings. For the purposes of this analysis, it was assumed that the settling basin has an initial elevation equal to the invert of the outlet pipe (268.4 feet).

The Runoff Pond is approximately 4-acres and sits north of the Landfill. The pond is partially surrounded by a run-on diversion dike to limit run-off inflow from the surrounding watershed area (see DWG No. DH-123 in Appendix B). The pond was designed to store contact run-off from the 25-

year, 24-hour storm and contains an emergency spillway for larger storm events. The concrete spillway was added to the pond in 1998 when the dike was raised and the reinforced concrete wall was installed. The top of dam is now approximately 270 feet.

From the Runoff Pond, water may be discharged via a valve-controlled outlet pipe. The valve is normally closed and has an invert elevation of 253 feet. For the purposes of this modeling effort, Burns & McDonnell has assumed that the plant operations team maintains the normal water level (prior to rain events) at or below elevation 259.0. Water stored in the pond may be discharged under the conditions of the discharge permit (LA0062600); consequently, all discharges are permitted and conform to CFR 257.3-3.

4.0 DESIGN BASIS / FLOOD CONTROL SYSTEM

4.1 Capacity Criteria

The CCR Rule discusses that stormwater control systems at CCR landfills must have adequate hydrologic and hydraulic capacity to manage flows for the 25-year, 24-hour storm event. For this analysis, the criteria was interpreted as being that the top of the stormwater conveyance and storage features should not be overtopped during the design storm event.

4.2 Project Mapping

Project mapping for this analysis consisted of an inventory of stormwater assets that contribute to the surface impoundment. The following sources were utilized: construction record drawings, landfill closure design drawings, and publicly available LiDAR survey data.

4.2.1 Mapping Sources

The analysis utilized the closure design drawings, which included topographic survey collected by Morrison Surveying (January 2021) and Aucoin & Associates (March 2025). This was supplemented with LiDAR data from USGS which was posted in 2018 and retrieved in September of 2026. Construction record drawings of the Landfill, sump area, and surface impoundment were also utilized in the analysis.

4.2.2 Vertical Datum

Mapping sources referenced were in the North American Vertical Datum of 1988 (NAVD 88).

4.2.3 Horizontal Coordinate System

Mapping sources were referenced to the Louisiana State Plane North Zone, North American Datum of 1983 (NAD 83) coordinate system.

5.0 HYDROLOGIC AND HYDRAULIC CAPACITY

HydroCAD 10.20-7a was used to model the drainage areas and ponds using the design storm event and FlowMaster V8i was used to model channel characteristics. Inputs to the HydroCAD and FlowMaster models are described in the following sections. For detailed calculations, refer to Appendix C.

5.1 Rainfall Distribution and Depth

The Soil Conservation Service (SCS) Type III rainfall distribution was used for computations associated with this evaluation. Precipitation data was acquired from the NOAA Precipitation Frequency Data Server (PFDS). Precipitation depth for design storm event is 8.16 inches.

5.2 Subbasin Characteristics

5.2.1 Landfill

Stormwater falling over the Landfill (see SK-CIVIL-001 in Appendix A for approximate limits) is collected in the perimeter ditches which drain offsite. Stormwater falling over the sediment basin is directed to the Runoff Pond via the 48-inch collector pipe. The landfill watershed consists mainly of steeply sloped (4H:1V), artificial turf. Calculations for this watershed area were determined based on the parameters shown in Table 5-1.

Table 5-1: Watershed Run-off Calculated Data for Landfill Run-off

Watershed (Subcatchment)	Component	Value	Unit
Northwest Landfill Area (closed)	Area	27.4	ac
	Weighted Curve Number	93	-
	Time of Concentration	6.8	min
Northeast Landfill Area (closed)	Area	28.3	ac
	Weighted Curve Number	91	-
	Time of Concentration	11.8	min
Settling Basin Area	Area	1.1	ac
	Weighted Curve Number	92	-
	Time of Concentration	6	min

5.2.2 Runoff Pond

Drainage into the Runoff Pond (see SK-CIVIL-001 in Appendix A for approximate watershed limits) is minimized, in part, by dikes surrounding the pond. This watershed consists mainly of moderately

sloped (8H:1V), undeveloped, forested area with grassy, open areas near the pond limits. Calculations for the Runoff Pond watershed area were determined based on the parameters shown in Table 5-2.

Table 5-2: Watershed Run-off Calculated Data for Runoff Pond

Component	Value	Unit
Watershed Area	7.3	ac
Weighted Curve Number	86	-
Time of Concentration	147.3	min

5.2.3 Outside Run-off

Stormwater falling over the outside run-off area (see SK-CIVIL-001 in Appendix A for approximate watershed limits) is collected in run-off collection ditches which drain offsite. This watershed consists mainly of moderately sloped (8H:1V), undeveloped, forested area with grassy, open areas near the Landfill limits. Calculations for this watershed were determined based on the parameters shown in Table 5-3.

Table 5-3: Watershed Run-off Calculated Data for Outside Run-off

Component	Value	Unit
Watershed Area	5.6	ac
Weighted Curve Number	79	-
Time of Concentration	7.3	min

5.3 Channel Characteristics

Flow calculations were determined based on the channel parameters shown in Table 5-4 (landfill stormwater run-off) and Table 5-5 (offsite stormwater run-off).

Table 5-4: Channel Data for Landfill Stormwater Run-off, Flat Bottom Ditch

Component	Value	Unit
Roughness Coefficient, Manning's n	0.020 (HydroBinder)	-
Channel Slope	0.05	ft/ft
Left Side Slope	3H:1V	-
Right Side Slope	3H:1V	-
Bottom Width	5.00	ft

Table 5-5: Channel Data for Offsite Stormwater Run-off, Triangular Ditch

Component	Value	Unit
Roughness Coefficient, Manning's n	0.025 (earth, clean & winding)	-
Channel Slope	0.0286	ft/ft
Left Side Slope	3H:1V	-
Right Side Slope	3H:1V	-

6.0 RESULTS

The Landfill areas were modeled under the 25-year, 24-hour storm event with results as follows:

Table 6-1: Modeled Conditions – Landfill

Component	Property	Value	Unit
Subcatchment Northwest Landfill Area (closed)	Runoff Depth	7.31	in
	Peak Runoff	208.48	cfs
Subcatchment Northeast Landfill Area (closed)	Runoff Depth	7.07	in
	Peak Runoff	180.77	cfs
Subcatchment Settling Basin	Runoff Depth	7.20	in
	Peak Runoff	8.56	cfs
Pond Settling Basin	Initial Elevation	268.4	ft
	Peak Inflow	8.56	cfs
	Peak Discharge	5.41	cfs
	Peak Elevation	269.2	ft
	Freeboard	12.8	ft
Subcatchment Runoff Pond	Runoff Depth	6.31	in
	Peak Runoff	11.94	cfs
Pond Runoff Pond	Initial Elevation	259	ft
	Peak Inflow	12.55	cfs
	Peak Discharge	0.00	cfs
	Peak Elevation	261.25	ft
	Freeboard	8.75	ft

Table 6-2: Modeled Conditions – Outside Run-off

Component	Property	Value	Unit
Subcatchment Watershed	Runoff Depth	5.65	in
	Peak Runoff	35.02	cfs

Table 6-3: Modeled Channel Conditions

Property	Peak Discharge (cfs)	Normal Depth (in)	Freeboard (in)
Non-Contact Stormwater Run-off Landfill Runoff	208.48	17.0	7.0
Non-Contact Stormwater Run-off Outside Runoff	35.02	16.4	7.6

Under the modeled conditions the Settling Basin, Runoff Pond, and the stormwater runoff channels do not overtop; therefore, it can be assumed that they are adequately sized to control run-off the 25-year, 24-hour storm event. After a significant storm event, excess water collected in the Runoff Pond can be discharged under the conditions of the LPDES permit.

7.0 STORMWATER BEST MANAGEMENT PRACTICES

Stormwater best management practices (BMPs) shall be employed at the site to comply with CFR 257.3-3 which, in summary, stipulates that a facility shall not cause a discharge of pollutants, dredged material, or fill material to waters of the United States or cause non-point source pollution of waters of the United States.

Vegetation enhances evapotranspiration and reduces erosion, thus playing an important part in surface water control. Channels not requiring riprap or artificial turf were prepared for seeding as they were constructed. Similarly, final cover areas were prepared for seeding or for artificial turf after placement. The use of terrace and downdrain channels for stormwater conveyance provides a means to control run-off velocities and reduce sediment transport.

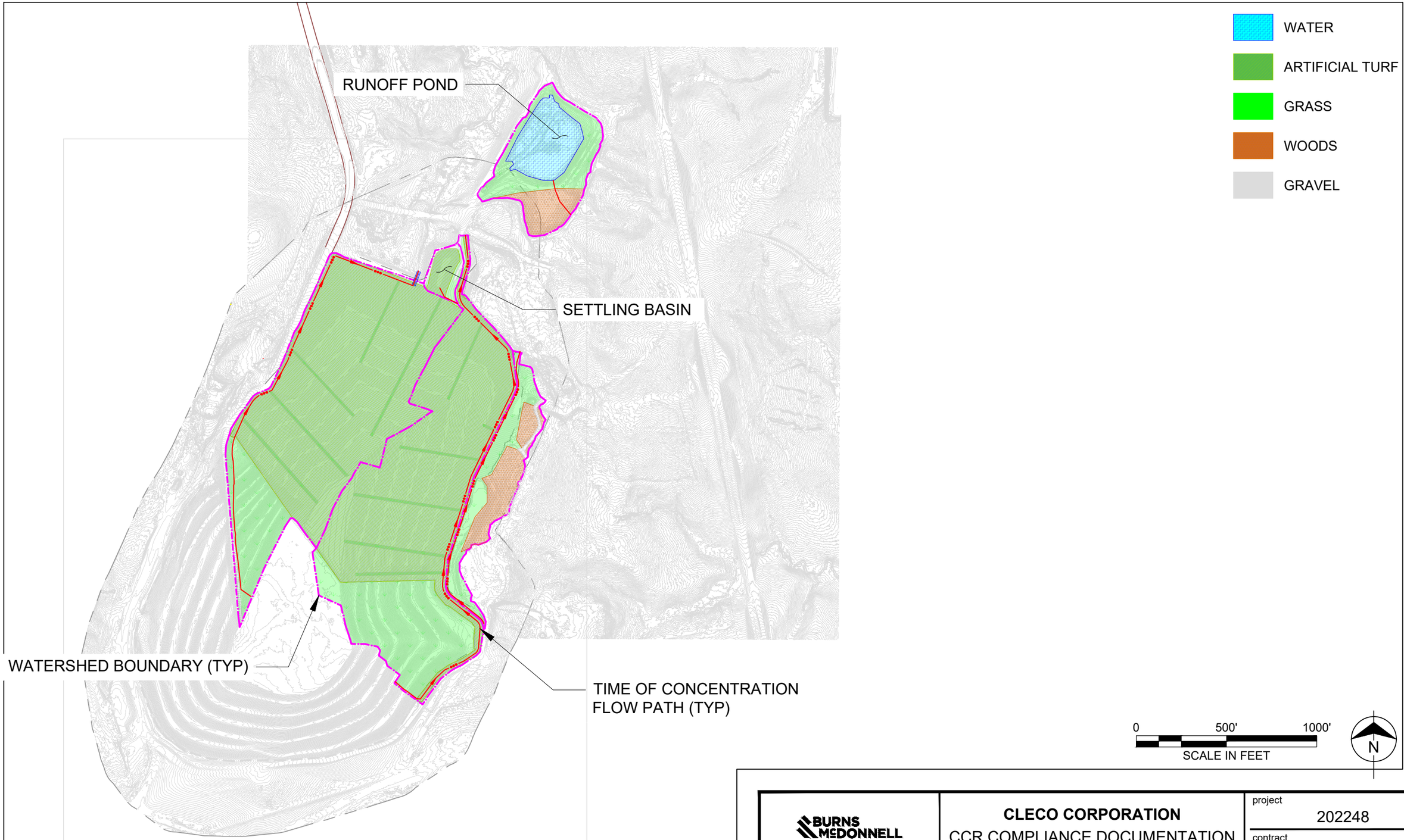
8.0 PERIODIC ASSESSMENT AND AMMENDMENT

CLECO placed the initial run-on and run-off control system plan in the CCR Operating Record by October 17, 2016. CLECO must prepare periodic run-on and run-off control system plans every five years. This report serves as the second periodic update to the initial run-on and run-off control system plan. CLECO may amend the plan at any time and is required to do so whenever there is a change in conditions which would substantially affect the written plan in effect. Preparing the periodic plans may be achieved by reviewing the current plan in effect and amending the plan as required. In all cases, the date for completing the previous plan is the basis for establishing the deadline to complete the subsequent periodic plan. Each periodic plan shall be certified by a qualified professional engineer in the state of Louisiana. All amendments and revisions must be placed on the CCR public website within a reasonable amount of time following placement in the facility's CCR Operating Record. A record of revisions made to this document is included in Section 9.0.

9.0 REVISIONS AND UPDATES

Revision Number	Date	Revisions Made	By Whom
0	10/14/2016	Initial Issue	Burns & McDonnell
1	10/13/2021	Periodic Update to the Initial Plan	Burns & McDonnell
2	10/13/2026	Periodic Update to the Plan	Burns & McDonnell

APPENDIX A – SITE PLAN



BURNS McDONNELL

date 9/2/2026

designed A. MYERS

CLECO CORPORATION

CCR COMPLIANCE DOCUMENTATION
RUN-ON AND RUN-OFF CONTROL
SITE PLAN

project 202248

contract -

SK - CIVIL-001

APPENDIX B – EXISTING PERMIT INFORMATION / DRAWINGS

ATTACHMENT 22

DESCRIPTION OF FACILITY RUNOFF COLLECTION SYSTEM

ATTACHMENT 22

DESCRIPTION OF FACILITY RUNOFF COLLECTION SYSTEM

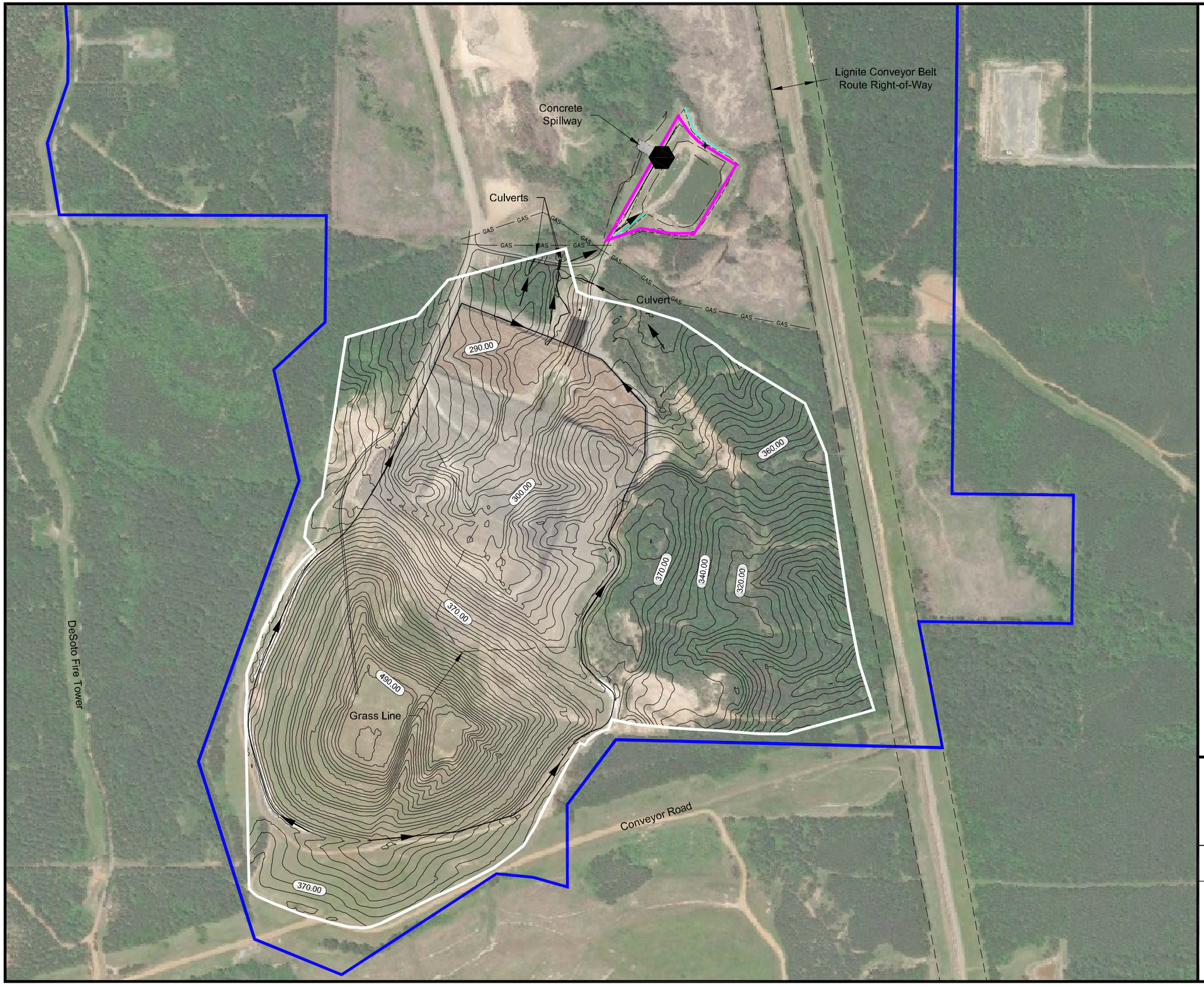
The purpose of the impoundment is to store contact storm water runoff from the active cell(s) of the landfill resulting from the 25-year, 24-hour design storm. The stored runoff will be discharged under conditions of the power station's water discharge permits. **Attachment 22** shows the site runoff collection system. This system consists of levees that will be constructed along the northern edge of the covered cells and down the valley slopes on either side of the active cell. The levee alignment along the valley slope will be such that it will encompass all of the waste placed in the active cells. This levee will segregate non-contact storm water runoff from contact storm water runoff. All rain falling within the levee will flow by gravity to a temporary collection sump located at the lowest end of the active cell area. A dike will be constructed across the valley to prevent any runoff from leaving the site. The collected runoff will then be transferred to the surface impoundment where it will be discharged under the conditions of the power station's water discharge permits. All rainfall that contacts the waste will be collected and transported to the surface impoundment.

Since the waste will have no free liquids nor generate any leachate, the only water that must be removed is rainfall. The system of culverts, levees, swales, sumps, and pumps described previously will be used to dewater the site when necessary.

All rain falling within areas of the landfill that have received certified final cover will be routed to other storm water outfalls.

Waste materials disposed in the expansion area will have no free liquids or generate any leachate. The only water required to be removed from the expansion area is rainfall. A perimeter levee system will be constructed around the expansion area to prevent runon from entering the operational areas of the facility expansion. This perimeter levee will also prevent runoff from leaving the operational areas of the facility expansion. As each disposal cell is constructed in the expansion area, a cell separation berm will be constructed to contain and capture all contact storm water runoff from each active cell. This collected storm water will be pumped from within each active cell and transported via gravity main or force main to the existing surface impoundment.

As final cover is installed in the expansion area, all non-contact storm water runoff from the certified closed areas will be routed to other storm water outfalls.



Legend

- Property Boundary
- Permitted Landfill Limit
- Fly Ash/FGD Landfill Runoff Pond
- Drainage Ditch
- Right-of-Way
- GAS — Gas Pipeline
- 5' Lidar Contours (10/17/19)
- Surface Flow Direction
- Outfall Location

Reference

Base map comprised of Bing aerial imagery 06/18/19 and client provided file name: 13-51 topo.dwg.

500 0 500

Scale: 1" = 500'

Facility Runoff Collection System

Solid Waste Permit Renewal
Mansfield, DeSoto Parish, Louisiana

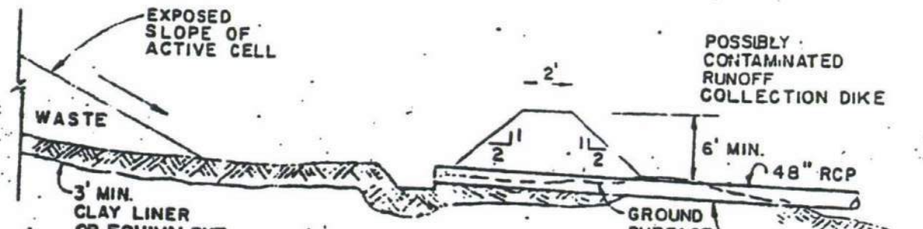
Cleco Power LLC
Dolet Hills Power Station

PROVIDENCE

Drawn By	DSG	10/22/19
Checked By	LMH	10/22/19
Approved By	EKS	10/22/19
Project Number	002-255	22 Attachment
Drawing Number	002-255-B018	

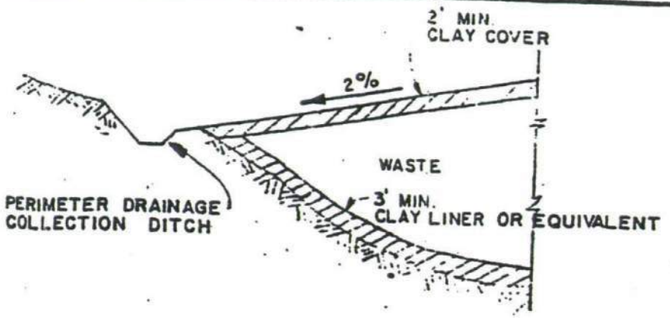
Providence Engineering and Environmental Group LLC

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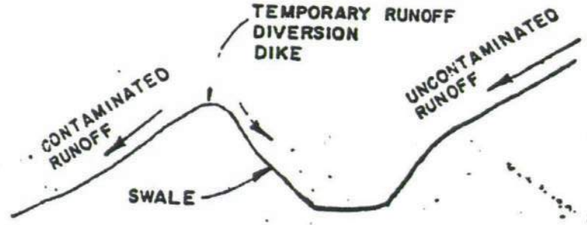


RUNOFF COLLECTION SUMP

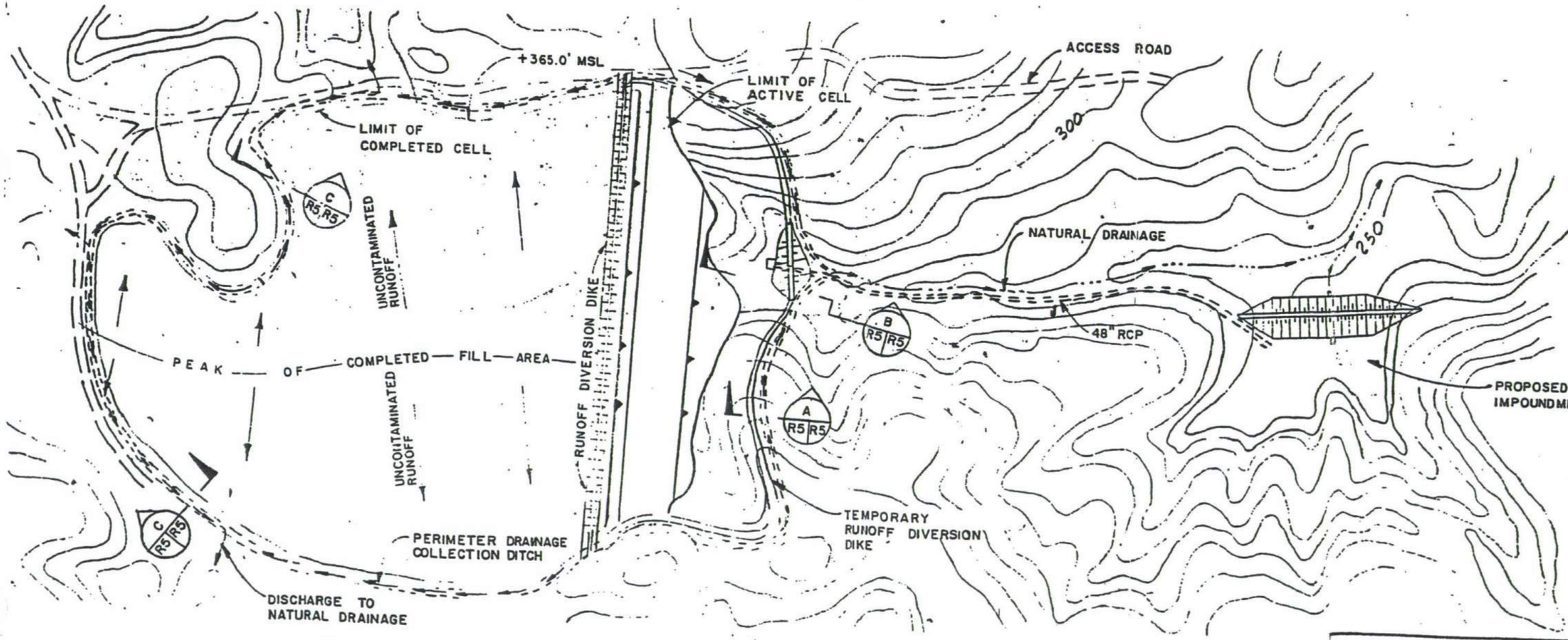
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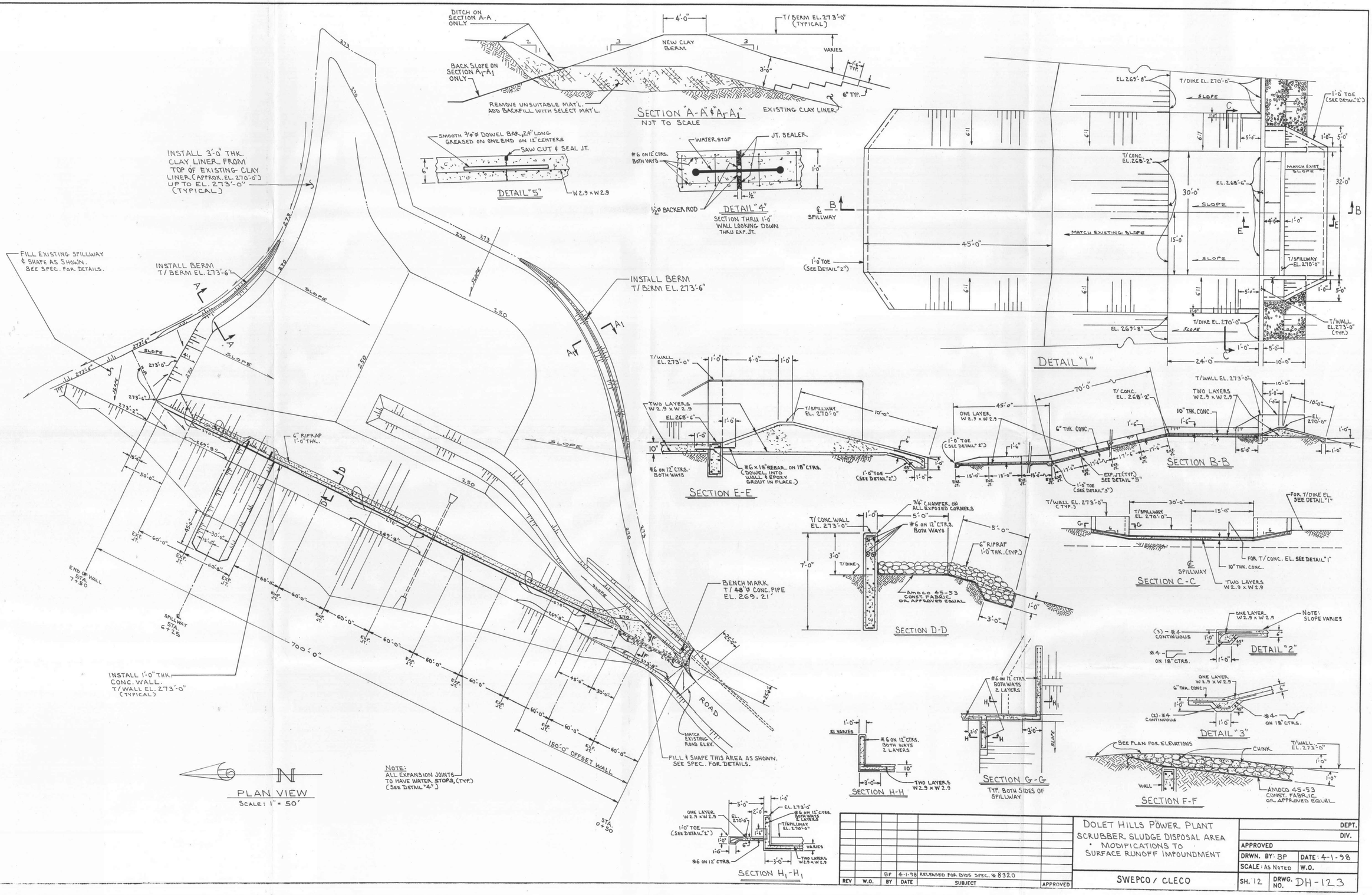
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SCALE: NONE



PLAN
SCALE: 1"= 400'

WALDEMAR S. NELSON AND COMPANY
INCORPORATED

SWEPCO/CLECO
DOLET HILLS POWER PLANT
SOLID WASTE PERMIT APPLICATION
SURFACE RUNOFF DIVERSION



REV	W.O.	BY	DATE	SUBJECT	APPROVED
		BP	4-1-98	RELEASED FOR BIDS SPEC. # 8320	

DOLET HILLS POWER PLANT
SCRUBBER SLUDGE DISPOSAL AREA
• MODIFICATIONS TO •
SURFACE RUNOFF IMPOUNDMENT

SWEPCO / CLECO

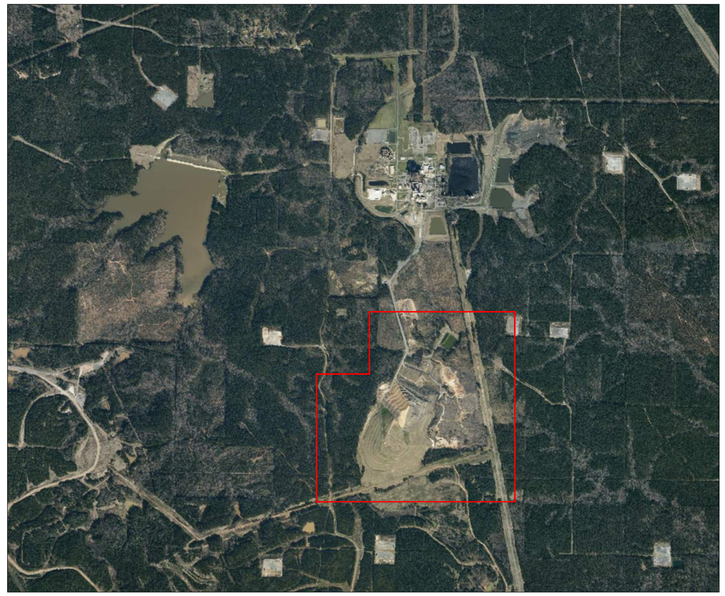
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APPROVED	
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SCALE: AS NOTED	W.O.
SH. 12	DRWG. NO. DH-123

LANDFILL CLOSURE PLANS
DOLET HILLS POWER STATION

963 POWER PLANT ROAD, MANSFIELD, LOUISIANA 71052

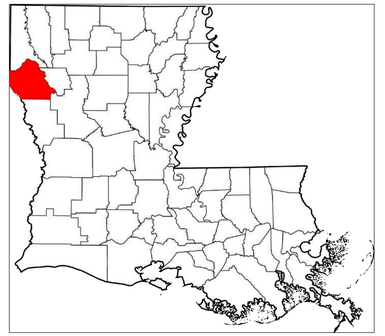
DESOTO PARISH, LOUISIANA

LATITUDE: 32°01'55.59" N, LONGITUDE: 93°34'09.86" W



SITE OVERVIEW MAP - 1" = 2000'

DWG NO	DRAWING DESCRIPTION
001	TITLE SHEET
002	GENERAL NOTES
003A	EXISTING SITE AERIAL - LANDFILL
003B	EXISTING SITE AERIAL - LANDFILL SURFACE IMPOUNDMENT
004A	DEMOLITION PLAN - LANDFILL
004B	DEMOLITION PLAN - LANDFILL SURFACE IMPOUNDMENT
005	FINAL GRADE - TOP OF REGRADED WASTE
006	FINAL GRADE - FINAL COVER SYSTEM TERMINATION PLAN
007	FINAL GRADE - SETTLING BASIN
008	PROFILES I
009	PROFILES II
010	PROFILES III
011	PROFILE C - CROSS SECTIONS I
012	PROFILE C - CROSS SECTIONS II
013	PROFILE C - CROSS SECTIONS III
014	PROFILE C - CROSS SECTIONS IV
015	DETAILS I
016	DETAILS II
017	DETAILS III
018	EROSION AND SEDIMENT CONTROL PLAN
019	EROSION AND SEDIMENT CONTROL DETAILS
020	STORMWATER MANAGEMENT PLAN
021	TOE DRAIN (BULLETIN NO. 1)



LOUISIANA STATE MAP - NTS

PREPARED FOR:
CLECO CORPORATE HOLDINGS,LLC (OWNER)
2030 DONAHUE FERRY ROAD
P.O. BOX 5000
PINEVILLE, LA 71361

PREPARED BY:
AECOM TECHNICAL SERVICES
7389 FLORIDA BOULEVARD
BATON ROUGE, LA 70806

REV	DATE	JOB NO.	PROJECT TYPE	DES	DFTR	CHKD	ENG	APPD	DESCRIPTION
0	7-14-2023	60639309	LANDFILL CLOSURE	AMC	AMC	AGH	AGH	AGH	ISSUED FOR CONSTRUCTION
1	7-25-2025	60639309	LANDFILL CLOSURE	JET	CAM	AGH	AGH	AGH	ISSUED FOR BID
2	8-25-2025	60639309	LANDFILL CLOSURE	JET	CAM	AGH	AGH	AGH	ISSUED FOR BID ADDENDUM FOR REFERENCE
3	11-18-2025	60753972	LANDFILL CLOSURE	JET	CAM	AGH	AGH	AGH	ISSUED FOR CONSTRUCTION
B1	2-20-2026	60753972	LANDFILL CLOSURE	AGH	AGH	VKG	AGH	AGH	BULLETIN 1 - TOE DRAIN
4	3-27-2026	60753972	LANDFILL CLOSURE	JET	JET	AGH	AGH	AGH	BULLETIN 2 - NORTH DITCH & SEDIMENT BASIN
5	4-6-2026	60753972	LANDFILL CLOSURE	JET	JET	AGH	AGH	AGH	BULLETIN 2 - NORTH DITCH & SEDIMENT BASIN

AECOM

SEAL
STATE OF LOUISIANA
ANGELA G. HARRIGAL
License No. 46684
PROFESSIONAL ENGINEER
4/6/2026

8555 United Plaza Blvd., Suite 300
Baton Rouge, Louisiana 70809
225/922-5700

DESIGNER ANGELA G. HARRIGAL
LICENSE NUMBER 46684

THESE DRAWINGS DO NOT INCLUDE NECESSARY COMPONENTS FOR CONSTRUCTION SAFETY.

TITLE
TITLE SHEET

LANDFILL CLOSURE PLANS
DOLET HILLS POWER STATION
MANSFIELD, LOUISIANA

FOR
ISSUED FOR CONSTRUCTION

CLECO

SCALE: AS SHOWN	DES: JET
DWG TYPE:.DWG	DFTR: JET
JOB NO: 60753972	CHKD: AGH
DATE: 4-6-2026	ENGR: AGH
FILENAME: 001 - TITLE SHEET.DWG	APPD: AGH

DWG SIZE	DRAWING NO.	REVISION
ANSI D 22.0"x34.0"	SHEET 001	5

SHEET 001 REV. 5

C:\Users\jared.taylor\OneDrive - AECOM\60753972_Cleco_Dolet_LF_IFC - General\900_CAD_GIS\910_CAD\05-SHEETS\001 - TITLE SHEET.dwg User: jared.taylor Apr 06, 2026 - 2:03pm

1. PURPOSE

2. DEFINITIONS

2.1. THE FACILITY AND ITS ANCILLARY FUNCTIONS ARE OWNED AND OPERATED BY CLECO (OWNER). THE OWNER WILL BE RESPONSIBLE FOR OVERALL MANAGEMENT OF CONSTRUCTION ACTIVITIES INCLUDING CONTRACTING AND ADMINISTRATION. THE OWNER WILL DESIGNATE AN ON-SITE REPRESENTATIVE TO SERVE AS CONSTRUCTION SUPERVISOR (CS). THE OWNER CS WILL SERVE AS THE OWNER REPRESENTATIVE AND OVERSEE ALL ASPECTS OF THE PROJECT. THE OWNER IS CLECO.

2.2. THE ENGINEER IS RESPONSIBLE FOR THE ENGINEERING DESIGN AND THESE PLANS. THE ENGINEER IS AN OFFICIAL REPRESENTATIVE OF THE OWNER. THE ENGINEER WILL BE RESPONSIBLE FOR PROVIDING CONSTRUCTION QUALITY ASSURANCE FOR THE PROJECT AND WORKING WITH THE OWNER ON PROPER RESOLUTION OF QUALITY ISSUES THAT ARISE DURING CONSTRUCTION. THE ENGINEER IS AECOM. AECOM HAS PREPARED THE PLANS AND NOTES HEREIN, AND IS TO ASSIST THE OWNER IN MONITORING THE PERFORMANCE OF THE WORK.

2.3. THE CONTRACTOR FOR THE WORK WILL BE SELECTED BY THE OWNER. THE CONTRACTOR IS RESPONSIBLE FOR COMPLETING THE WORK OUTLINED IN THESE PLANS. ANY SUB-CONTRACTORS PROPOSED BY THE CONTRACTOR MUST BE APPROVED BY THE OWNER. THE CONTRACTOR SHALL BE RESPONSIBLE FOR CONSTRUCTION ACTIVITIES AND CONSTRUCTION QUALITY CONTROL (CQC) ASSOCIATED WITH THIS PROJECT INCLUDING MEETING ALL OF THE REQUIREMENTS FOR PROJECT QUALITY AS DEFINED BY THESE PLANS FOR ITS WORK AS WELL AS THAT OF ITS SUB-CONTRACTORS.

3. SURVEY CONTROL AND EXISTING TOPOGRAPHIC/SITE INFORMATION

3.1. TOPOGRAPHIC SURVEY OF THE SITE IS BASED ON DATA COMPILED FROM TWO SURVEYS.

3.1.1. JANUARY 2021 TO NATIONAL MAP ACCURACY STANDARDS BY MORRISON SURVEYING, SULPHUR, LOUISIANA.

3.1.2. MARCH 2025 TO NATIONAL MAP ACCURACY STANDARDS BY AUCOIN & ASSOCIATES, EUNICE, LOUISIANA.

3.2. MAPPING DATA IS PROVIDED IN US STATE PLANE 1983, LOUISIANA NORTH (NAD 1983).

3.3. SUPPLEMENTAL SURVEY DATA HAS BEEN COLLECTED AND INCORPORATED FOR THE PURPOSE OF THIS WORK

3.4. BENCHMARK LOCATIONS ARE TO BE PROVIDED BY THE OWNER. THE CONTRACTOR SHALL COORDINATE WITH THE OWNER TO RE-ESTABLISH THE BENCHMARKS IF THEY ARE NOT AS PROVIDED BY THE OWNER.

3.5. CONTRACTOR SHALL MAKE HIS OWN INVESTIGATIONS AS TO THE LOCATION OF UTILITIES PRIOR TO COMMENCEMENT OF ANY WORK. EXISTING UTILITIES AND STRUCTURES (SUBSURFACE, SURFACE, AND OVERHEAD) ARE INDICATED ONLY TO THE EXTENT THAT SUCH INFORMATION WAS KNOWN, MADE AVAILABLE, OR DISCOVERED BY THE ENGINEER IN PREPARING THE PLANS. THE LOCATIONS, CONFIGURATIONS, AND ELEVATIONS OR SUBSURFACE FACILITIES AND UTILITIES ARE APPROXIMATE, AND NOT ALL UTILITIES AND FACILITIES MAY BE INDICATED. THE EXISTING CONDITIONS SHOWN ON THESE DRAWINGS ARE BELIEVED TO REPRESENT THE BEST KNOWN DATA AVAILABLE FROM HISTORIC DRAWINGS PROVIDED BY CLECO. ALTHOUGH THIS INFORMATION HAS BEEN INCLUDED IN THE CONSTRUCTION PLANS BY AECOM, THEY ARE NOT NECESSARILY THE WORK PRODUCT OF AECOM AND HAVE BEEN INCORPORATED IN RESPONSE TO CERTAIN ASSIGNED CONDITIONS. AECOM HAS NOT SURVEYED OR FIELD VERIFIED ANY OF THE EXISTING CONDITIONS INFORMATION AND CANNOT BE RESPONSIBLE FOR THE ACCURACY OR COMPLETENESS OF THIS DATA.

4. GENERAL REQUIREMENTS

4.1. THE PROCEDURES SPECIFIED IN THESE PLANS SHALL BE FOLLOWED. PROVIDED THEY ARE CONSISTENT WITH LOCAL PRACTICE IN THE REGION. IF PROCEDURES ARE NOT CONSISTENT WITH LOCAL PRACTICE, ALTERNATE PROCEDURES SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL PRIOR TO THE START OF WORK.

4.2. THE ENGINEER SHALL HAVE THE RIGHT TO REVIEW FOR ACCEPTANCE WORK BEING PERFORMED BY THE CONTRACTOR. THIS REVIEW BY THE ENGINEER DOES NOT ALLEVIATE ANY RESPONSIBILITY OF THE CONTRACTOR TO PROVIDE COMPLETE, ACCURATE, AND CORRECT DATA.

4.3. ACCESS INTO THE WORK AREA ("SITE") FOR DELIVERY OF EQUIPMENT, MATERIALS AND WORKFORCE MUST BE REVIEWED DAILY BY THE CONTRACTOR, AND CONTROLLED AS NEEDED TO PREVENT ANY DAMAGE TO OWNER'S FACILITY INFRASTRUCTURE, EQUIPMENT, OR MATERIALS. IF EQUIPMENT OR MATERIAL HAULING CAUSES DAMAGE, THE DAMAGE MUST BE IMMEDIATELY REPAIRED BY THE CONTRACTOR, AND THE METHODS FOR HAULING MUST BE ALTERED TO PREVENT FURTHER DAMAGE. THE CONTRACTOR SHALL NOT MAKE ANY UNPLANNED EXCAVATIONS WITHOUT THE APPROVAL OF THE OWNER AND THE ENGINEER. DURING CONSTRUCTION, THE CONTRACTOR IS RESPONSIBLE FOR PRECAUTIONS REQUIRED TO PROTECT THE INTEGRITY OF EXISTING PAVEMENT, UTILITIES, OR OTHER SITE FEATURES.

4.4. THE CONTRACTOR SHALL TAKE EVERY PRECAUTION TO AVOID OIL OR FUEL SPILLS DURING CONSTRUCTION. THE CONTRACTOR SHALL HAVE OIL/FUEL CONTROL EQUIPMENT (ABSORBENT BOOMS, ETC.) ON SITE AT ALL TIMES. IF OIL OR FUEL IS RELEASED OR OBSERVED DURING CONSTRUCTION, THE CONTRACTOR SHALL IMMEDIATELY NOTIFY THE OWNER AND DEPLOY OIL/FUEL CONTROL EQUIPMENT. THE CONTRACTOR SHALL COORDINATE WITH THE ENGINEER AND THE OWNER ON OIL/FUEL CLEANUP, AS NECESSARY.

4.5. THE CONTRACTOR SHALL COORDINATE WITH THE OWNER TO DETERMINE THE LOCATION OF STAGING OR PARKING AREAS FOR REQUIRED EQUIPMENT OR MATERIAL STORAGE AND FOR OTHER CONSTRUCTION LAY DOWN ACTIVITY, INCLUDING STOCKPILING. STAGING AND STOCKPILING AREAS SHALL BE PROVIDED WITH EROSION AND SEDIMENT PERIMETER CONTROLS. CONTRACTOR TO CONSULT WITH ENGINEER BEFORE SETTING UP STAGING AND STOCKPILING AREAS.

4.6. WHENEVER REFERENCE IS MADE TO LOUISIANA DEPARTMENT OF TRANSPORTATION AND DEVELOPMENT (LDOTD) STANDARD SPECIFICATIONS, LOUISIANA DEPARTMENT OF ENVIRONMENTAL QUALITY (LDEQ), AMERICAN CONCRETE INSTITUTE (ACI), AMERICAN WATER WORKS ASSOCIATION (AWWA), AMERICAN SOCIETY OF TESTING AND MATERIALS (ASTM) AND OTHER PUBLISHED STANDARDS OR SPECIFICATIONS, IT SHALL MEAN THE LATEST VERSION IN ITS ENTIRETY.

4.7. THE CONTRACTOR SHALL MAKE NO DEVIATIONS FROM THE PLANS, ASSOCIATED SPECIFICATIONS, OR SHOP DRAWINGS WITHOUT PRIOR APPROVAL FROM THE OWNER AND THE ENGINEER. THE CONTRACTOR SHALL KEEP A RECORD PLAN SET NOTING ALL DEVIATIONS IN LOCATION OR ELEVATION OF ANY INSTALLATION FROM THAT SHOWN ON THE PLANS (INCLUDING TEMPORARY EROSION AND SEDIMENTATION CONTROL PLANS). AT COMPLETION OF THE PROJECT, A SET OF RED-LINED DRAWINGS SHALL BE PROVIDED TO THE ENGINEER.

4.8. ALL EXISTING MONITORING WELLS AND ASSOCIATED INSTRUMENTATION SHALL BE PROTECTED BY THE CONTRACTOR DURING CONSTRUCTION ACTIVITIES.

4.9. THE CONTRACTOR IS RESPONSIBLE FOR CONTROL OF FUGITIVE DUST GENERATED DURING CONSTRUCTION AND SHALL COORDINATE WITH CLECO FOR SITE SPECIFIC REQUIREMENTS.

4.10. ALL MATERIALS, LABOR, AND EQUIPMENT ARE TO BE PROVIDED BY THE CONTRACTOR UNLESS STATED OTHERWISE HEREIN.

5. ENVIRONMENTAL PROTECTION

5.1. THE CONTROL OF ENVIRONMENTAL POLLUTION, WHICH COULD RESULT FROM CONSTRUCTION OPERATIONS UNDER THIS CONTRACT, REQUIRES CONSIDERATION OF LAND, WATER, AND AIR QUALITY AT THE SITE.

5.2. THE CONTRACTOR SHALL COMPLY WITH ALL APPLICABLE FEDERAL, STATE, AND LOCAL LAWS AND REGULATIONS CONCERNING ENVIRONMENTAL POLLUTION CONTROL OR ABATEMENT. THE CONTRACTOR SHALL MAKE SURE THAT NECESSARY PERMITS HAVE BEEN OBTAINED AND THAT THIS WORK IS IN COMPLIANCE WITH SUCH PERMITS CONCERNING ENVIRONMENTAL PROTECTION.

5.3. THE OWNER AND THE CONTRACTOR SHALL ESTABLISH THE CRITERIA FOR COMPLIANCE AND ADMINISTRATION OF THE ENVIRONMENTAL POLLUTION CONTROL PROGRAM PRIOR TO COMMENCEMENT OF WORK.

5.4. THE OWNER WILL NOTIFY THE CONTRACTOR IN WRITING OF ANY NONCOMPLIANCE WITH THESE PLANS AND THE ACTION TO BE TAKEN. THE CONTRACTOR SHALL IMMEDIATELY TAKE CORRECTIVE ACTION.

5.5. PROTECTION OF LAND RESOURCES

5.5.1. LAND RESOURCES ADJACENT TO THE PROJECT BOUNDARIES SHALL BE PRESERVED IN THEIR PRESENT CONDITION OR RESTORED TO A NATURAL APPEARANCE.

5.5.2. THE CONTRACTOR SHALL EXERCISE CARE IN CONDUCTING ALL OPERATIONS ON PRIVATE PROPERTY TO MINIMIZE THE AMOUNT OF DISTURBANCE AND DAMAGE RELATED TO GAINING ACCESS TO, AND WORKING AT THE PLANNED LOCATION(S). THE CONTRACTOR SHALL NOT INJURE OR DESTROY TREES OR SHRUBS ADJACENT TO THE PROJECT SITE THAT ARE NOT SHOWN ON THESE PLANS TO BE REMOVED. AREAS DISTURBED BY CONSTRUCTION ACTIVITIES SHALL BE CONFINED TO THE MAXIMUM EXTENT POSSIBLE.

5.5.3. THE CONTRACTOR SHALL MAINTAIN A STABLE, CLEAN, WORKABLE SURFACE IN THE CONSTRUCTION AREA. THE CONTRACTOR WILL BE RESPONSIBLE FOR PURCHASING, HAULING, AND PLACEMENT OF STONE, IF NEEDED. AREAS DISTURBED BY CONSTRUCTION ACTIVITIES SHALL BE MINIMIZED TO THE EXTENT POSSIBLE.

5.5.4. THE CONTRACTOR SHALL PERFORM THE FINAL RESTORATION OF THE CONSTRUCTION AREA (E.G. PLANT GRASS). REPAIRS WHICH ARE, IN THE OPINION OF THE OWNER, REQUIRED TO RESTORE ANY LAND, STRUCTURES, OR APPURTENANCES DAMAGED THROUGH CARELESSNESS, NEGLIGENCE, OR IRRESPONSIBLE ACTS ON THE PART OF THE CONTRACTOR, OR ANY OF THEIR EMPLOYEES, SHALL BE THE SOLE RESPONSIBILITY OF THE CONTRACTOR AND WILL NOT BE PAID FOR BY THE OWNER. SUCH REPAIRS SHALL BE CARRIED OUT IN A TIMELY MANNER AT THE DIRECTION OF, AND TO THE SATISFACTION OF THE OWNER.

5.6. PROTECTION OF WATER RESOURCES

5.6.1. THE CONTRACTOR SHALL NOT ADVERSELY AFFECT THE EXISTING WATER QUALITY WITHIN OR ADJACENT TO THE PROJECT SITE. NO CONSTRUCTION WASTES OR OTHER HARMFUL MATERIALS SHALL BE PERMITTED TO ENTER THESE WATER RESOURCES.

5.6.2. SURFACE DRAINAGE FROM WORK ACTIVITIES SHALL BE MANAGED USING EFFECTIVE EROSION AND SEDIMENT CONTROL BMPS OR THE WORK AREAS SHALL BE GRADED TO CONTROL EROSION WITHIN ACCEPTABLE LIMITS IN ACCORDANCE WITH THE SITE STORMWATER POLLUTION PREVENTION PLAN (SWPPP). THESE MEASURES SHALL BE MAINTAINED UNTIL ALL WORK HAS BEEN COMPLETED. REFER TO THE EROSION AND SEDIMENT CONTROL DETAILS FOR MORE INFORMATION.

6. EARTHWORK AND AGGREGATES

6.1. REFER TO SPECIFICATION SECTION 312300 FOR EXCAVATION SPECIFICATIONS AND SECTION 031200 FOR EARTHWORK AND AGGREGATE SPECIFICATIONS.

7. SURVEYING

6.1. REFER TO SPECIFICATION SECTION 022100 FOR SURVEY SPECIFICATIONS.

7. SEQUENCE OF CONSTRUCTION

7.1. TRAFFIC THROUGH/AROUND THE CONSTRUCTION SITE MUST BE MAINTAINED.

7.2. THE CONTRACTOR IS RESPONSIBLE FOR DETERMINING A DETAILED SEQUENCE OF CONSTRUCTION FOR SUCCESSFUL PROJECT COMPLETION. A RECOMMENDED SEQUENCE OF KEY ITEMS CRITICAL TO PROJECT SUCCESS INCLUDE, BUT ARE NOT LIMITED TO THE FOLLOWING:

7.2.1. INSTALL EROSION CONTROL DEVICES AS SHOWN ON THE PLANS.

7.2.2. EXISTING SETTLING BASIN TO BE EXCAVATED DOWN TO THE PROPOSED GRADES SHOWN ON THE PREVIOUS SHEETS AND THE EXISTING OUTLET RISER SHALL BE REMOVED AND REPLACED WITH A CONCRETE END SECTION, PRIOR TO THE PLACEMENT OF FINAL COVER AND THE INSTALLATION OF THE PIPE FROM THE SOUTHWEST SEEP AREA.

7.2.3. IMPLEMENT CONSTRUCTION CLOSURE DETAILS AS INDICATED ON THE PLANS.

7.2.4. STABILIZE ALL AREAS UPON COMPLETION OF CONSTRUCTION OF FINAL GRADES. IMPLEMENT PERMANENT STABILIZATION MEASURES AS SPECIFIED ON THE PLANS.

7.2.5. EROSION AND SEDIMENT CONTROL MEASURES SHALL BE MAINTAINED UNTIL FINAL STABILIZATION IS ACHIEVED FOR ALL DISTURBED AREAS. DO NOT REMOVE ANY EROSION AND SEDIMENT CONTROL MEASURES WITHOUT PRIOR APPROVAL FROM ENGINEER.

1. VOLUME SUMMARY

CUT/FILL QUANTITIES (IFC REV. 1)
TO REGRADE EXISTING CONDITIONS (AS OF APRIL 2025 SURVEY) TO SUBGRADE OF FINAL COVER SYSTEM

CUT (CY)	159,420
FILL (CY)	152,040
NET (CY)	7,380 (CUT)

AECOM

SEAL

STATE OF LOUISIANA
ANGELA G. HARRIGAL
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4/6/2026

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225/922-5700

DESIGNER ANGELA G. HARRIGAL

LICENSE NUMBER 46684

THESE DRAWINGS DO NOT INCLUDE NECESSARY COMPONENTS FOR CONSTRUCTION SAFETY.

TITLE

GENERAL NOTES

LANDFILL CLOSURE PLANS
DOLET HILLS POWER STATION
MANSFIELD, LOUISIANA

FOR

ISSUED FOR CONSTRUCTION

CLECO

SCALE: AS SHOWN	DES: JET
DWG TYPE:.DWG	DFTR: JET
JOB NO: 60753972	CHKD: AGH
DATE: 4-6-2026	ENGR: AGH
FILENAME: 002 - GENERAL NOTES.DWG	APPD: AGH

DWG SIZE	DRAWING NO.	REVISION
ANSI D 22.0"x34.0"	SHEET 002	5

SHEET 002 REV. 5

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LEGEND

- EXISTING MAJOR CONTOUR (10')
- EXISTING MINOR CONTOUR (2')
- UNPAVED ROAD
- APPROXIMATE PERMITTED LANDFILL LIMIT
- APPROXIMATE 200' BUFFER LIMIT
- LIMITS OF EXISTING LANDFILL CAPPED AREA
- CENTERLINE OF EXISTING PERIMETER DITCH
- STREAM
- UNDERGROUND PIPELINE
- OPEN WATER/POND
- CULVERT
- MONITORING WELL
- PIEZOMETER
- SURVEY CONTROL POINT
- SOIL BORING

POINT NO	NORTHING	EASTING	ELEV	DESCRIPTION
2000	560108.031	2949010.508	236.86	BM SCRIBE AT MONITOR WELL OW12
2001	559895.452	2951394.145	234.92	BM ROD AT MONITOR WELL OW32
2002	558083.07	2951296.893	250.64	BM ROD AT MONITOR WELL OW16
2003	554471.167	2949296.887	295.31	BM ROD NEAR LANDFILL
2005	560647.388	2952074.58	252.67	FLYOVER TARGET 4
2006	557589.946	2952278.502	253.62	FLYOVER TARGET 8
2007	559060.385	2948914.896	243.81	FLYOVER TARGE 6
2008	562118.656	2948917.978	185.25	FLYOVER TARGET 3
2009	560970.449	2942888.046	233.44	FLYOVER TARGET 2
2010	554604.299	2945623.195	351.85	FLYOVER TARGET 1
2011	550415.726	2947197.741	372.17	FLYOVER TARGET 7
2012	554420.868	2941663.24	274.07	FLYOVER TARGET 5

NOTES:

1. HORIZONTAL CONTROL REFERENCED TO NAD 83 LOUISIANA STATE PLANE COORDINATES, NORTH ZONE, PER SOLUTIONS PROVIDED BY LSU'S CENTER FOR GEOMATICS CONSTANTLY OPERATING REFERENCE STATION (CORS).
2. VERTICAL CONTROL IS REFERENCED TO NAVD88 GEOID18 AS PER SOLUTIONS PROVIDED BY LSU'S CENTER FOR GEOMATICS CONSTANTLY OPERATING REFERENCE STATION (CORS).
3. PLANIMETRIC MAPPING AND ORTHOPHOTOS WAS PROVIDED BY GULF COAST AERIAL MAPPING CO., INC. DATE NOVEMBER 8, 2021.
4. THE INFORMATION DEPICTED ON THIS MAP REPRESENTS THE RESULTS OF SURVEYS MADE ON THE DATES INDICATED AND CAN ONLY BE CONSIDERED AS INDICATING THE GENERAL CONDITIONS EXISTING AT THAT TIME.
5. SUPPLEMENTAL SURVEY PROVIDED BY AUCOIN AND ASSOCIATES DATED MARCH 2025. ALL DATA REFERENCE TO NAD83 AND NAVD88.

N

0 200 400 600
GRAPHIC SCALE (IN FEET)

AECOM

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SEAL
STATE OF LOUISIANA
ANGELA G. HARRIGAL
License No. 46684
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4/6/2026

DESIGNER ANGELA G. HARRIGAL
LICENSE NUMBER 46684

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TITLE
EXISTING SITE AERIAL - LANDFILL

LANDFILL CLOSURE PLANS
DOLET HILLS POWER STATION
MANSFIELD, LOUISIANA

FOR
ISSUED FOR CONSTRUCTION

CLECO

SCALE: AS SHOWN	DES: JET
DWG TYPE:.DWG	DFTR: JET
JOB NO: 60753972	CHKD: AGH
DATE: 4-6-2026	ENGR: AGH
FILENAME: 003A - EXISTING SITE AERIAL - LANDFILL.DWG	APPD: AGH

DWG SIZE	DRAWING NO.	REVISION
ANSI D 22.0"x34.0"	SHEET 003A	5

SHEET 003A REV. 5

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LEGEND

- EXISTING MAJOR CONTOUR (10')
- EXISTING MINOR CONTOUR (2')
- UNPAVED ROAD
- DITCH/STREAM CENTERLINE
- CULVERT
- TRAIL
- OPEN WATER
- MONITORING WELL
- PIEZOMETER

N

0 50 100 150
GRAPHIC SCALE (IN FEET)

0 INCHES 1 2 3 TENTHS 10 20 30

AECOM

TITLE
EXISTING SITE AERIAL - LANDFILL SURFACE IMPOUNDMENT
LANDFILL CLOSURE PLANS
DOLET HILLS POWER STATION
MANSFIELD, LOUISIANA

FOR
ISSUED FOR CONSTRUCTION

SEAL
ANGELA G. HARRIGAL
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4/6/2026

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DESIGNER ANGELA G. HARRIGAL
LICENSE NUMBER 46684

THESE DRAWINGS DO NOT INCLUDE NECESSARY COMPONENTS FOR CONSTRUCTION SAFETY.

CLECO

SCALE: AS SHOWN	DES: JET
DWG TYPE:.DWG	DFTR: JET
JOB NO: 60753972	CHKD: AGH
DATE: 4-6-2026	ENGR: AGH
FILENAME: 003B - EXISTING SITE AERIAL - LANDFILL SURFACE IMPOUNDMENT.DWG	APPD: AGH

DWG SIZE	DRAWING NO.	REVISION
ANSI D 22.0"x34.0"	SHEET 003B	5

SHEET 003B REV. 5

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BM ROD NEAR LANDFILL 2003

RUNOFF COLLECTION SUMP

24" CPP INV. EL. = 271.62'

36" CPP INV. EL. = 274.5'

36" DIA. INLET TOP EL. = 279.48'

24" CPP INV. EL. = 274.13'

CONCRETE WEIR

36" CPP INV. EL. = 276.5'

24" DIA. INLET TOP EL. = 318.76'

24" DIA. INLET TOP EL. = 319.78'

24" DIA. INLET TOP EL. = 319.44'

24" DIA. INLET TOP EL. = 319.49'

SETTLING BASIN TO BE DEMOLISHED AND RE-CONFIGURED

REMOVE EXISTING CULVERTS

24" DIA. INLET TOP EL. = 324.68'

24" DIA. INLET TOP EL. = 324.68'

24" DIA. INLET TOP EL. = 325.14'

24" DIA. INLET TOP EL. = 335.74'

EX. 6" DRAIN LINE

APPROXIMATE CENTERLINE OF EXISTING PERIMETER DITCH

24" DIA. INLET TOP EL. = 360.10'

24" DIA. INLET TOP EL. = 360.21'

EXISTING CULVERTS

24" DIA. INLET TOP EL. = 362.43'

24" DIA. INLET TOP EL. = 362.48'

CLEAN EXISTING DITCH AND REMOVE ALL ASH AND PLACE BACK INTO THE LANDFILL. CONTRACTOR TO INFORMATION OWNER/ENGINEER WHEN COMPLETE FOR A VISUAL INSPECTION.

LIMITS OF EXISTING LANDFILL CAPPED AREA

REMOVE EXISTING WHITE HDPE LINER IN CLOSURE AREA ONLY LINER EXTENDING SOUTH TO CLOSED PORTION OF THE LANDFILL SHALL BE CUT AND ANCHORED

APPROX. PERMITTED LANDFILL LIMIT

APPROX. 200' BUFFER LIMIT

LEGEND

- EXISTING MAJOR CONTOUR (10')
- EXISTING MINOR CONTOUR (2')
- UNPAVED ROAD
- APPROXIMATE PERMITTED LANDFILL LIMIT
- APPROXIMATE 200' BUFFER LIMIT
- LIMITS OF EXISTING LANDFILL CAPPED AREA
- CENTERLINE OF EXISTING PERIMETER DITCH
- STREAM
- UNDERGROUND PIPELINE
- OPEN WATER
- CULVERT
- MONITORING WELL
- PIEZOMETER
- SURVEY CONTROL POINT

N

0 200 400 600

GRAPHIC SCALE (IN FEET)

0 INCHES 1 2 3 TENTHS 10 20 30

AECOM

TITLE

DEMOLITION PLAN - LANDFILL

LANDFILL CLOSURE PLANS
DOLET HILLS POWER STATION
MANSFIELD, LOUISIANA

FOR

ISSUED FOR CONSTRUCTION

SEAL

ANGELA G. HARRIGAL
License No. 46684
STATE OF LOUISIANA
PROFESSIONAL ENGINEER
4/6/2026

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225/922-5700

DESIGNER ANGELA G. HARRIGAL

LICENSE NUMBER 46684

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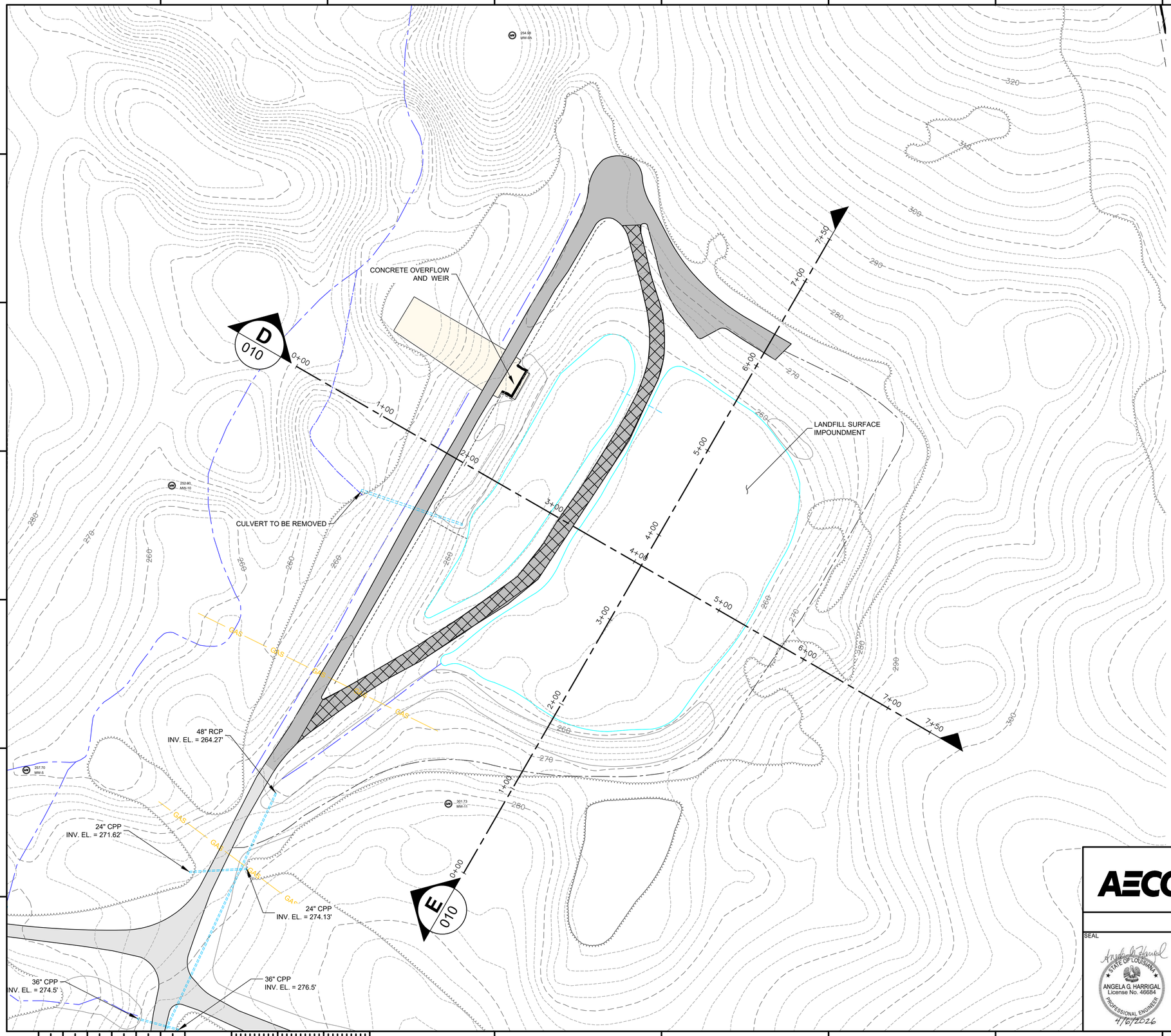
CLECO

SCALE: AS SHOWN	DES: JET
DWG TYPE:.DWG	DFTR: JET
JOB NO: 60753972	CHKD: AGH
DATE: 4-6-2026	ENGR: AGH
FILENAME: 004A - DEMOLITION PLAN - LANDFILL.DWG	APPD: AGH

DWG SIZE	DRAWING NO.	REVISION
ANSI D 22.0"x34.0"	SHEET 004A	5

SHEET 004A REV. 5

C:\Users\jared.taylor\OneDrive - AECOM\60753972_Cleco_Dolet_LF_IFC - General\900_CAD_GIS\910_CAD\05-SHEETS\004A - DEMOLITION PLAN - LANDFILL.dwg User: jared.taylor Apr 06, 2026 - 2:09pm



LEGEND

- EXISTING MAJOR CONTOUR (10')
- EXISTING MINOR CONTOUR (2')
- UNPAVED ROAD
- ROAD TO BE REMOVED
- STREAM
- CULVERT
- OPEN WATER
- MONITORING WELL
- PIEZOMETER

NOTES:

1. THE POND IS TO BE CONTINUOUSLY CLEANED. CONTOURS SHOWN ON THIS SHEET ARE FROM SURVEY CONDUCTED IN 2021 AND MAY NOT ACCURATELY REPRESENT CURRENT SITE CONDITIONS/TOPOGRAPHY/BATHYMETRY.
2. ALL EXCAVATED SEDIMENTS FROM THIS POND ARE TO BE PLACED IN THE ONSITE LANDFILL.

N

0 50 100 150
GRAPHIC SCALE (IN FEET)

AECOM

TITLE
DEMOLITION PLAN- LANDFILL
SURFACE IMPOUNDMENT
LANDFILL CLOSURE PLANS
DOLET HILLS POWER STATION
MANSFIELD, LOUISIANA

FOR
ISSUED FOR CONSTRUCTION

SEAL
STATE OF LOUISIANA
ANGELA G. HARRIGAL
License No. 46684
PROFESSIONAL ENGINEER
4/6/2026

8555 United Plaza Blvd., Suite 300
Baton Rouge, Louisiana 70809
225/922-5700

DESIGNER ANGELA G. HARRIGAL
LICENSE NUMBER 46684

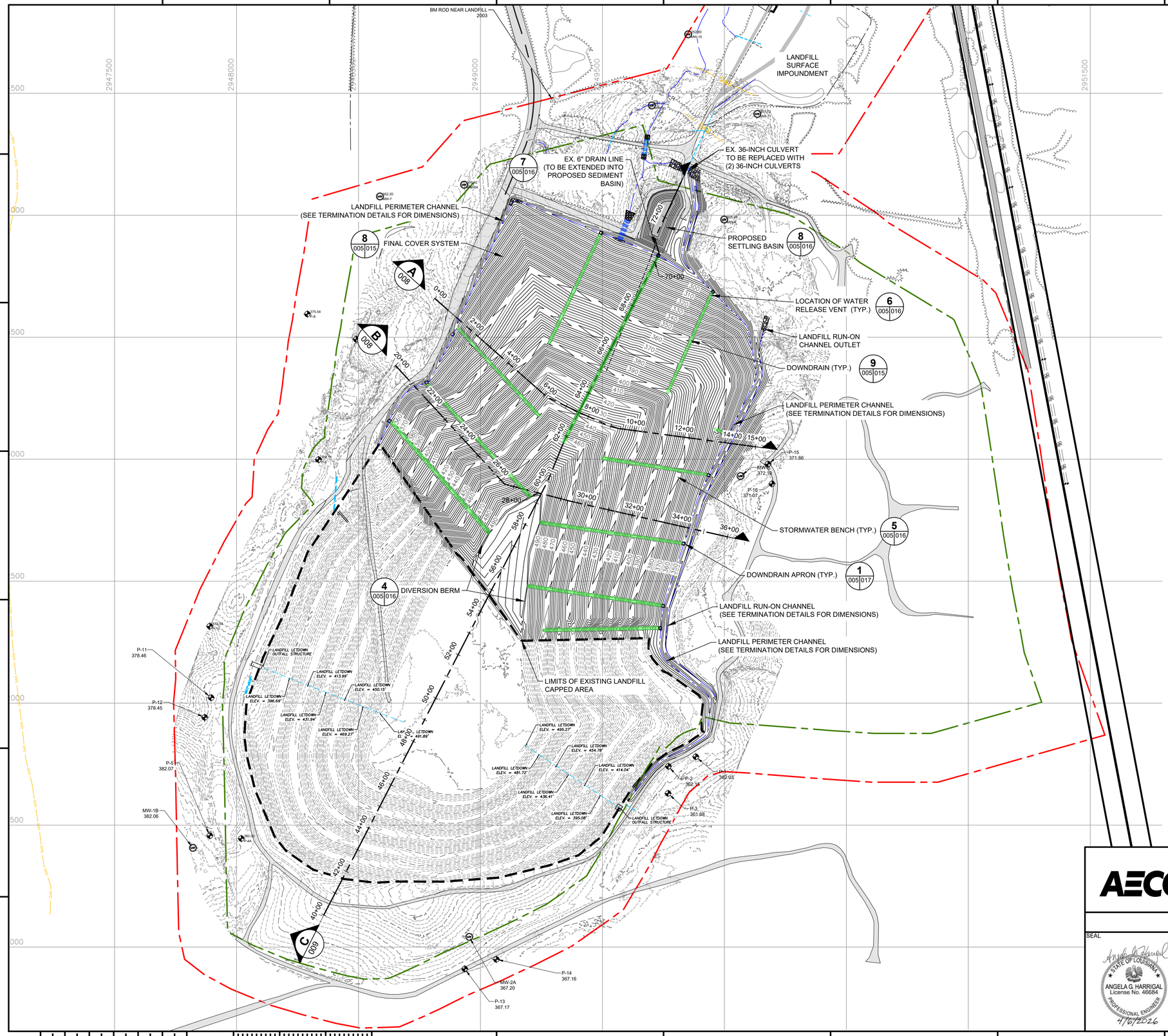
THESE DRAWINGS DO NOT INCLUDE NECESSARY COMPONENTS FOR CONSTRUCTION SAFETY.

CLECO

SCALE: AS SHOWN	DES: JET
DWG TYPE:.DWG	DFTR: JET
JOB NO: 60753972	CHKD: AGH
DATE: 4-6-2026	ENGR: AGH
FILENAME: 004B - DEMOLITION PLAN - LANDFILL SURFACE IMPOUNDMENT.DWG	APPD: AGH
DWG SIZE: ANSI D 22.0"x34.0"	DRAWING NO.: SHEET 004B
	REVISION: 5

SHEET 004B REV. 5

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LEGEND

- EXISTING MAJOR CONTOUR (10')
- EXISTING HISTORIC MINOR CONTOUR (2')
- PROPOSED MAJOR CONTOUR (10') (SEE NOTE 1)
- PROPOSED MINOR CONTOUR (2') (SEE NOTE 1)
- UNPAVED ROAD
- APPROXIMATE PERMITTED LANDFILL LIMIT
- APPROXIMATE 200' BUFFER LIMIT
- LIMITS OF EXISTING LANDFILL CAPPED AREA
- CENTERLINE OF EXISTING PERIMETER DITCH
- DITCH/STREAM CENTERLINE
- OPEN WATER
- EXISTING CULVERT
- PROPOSED CULVERT
- DOWNDRAIN
- CONCRETE MAT
- MONITORING WELL
- PIEZOMETER
- SURVEY CONTROL POINT

NOTES:
1. PROPOSED CONTOURS GENERALLY REPRESENT THE REGRADED TOP OF WASTE PRIOR TO THE PLACEMENT OF THE FINAL COVER SYSTEM, BUT ALSO INCLUDES SOME REGRADING AROUND THE PERIMETER IN EXISTING SOIL THAT IS NOT ASH. ASH SHOULD NOT BE REGRADED OUTSIDE THE LIMITS OF THE FINAL COVER SYSTEM.

N

0 200 400 600
GRAPHIC SCALE (IN FEET)

0 INCHES 1 2 3 TENTHS 10 20 30

AECOM

TITLE
FINAL GRADE - REGRADED TOP OF WASTE
LANDFILL CLOSURE PLANS
DOLET HILLS POWER STATION
MANSFIELD, LOUISIANA

FOR
ISSUED FOR CONSTRUCTION

SEAL
STATE OF LOUISIANA
ANGELA G. HARRIGAL
License No. 46684
PROFESSIONAL ENGINEER
4/6/2026

8555 United Plaza Blvd., Suite 300
Baton Rouge, Louisiana 70809
225/922-5700

DESIGNER ANGELA G. HARRIGAL
LICENSE NUMBER 46684

THESE DRAWINGS DO NOT INCLUDE NECESSARY COMPONENTS FOR CONSTRUCTION SAFETY.

CLECO

SCALE: AS SHOWN	DES: JET
DWG TYPE:.DWG	DFTR: JET
JOB NO: 60753972	CHKD: AGH
DATE: 4-6-2026	ENGR: AGH
FILENAME: 005 - FINAL GRADE - TOP OF REGRADED WASTE.DWG	APPD: AGH

DWG SIZE	DRAWING NO.	REVISION
ANSI D 22.0"x34.0"	SHEET 005	5

SHEET 005 REV. 5

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TERMINATE FINAL COVER SYSTEM AT HIGH POINT OF PERIMETER CHANNEL SIDE SLOPE

TERMINATION TYPE 4 (4|006|015)

PROPOSED SETTLING BASIN

(4) PROP. 36-INCH HDPE CULVERTS

EX. 36-INCH CULVERT TO BE REPLACED WITH (2) 36-INCH CULVERTS

TERMINATION TYPE 3 (3|006|015)

TERMINATION TYPE 10 (3|006|016)

EX. 6" DRAIN LINE (TO BE EXTENDED INTO PROPOSED SETTLING BASIN) (7|006|016)

TERMINATION TYPE 8 (1|006|016)

TERMINATION TYPE 5 (5|006|015)

TERMINATION TYPE 4 (4|006|015)

TERMINATION TYPE 7 (7|006|015)

TERMINATION TYPE 3 (3|006|015)

TERMINATION TYPE 6 (6|006|015)

TERMINATION TYPE 2 (2|006|015)

LIMITS OF EXISTING LANDFILL CAPPED AREA

MATCHLINE (SEE THIS SHEET)

LEGEND

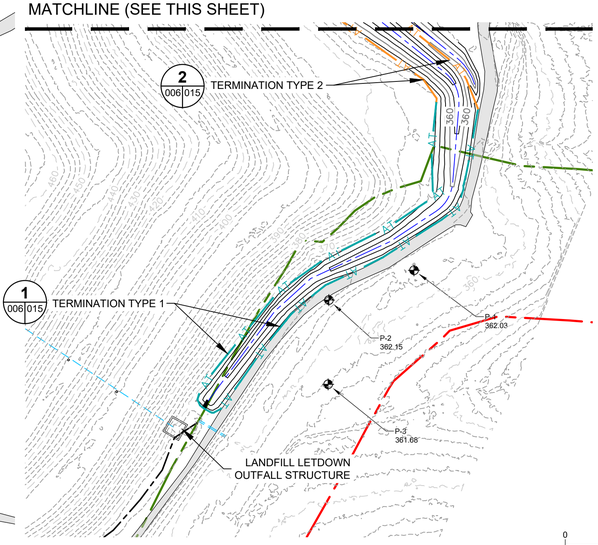
- EXISTING MAJOR CONTOUR (10')
- EXISTING HISTORIC MINOR CONTOUR (2')
- PROPOSED MAJOR CONTOUR (10') (SEE NOTE 1)
- PROPOSED MINOR CONTOUR (2') (SEE NOTE 1)
- UNPAVED ROAD
- APPROXIMATE PERMITTED LANDFILL LIMIT
- APPROXIMATE 200' BUFFER LIMIT
- DITCH/STREAM CENTERLINE
- OPEN WATER
- EXISTING CULVERT
- PROPOSED CULVERT
- DOWNDRAIN
- MONITORING WELL
- PIEZOMETER
- SURVEY CONTROL POINT

ANCHOR TRENCH TERMINATIONS

- TERMINATION TYPE 1
- TERMINATION TYPE 2
- TERMINATION TYPE 3
- TERMINATION TYPE 4
- TERMINATION TYPE 5
- TERMINATION TYPE 6
- TERMINATION TYPE 7
- TERMINATION TYPE 8
- (NOT USED) TERMINATION TYPE 9
- TERMINATION TYPE 10

NOTES:
1. PROPOSED CONTOURS GENERALLY REPRESENT THE REGRADED TOP OF WASTE PRIOR TO THE PLACEMENT OF THE FINAL COVER SYSTEM, BUT ALSO INCLUDES SOME REGRADING AROUND THE PERIMETER IN EXISTING SOIL THAT IS NOT ASH. ASH SHOULD NOT BE REGRADED OUTSIDE THE LIMITS OF THE FINAL COVER SYSTEM.
2. ANCHOR TRENCH LIMITS ARE APPROXIMATE AND MAY NEED TO BE EXTENDED FURTHER IF ASH IS ENCOUNTERED OUTSIDE OF THE PROPOSED LIMITS OF LINER/ANCHOR TRENCH. SEE THE FINAL GRADE SHEETS AND TERMINATION DETAILS FOR MORE INFORMATION.

MATCHLINE (SEE THIS SHEET)



N

0 100 200 300
GRAPHIC SCALE (IN FEET)

AECOM

TITLE
FINAL GRADE - FINAL COVER SYSTEM TERMINATION PLAN
LANDFILL CLOSURE PLANS
DOLET HILLS POWER STATION
MANSFIELD, LOUISIANA

FOR
ISSUED FOR CONSTRUCTION

SEAL
STATE OF LOUISIANA
ANGELA G. HARRIGAL
License No. 46684
PROFESSIONAL ENGINEER
4/6/2026

8555 United Plaza Blvd., Suite 300
Baton Rouge, Louisiana 70809
225/922-5700

DESIGNER ANGELA G. HARRIGAL
LICENSE NUMBER 46684

THESE DRAWINGS DO NOT INCLUDE NECESSARY COMPONENTS FOR CONSTRUCTION SAFETY.

CLECO

SCALE: AS SHOWN	DES: JET
DWG TYPE:.DWG	DFTR: JET
JOB NO: 60753972	CHKD: AGH
DATE: 4-6-2026	ENGR: AGH
FILENAME: 006 - FINAL GRADE - FINAL COVER SYSTEM TERMINATION PLAN.DWG	APPD: AGH

DWG SIZE	DRAWING NO.	REVISION
ANSI D 22.0"x34.0"	SHEET 006	5

SHEET 006 REV. 5

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0 INCHES 1 2 3 TENTHS 10 20 30

PROP. LA DOTD 55 LB. RIPRAP OUTLET PROTECTION 18" THICK MIN. 20' WIDE X 20' LONG MIN.

EX. 48" RCP PIPE

PROP. (2) 36" HDPE CUVLERT PIPE INVERTS TO MATCH EX. 36" CUVLERT

PROP. LA DOTD 10 LB. RIPRAP OUTLET PROTECTION 18" THICK MIN. 40' WIDE X 40' LONG MIN.

3 007|016 TERMINATION TYPE 10

TERMINATE FINAL COVER SYSTEM AT HIGH POINT OF PERIMETER CHANNEL SIDE SLOPE

PROP. LA DOTD 10 LB. RIPRAP OUTLET PROTECTION 30' UPSTREAM WIDTH 50' DOWNSTREAM WIDTH 40' LONG 18" THICK MIN.

PROPOSED SETTLING BASIN BOTTOM EL. 268.0

EX. 36-INCH CULVERT TO BE REPLACED WITH (2) 36-INCH CULVERTS

LANDFILL PERIMETER CHANNEL (SEE TERMINATION DETAILS FOR DIMENSIONS)

2 007|016 7 007|016 PROP. (4) 36" HDPE CUVLERT INLET INV. = 283.0' OUTLET INV. = 272.0'

4 007|015 TERMINATION TYPE 4

TERMINATION TYPE 3 3 007|015

REMOVE EX. RISER DOWN TO EL. 268.4' AND REPLACE WITH CONCRETE END SECTION (EXISTING OUTLET PIPE TO REMAIN) 7 007|016 5 007|017

PROP. LA DOTD 100 LB. RIPRAP 18" THICK MIN.

EXISTING SETTLING POND LINER SYSTEM TO BE REMOVED AND REPLACE WITH PROPOSED POND LINER SYSTEM 3 007|016

EX. 6" DRAIN LINE OUTLET INV. 287.21' (TO BE EXTENDED TO PROPOSED SETTLING BASIN) 7 007|016

1 007|016 TERMINATION TYPE 8

PROP. LA DOTD 55 LB. RIPRAP OUTLET PROTECTION 18" THICK MIN. 5' WIDE X 40' LONG MIN.

TERMINATION TYPE 5 5 007|015

LANDFILL PERIMETER CHANNEL (SEE TERMINATION DETAILS FOR DIMENSIONS)

PROP. LA DOTD 10 LB. RIPRAP OUTLET PROTECTION 18" THICK MIN. 10' WIDE X 30' LONG MIN.

LANDFILL RUN-ON CHANNEL OUTLET

LANDFILL RUN-ON CHANNEL (SEE TERMINATION DETAILS FOR DIMENSIONS)

LEGEND

- EXISTING MAJOR CONTOUR (10')
- EXISTING HISTORIC MINOR CONTOUR (2')
- PROPOSED TOP OF WASTE MAJOR CONTOUR (10')
- PROPOSED TOP OF WASTE MINOR CONTOUR (2')
- UNPAVED ROAD
- APPROXIMATE PERMITTED LANDFILL LIMIT
- APPROXIMATE 200' BUFFER LIMIT
- DITCH/STREAM CENTERLINE
- OPEN WATER
- EXISTING CULVERT
- PROPOSED CULVERT
- DOWNDRAIN
- CONCRETE MAT
- STORMWATER FLOW
- MONITORING WELL
- PIEZOMETER
- SURVEY CONTROL POINT

ANCHOR TRENCH TERMINATIONS

- TERMINATION TYPE 1
- TERMINATION TYPE 2
- TERMINATION TYPE 3
- TERMINATION TYPE 4
- TERMINATION TYPE 5
- TERMINATION TYPE 6
- TERMINATION TYPE 7
- TERMINATION TYPE 8
- (NOT USED) TERMINATION TYPE 9
- TERMINATION TYPE 10

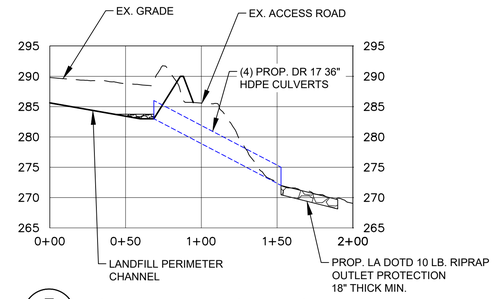
NOTES:

1. PROPOSED CONTOURS GENERALLY REPRESENT THE REGRADED TOP OF WASTE PRIOR TO THE PLACEMENT OF THE FINAL COVER SYSTEM, BUT ALSO INCLUDES SOME REGRADING AROUND THE PERIMETER IN EXISTING SOIL THAT IS NOT ASH. ASH SHOULD NOT BE REGRADED OUTSIDE THE LIMITS OF THE FINAL COVER SYSTEM.
2. ANCHOR TRENCH LIMITS ARE APPROXIMATE AND MAY NEED TO BE EXTENDED FURTHER IF ASH IS ENCOUNTERED OUTSIDE OF THE PROPOSED LIMITS OF LINER/ANCHOR TRENCH. SEE THE FINAL GRADE SHEETS AND TERMINATION DETAILS FOR MORE INFORMATION.
3. AECOM ASSUMES THAT GROUNDWATER IS BELOW THE BOTTOM OF THE PROPOSED EXCAVATION OF THE EXISTING SUMP/PROPOSED SETTLING BASIN. IF GROUNDWATER IS ENCOUNTERED DURING EXCAVATION, AECOM SHOULD BE NOTIFIED AND CONSULTED FOR RECOMMENDATIONS/MODIFICATIONS TO THE BASIN AND ITS LINER SYSTEM

N

0 50 100 150
HORZ. GRAPHIC SCALE (IN FEET)

0 12.5 25 37.5
VERT. GRAPHIC SCALE (IN FEET)



F 007 **PROFILE F - PERIMETER CHANNEL CULVERTS**

AECOM

SEAL

ANGELA G. HARRIGAL License No. 46684 PROFESSIONAL ENGINEER STATE OF LOUISIANA 4/6/2026

8555 United Plaza Blvd., Suite 300
Baton Rouge, Louisiana 70809
225/922-5700

DESIGNER ANGELA G. HARRIGAL
LICENSE NUMBER 46684

THESE DRAWINGS DO NOT INCLUDE NECESSARY COMPONENTS FOR CONSTRUCTION SAFETY.

TITLE

FINAL GRADE - SETTLING BASIN

LANDFILL CLOSURE PLANS
DOLET HILLS POWER STATION
MANSFIELD, LOUISIANA

FOR ISSUED FOR CONSTRUCTION

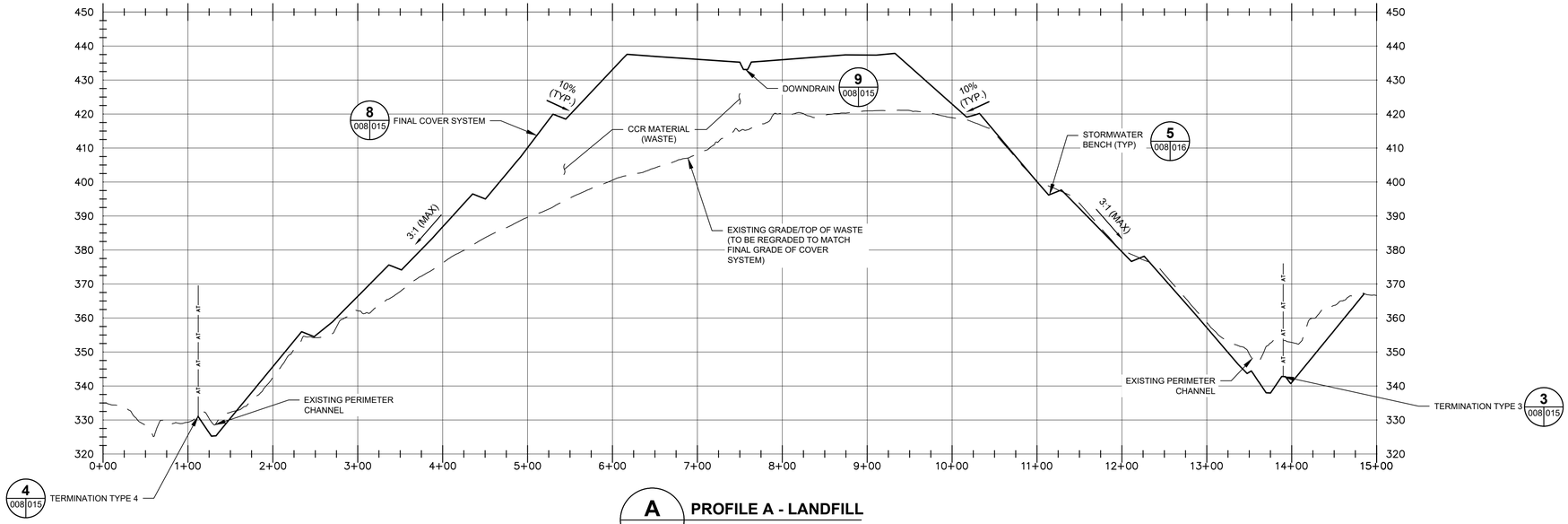
CLECO

SCALE: AS SHOWN	DES: JET
DWG TYPE:.DWG	DFTR: JET
JOB NO: 60753972	CHKD: AGH
DATE: 4-6-2026	ENGR: AGH
FILENAME: 007 - FINAL GRADE - SETTLING BASIN.DWG	APPD: AGH

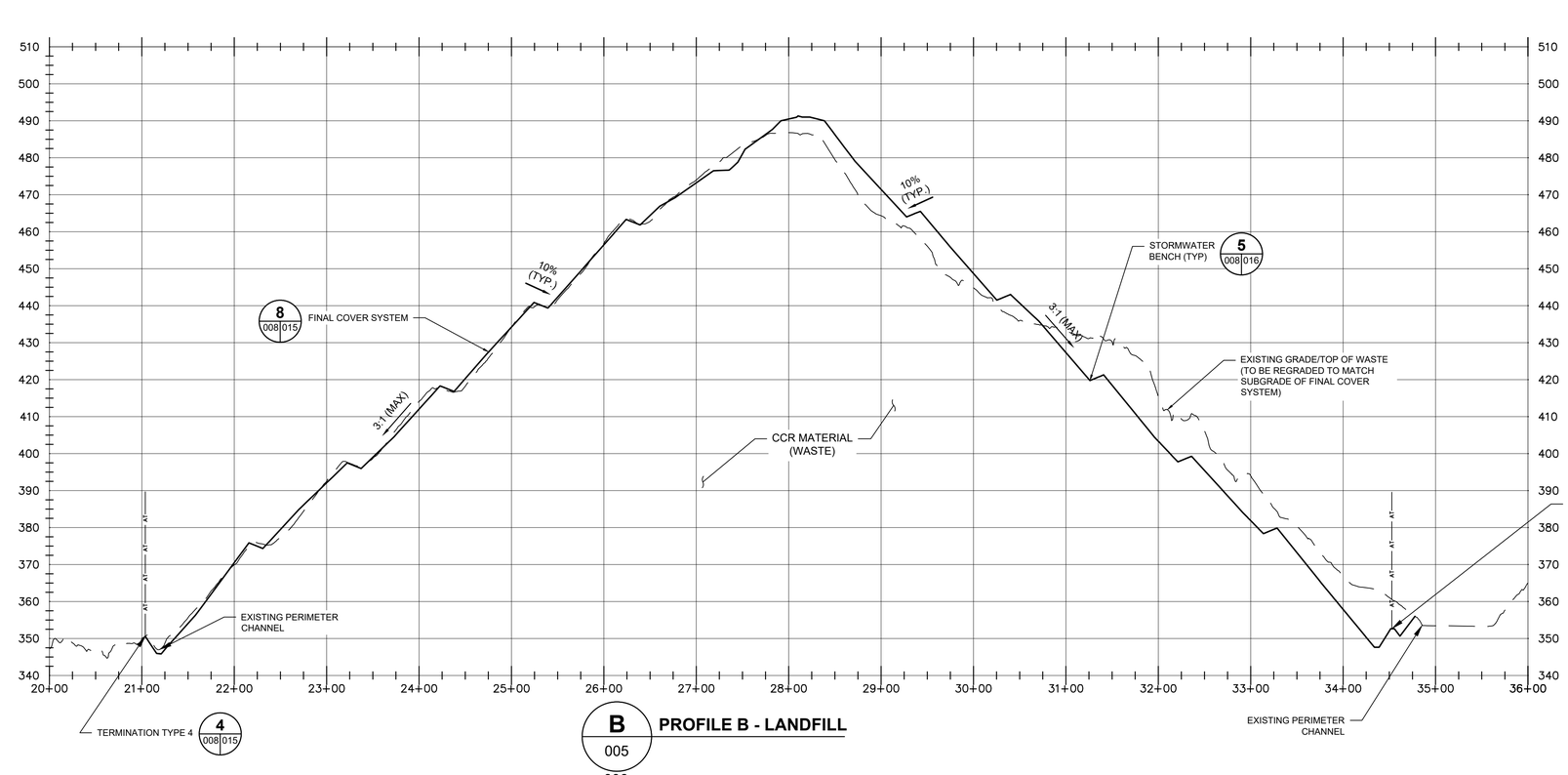
DWG SIZE	DRAWING NO.	REVISION
ANSI D 22.0"x34.0"	SHEET 007	5

SHEET 007 REV. 5

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A 005 006 **PROFILE A - LANDFILL**



B 005 006 **PROFILE B - LANDFILL**

LEGEND

- EXISTING GRADE
- PROPOSED GRADES (SEE NOTE 3)
- AT AT AT AT — ANCHOR TRENCH (FINAL COVER SYSTEM)

NOTES:

1. MAXIMUM PERMITTED AIRSPACE OF LANDFILL IS EL. 544'. ENGINEER SHOULD BE NOTIFIED IF ASH IS PROPOSED TO BE INSTALLED ABOVE THE PROPOSED GRADES SHOWN IN THESE DRAWINGS.
2. ANCHOR TRENCH LIMITS ARE APPROXIMATE AND MAY NEED TO BE EXTENDED FURTHER IF ASH IS ENCOUNTERED OUTSIDE OF THE PROPOSED LIMITS OF LINER/ANCHOR TRENCH. SEE THE FINAL GRADE SHEETS AND TERMINATION DETAILS FOR MORE INFORMATION.
3. PROPOSED CONTOURS GENERALLY REPRESENT THE REGRADED TOP OF WASTE PRIOR TO THE PLACEMENT OF THE FINAL COVER SYSTEM, BUT ALSO INCLUDES SOME REGRADING AROUND THE PERIMETER IN EXISTING SOIL THAT IS NOT ASH. ASH SHOULD NOT BE GRADED OUTSIDE OF THE LIMITS OF THE FINAL COVER SYSTEM.

0 80 160 240
HORIZONTAL SCALE (IN FEET)

0 20 40 60
VERTICAL SCALE

AECOM

TITLE
PROFILES I

LANDFILL CLOSURE PLANS
DOLET HILLS POWER STATION
MANSFIELD, LOUISIANA

FOR
ISSUED FOR CONSTRUCTION

SEAL
ANGELA G. HARRIGAL
STATE OF LOUISIANA
License No. 46684
PROFESSIONAL ENGINEER
4/6/2026

8555 United Plaza Blvd., Suite 300
Baton Rouge, Louisiana 70809
225/922-5700

DESIGNER ANGELA G. HARRIGAL
LICENSE NUMBER 46684

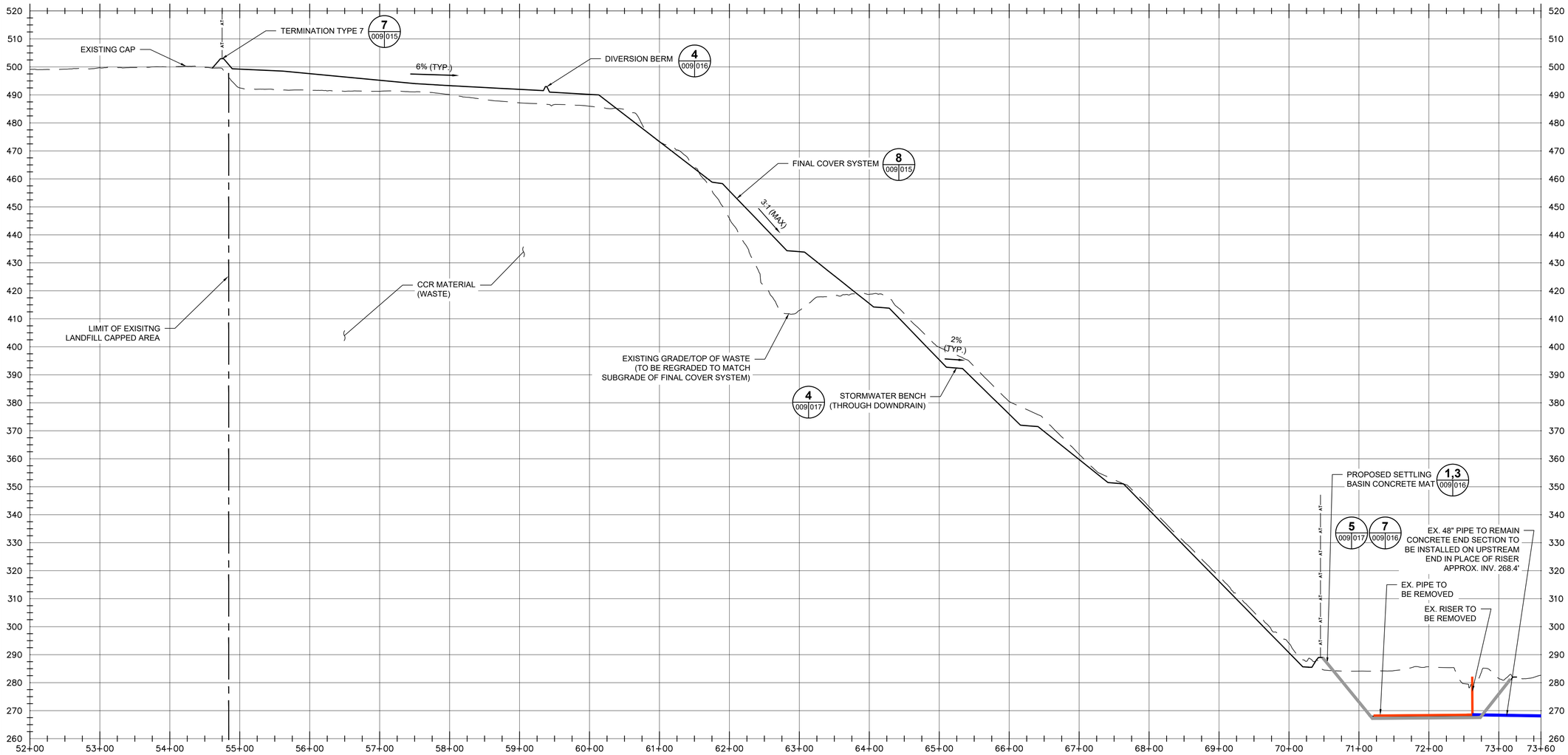
THESE DRAWINGS DO NOT INCLUDE NECESSARY COMPONENTS FOR CONSTRUCTION SAFETY.

CLECO

SCALE: AS SHOWN	DES: JET
DWG TYPE:.DWG	DFTR: JET
JOB NO: 60753972	CHKD: AGH
DATE: 4-6-2026	ENGR: AGH
FILENAME: 008-009 - PROFILES I&II.DWG	APPD: AGH

DWG SIZE	DRAWING NO.	REVISION
ANSI D 22.0"x34.0"	SHEET 008	5

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LEGEND

- EXISTING GRADE
- PROPOSED GRADES (SEE NOTE 3)
- AT AT AT AT — ANCHOR TRENCH (FINAL COVER SYSTEM)

NOTES:

1. MAXIMUM PERMITTED AIRSPACE OF LANDFILL IS EL. 544'. ENGINEER SHOULD BE NOTIFIED IF ASH IS PROPOSED TO BE INSTALLED ABOVE THE PROPOSED GRADES SHOWN IN THESE DRAWINGS.
2. ANCHOR TRENCH LIMITS ARE APPROXIMATE AND MAY NEED TO BE EXTENDED FURTHER IF ASH IS ENCOUNTERED OUTSIDE OF THE PROPOSED LIMITS OF LINER/ANCHOR TRENCH. SEE THE FINAL GRADE SHEETS AND TERMINATION DETAILS FOR MORE INFORMATION.
3. PROPOSED CONTOURS GENERALLY REPRESENT THE REGRADED TOP OF WASTE PRIOR TO THE PLACEMENT OF THE FINAL COVER SYSTEM, BUT ALSO INCLUDES SOME REGRADING AROUND THE PERIMETER IN EXISTING SOIL THAT IS NOT ASH. ASH SHOULD NOT BE GRADED OUTSIDE OF THE LIMITS OF THE FINAL COVER SYSTEM.

0 80 160 240
HORIZONTAL SCALE (IN FEET)

0 20 40 60
VERTICAL SCALE

C 005 — **PROFILE C - LANDFILL**

006
007

AECOM

TITLE

PROFILES II

LANDFILL CLOSURE PLANS
DOLET HILLS POWER STATION
MANSFIELD, LOUISIANA

FOR

ISSUED FOR CONSTRUCTION

SEAL

STATE OF LOUISIANA
ANGELA G. HARRIGAL
License No. 46684
PROFESSIONAL ENGINEER
4/6/2026

8555 United Plaza Blvd., Suite 300
Baton Rouge, Louisiana 70809
225/922-5700

DESIGNER ANGELA G. HARRIGAL

LICENSE NUMBER 46684

THESE DRAWINGS DO NOT INCLUDE NECESSARY COMPONENTS FOR CONSTRUCTION SAFETY.

CLECO

SCALE: AS SHOWN		DES: JET	
DWG TYPE: .DWG		DFTR: JET	
JOB NO: 60753972		CHKD: AGH	
DATE: 4-6-2026		ENGR: AGH	
FILENAME: 008-009 - PROFILES I&II.DWG		APPD: AGH	
DWG SIZE	DRAWING NO.		REVISION
ANSI D 22.0"x34.0"	SHEET 009		5

SHEET 009 REV. 5

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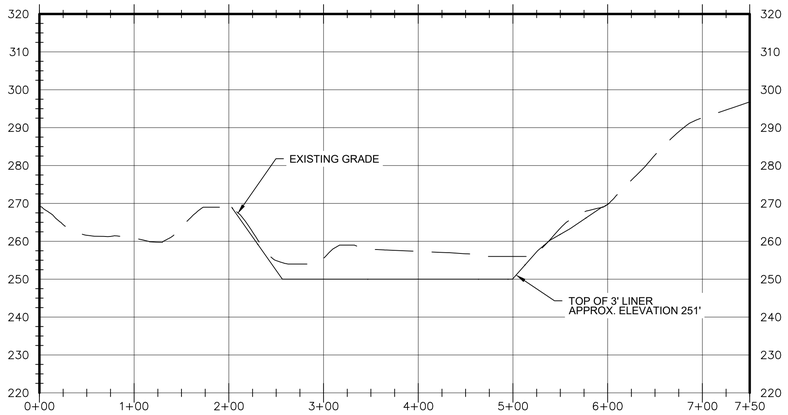
LEGEND

EXISTING GRADE/TOP OF WASTE

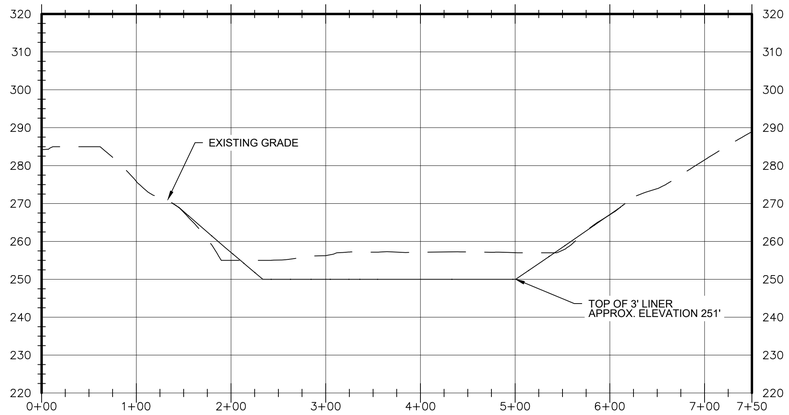
APPROX. TOP OF CLAY LINER

NOTES:

1. THE LANDFILL SURFACE IMPOUNDMENT IS TO BE CONTINUOUSLY CLEANED. CONTOURS SHOWN ON THIS SHEET ARE FROM SURVEY CONDUCTED IN 2021 AND MAY NOT ACCURATELY REPRESENT CURRENT SITE CONDITIONS/TOPOGRAPHY/BATHYMETRY.
2. ALL EXCAVATED SEDIMENTS FROM THE LANDFILL SURFACE IMPOUNDMENT ARE TO BE PLACED IN THE ONSITE LANDFILL.



D 004B **PROFILE D - LANDFILL SURFACE IMPOUNDMENT**



E 004B **PROFILE E- LANDFILL SURFACE IMPOUNDMENT**

HORIZONTAL SCALE (IN FEET): 0, 80, 160, 240

VERTICAL SCALE (IN FEET): 0, 20, 40, 60

AECOM

TITLE

PROFILES III

LANDFILL CLOSURE PLANS
DOLET HILLS POWER STATION
MANSFIELD, LOUISIANA

FOR

ISSUED FOR CONSTRUCTION

SEAL

ANGELA G. HARRIGAL
License No. 46684
STATE OF LOUISIANA
PROFESSIONAL ENGINEER
4/6/2026

8555 United Plaza Blvd., Suite 300
Baton Rouge, Louisiana 70809
225/922-5700

DESIGNER ANGELA G. HARRIGAL

LICENSE NUMBER 46684

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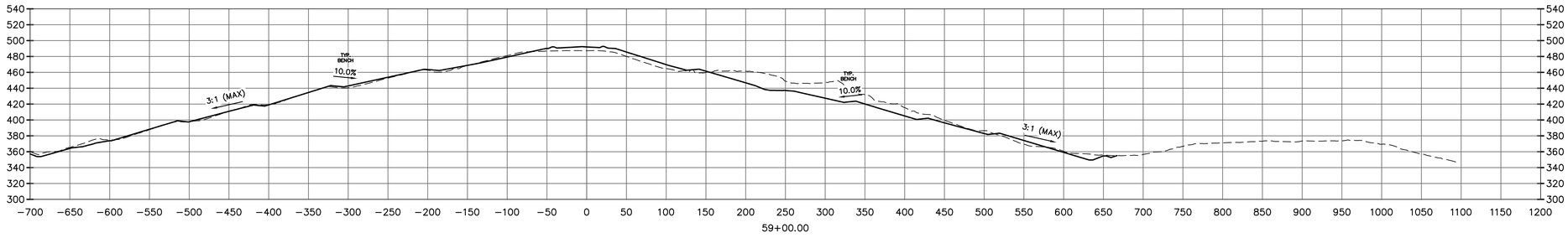
CLECO

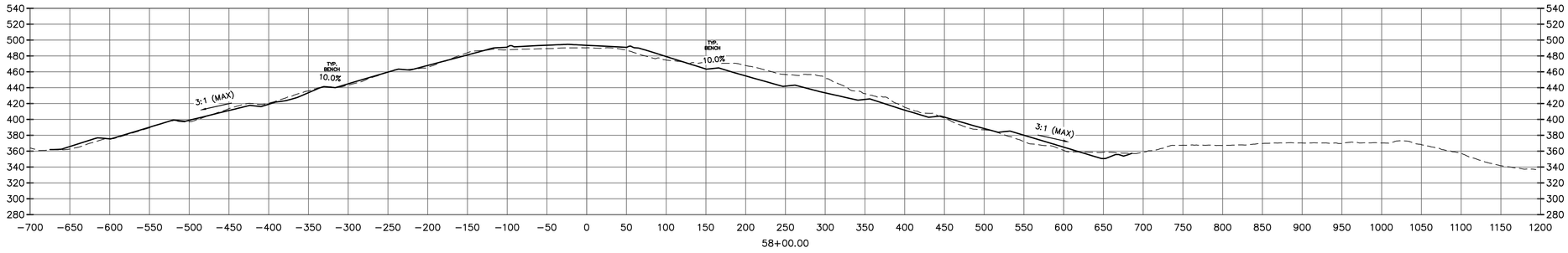
SCALE: AS SHOWN	DES: JET
DWG TYPE:.DWG	DFTR: JET
JOB NO: 60753972	CHKD: AGH
DATE: 4-6-2026	ENGR: AGH
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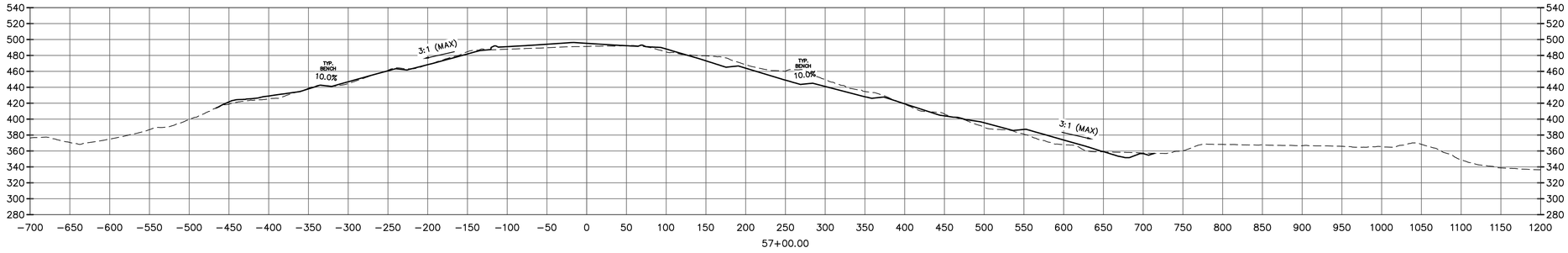
DWG SIZE	DRAWING NO.	REVISION
ANSI D 22.0"x34.0"	SHEET 010	5

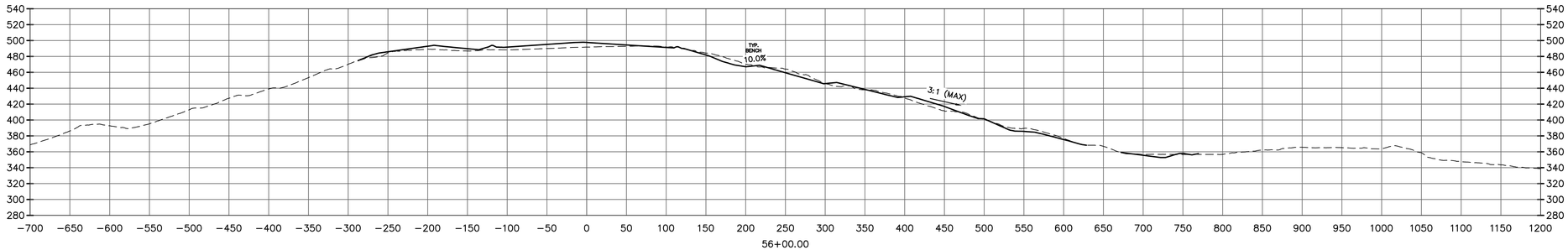
SHEET 010 REV. 5

Apr 06, 2026 – 2:31pm User: jared.taylor C:\Users\jared.taylor\OneDrive – AECOM\60753972_Cleco_Dolet_LF_IFC – General\900_CAD_GIS\910_CAD\05-SHEETS\010 – PROFILES III.dwg
Ref: J:\03-REFERENCE\ANDELL\Cleco Title Block - 22X34 - Dolet Hills Landfill Closure Older.dwg









LEGEND

EXISTING GRADE

PROPOSED GRADES (SEE NOTE 2)

NOTES:

1. MAXIMUM PERMITTED AIRSPACE OF LANDFILL IS EL. 544'. ENGINEER SHOULD BE NOTIFIED IF ASH IS PROPOSED TO BE INSTALLED ABOVE THE PROPOSED GRADES SHOWN IN THESE DRAWINGS.
2. PROPOSED CONTOURS GENERALLY REPRESENT THE REGRADED TOP OF WASTE PRIOR TO THE PLACEMENT OF THE FINAL COVER SYSTEM, BUT ALSO INCLUDES SOME REGRADING AROUND THE PERIMETER IN EXISTING SOIL THAT IS NOT ASH. ASH SHOULD NOT BE GRADED OUTSIDE OF THE LIMITS OF THE FINAL COVER SYSTEM.

0 80 160 240
GRAPHIC SCALE (IN FEET)

C
005
006
007
CROSS SECTIONS STA 56+00 TO 59+00

AECOM

TITLE
CROSS SECTIONS I

LANDFILL CLOSURE PLANS
DOLET HILLS POWER STATION
MANSFIELD, LOUISIANA

FOR
ISSUED FOR CONSTRUCTION

SEAL
STATE OF LOUISIANA
ANGELA G. HARRIGAL
License No. 46684
PROFESSIONAL ENGINEER
4/6/2026

8555 United Plaza Blvd., Suite 300
Baton Rouge, Louisiana 70809
225/922-5700

DESIGNER ANGELA G. HARRIGAL
LICENSE NUMBER 46684

THESE DRAWINGS DO NOT INCLUDE NECESSARY COMPONENTS FOR CONSTRUCTION SAFETY.

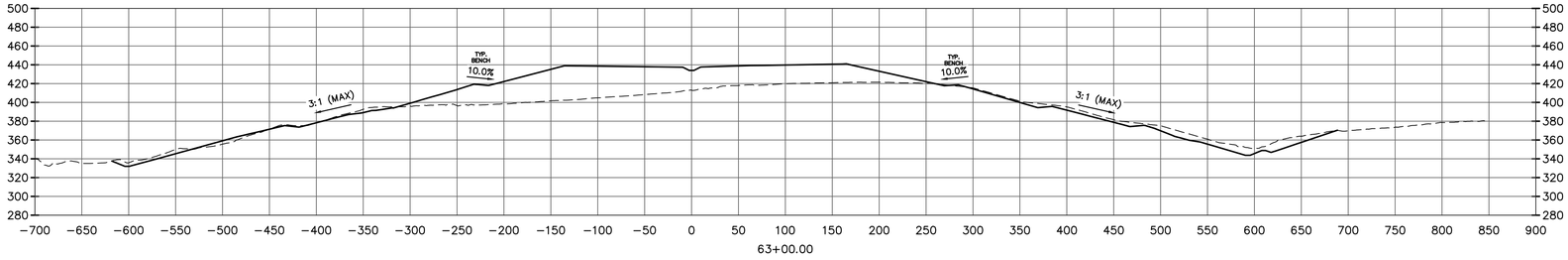
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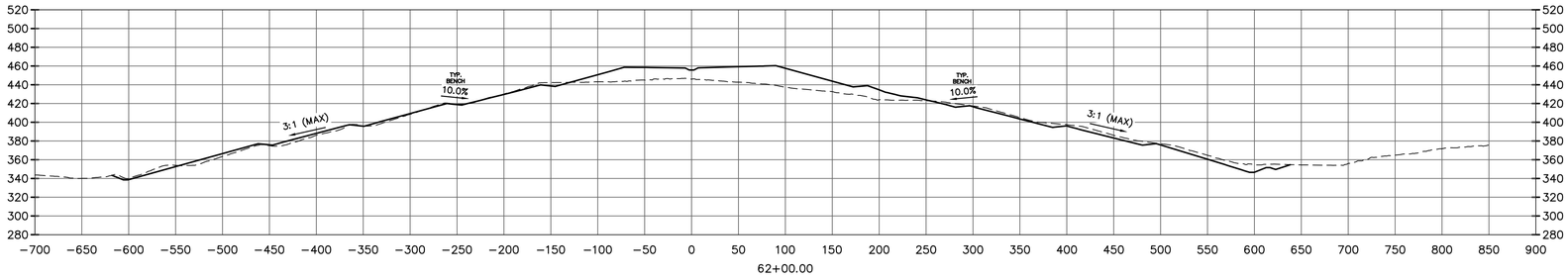
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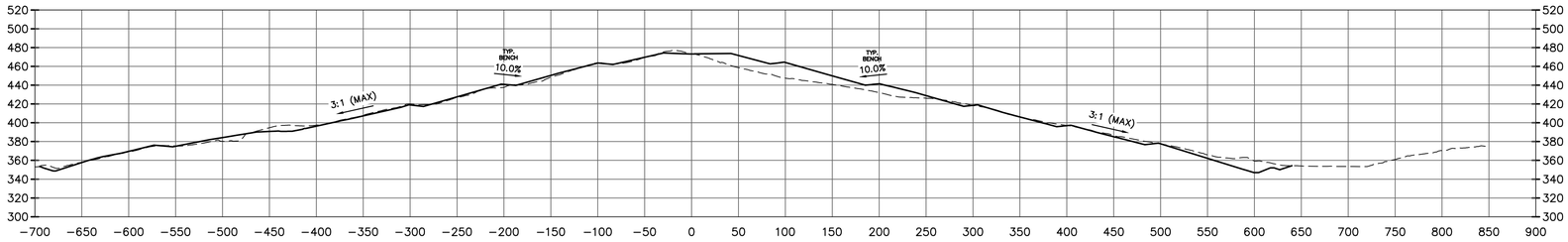
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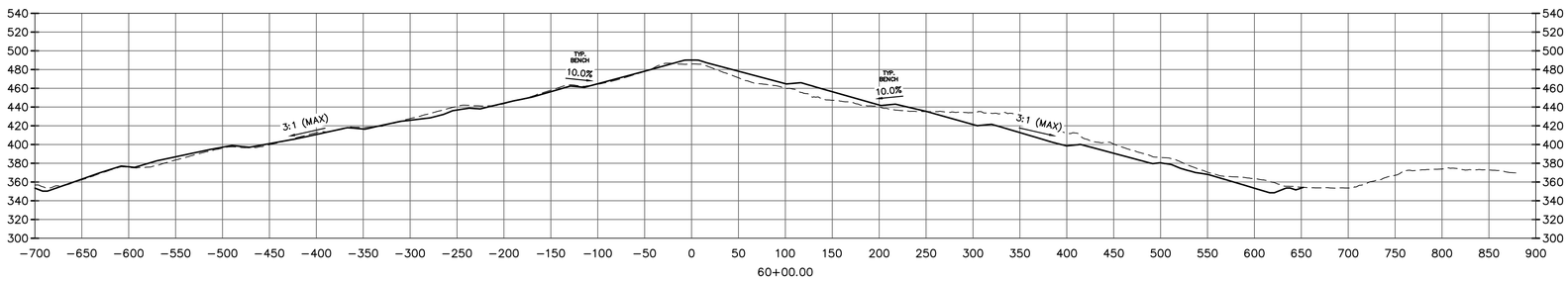
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LEGEND

EXISTING GRADE

PROPOSED GRADES (SEE NOTE 2)

NOTES:

1. MAXIMUM PERMITTED AIRSPACE OF LANDFILL IS EL. 544'. ENGINEER SHOULD BE NOTIFIED IF ASH IS PROPOSED TO BE INSTALLED ABOVE THE PROPOSED GRADES SHOWN IN THESE DRAWINGS.
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0 80 160 240
GRAPHIC SCALE (IN FEET)

C
005
006
007
CROSS SECTIONS STA 60+00 TO 63+00

AECOM

TITLE
CROSS SECTIONS II

LANDFILL CLOSURE PLANS
DOLET HILLS POWER STATION
MANSFIELD, LOUISIANA

FOR
ISSUED FOR CONSTRUCTION

SEAL
STATE OF LOUISIANA
ANGELA G. HARRIGAL
License No. 46684
PROFESSIONAL ENGINEER
4/6/2026

8555 United Plaza Blvd., Suite 300
Baton Rouge, Louisiana 70809
225/922-5700

DESIGNER ANGELA G. HARRIGAL
LICENSE NUMBER 46684

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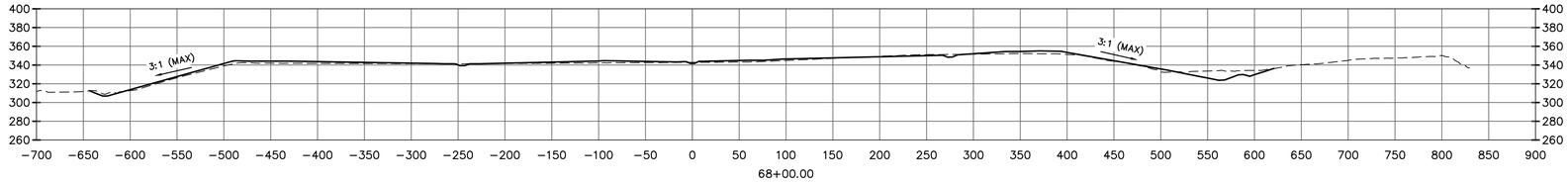
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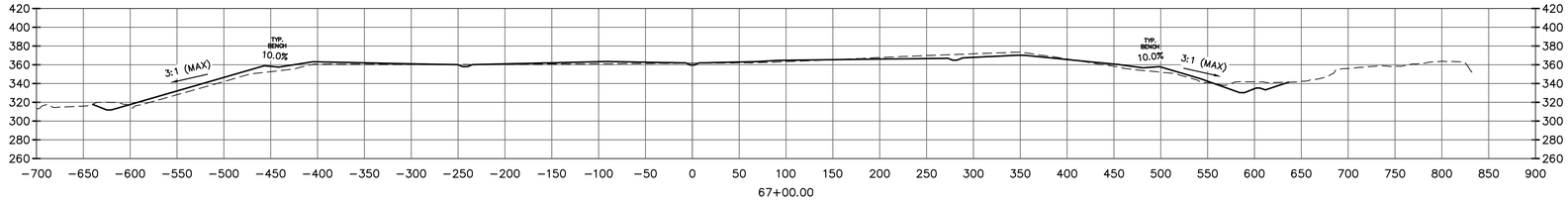
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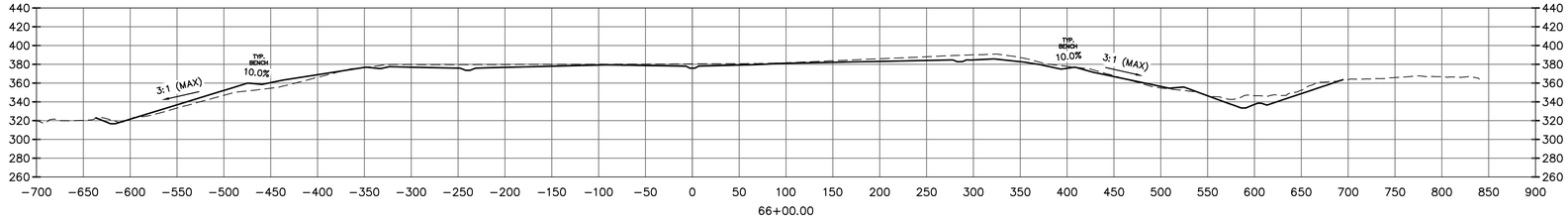
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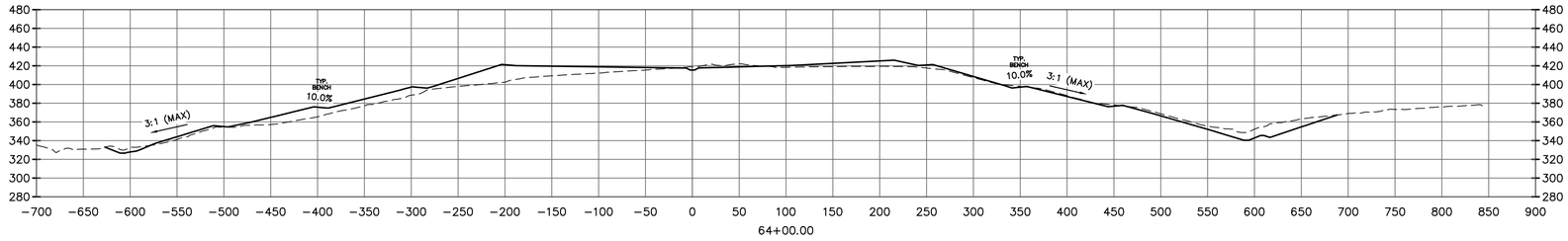
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LEGEND

EXISTING GRADE

PROPOSED GRADES (SEE NOTE 2)

NOTES:

1. MAXIMUM PERMITTED AIRSPACE OF LANDFILL IS EL. 544'. ENGINEER SHOULD BE NOTIFIED IF ASH IS PROPOSED TO BE INSTALLED ABOVE THE PROPOSED GRADES SHOWN IN THESE DRAWINGS.
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0 80 160 240
GRAPHIC SCALE (IN FEET)

C
005
CROSS SECTIONS STA 64+00 TO 68+00
006
007

AECOM

TITLE
CROSS SECTIONS III

LANDFILL CLOSURE PLANS
DOLET HILLS POWER STATION
MANSFIELD, LOUISIANA

FOR
ISSUED FOR CONSTRUCTION

SEAL
STATE OF LOUISIANA
ANGELA G. HARRIGAL
License No. 46684
PROFESSIONAL ENGINEER
4/6/2026

8555 United Plaza Blvd., Suite 300
Baton Rouge, Louisiana 70809
225/922-5700

DESIGNER ANGELA G. HARRIGAL
LICENSE NUMBER 46684

THESE DRAWINGS DO NOT INCLUDE NECESSARY COMPONENTS FOR CONSTRUCTION SAFETY.

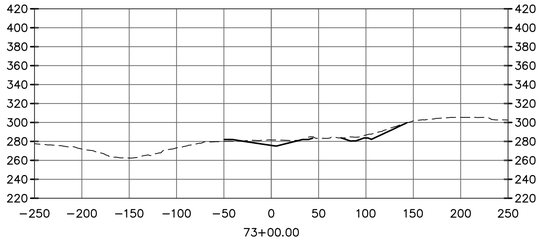
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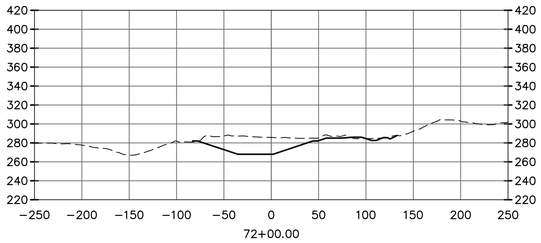
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SHEET 013 REV. 5

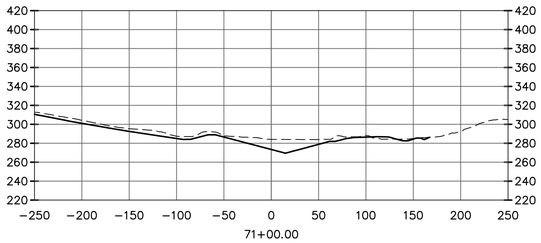
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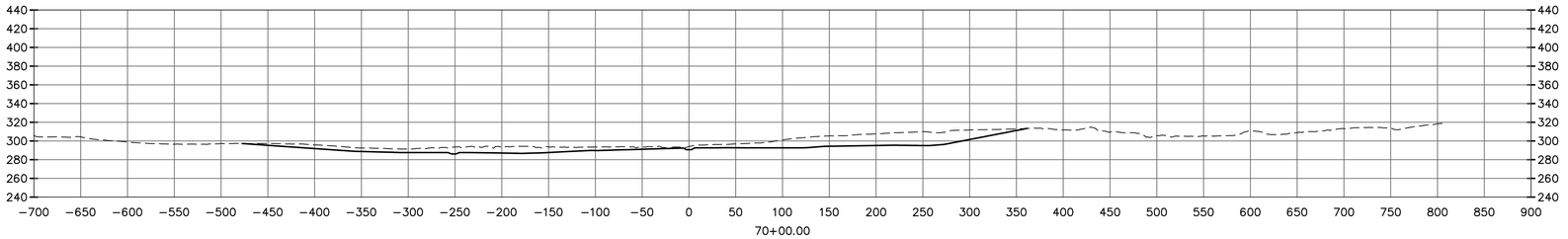
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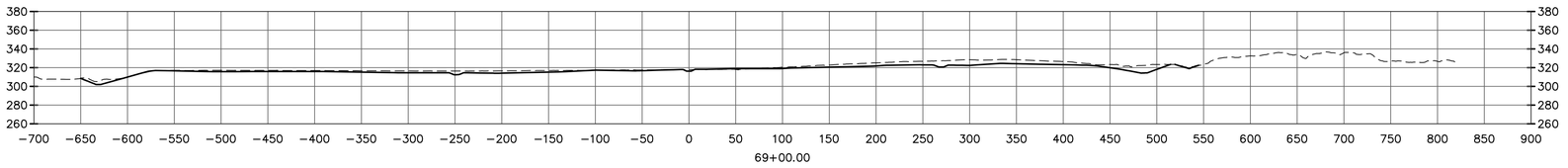
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71+00.00



70+00.00



69+00.00

LEGEND

EXISTING GRADE

PROPOSED GRADES (SEE NOTE 2)

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0 80 160 240
GRAPHIC SCALE (IN FEET)

C
005
CROSS SECTIONS STA 69+00 TO 73+00
006
007

AECOM

8555 United Plaza Blvd., Suite 300
Baton Rouge, Louisiana 70809
225/922-5700

SEAL
ANGELA G. HARRIGAL
STATE OF LOUISIANA
License No. 46684
PROFESSIONAL ENGINEER
4/6/2026

DESIGNER ANGELA G. HARRIGAL
LICENSE NUMBER 46684

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TITLE
CROSS SECTIONS IV

LANDFILL CLOSURE PLANS
DOLET HILLS POWER STATION
MANSFIELD, LOUISIANA

FOR
ISSUED FOR CONSTRUCTION

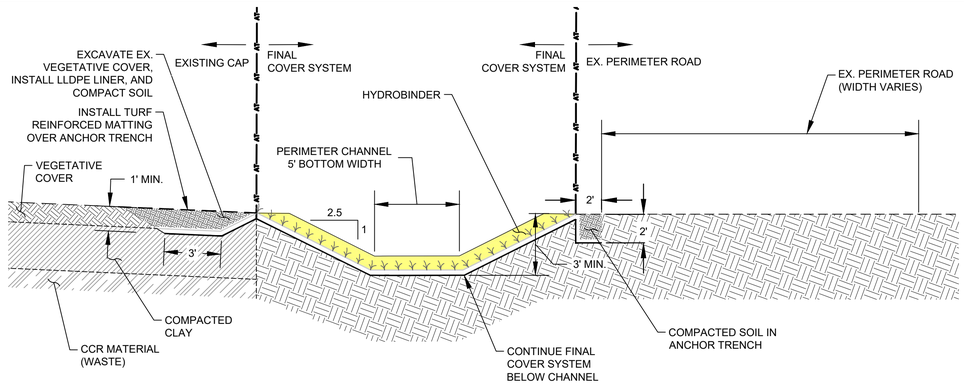
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FILENAME: 011-014 - CROSS SECTIONS I-IV.DWG	APPD: AGH

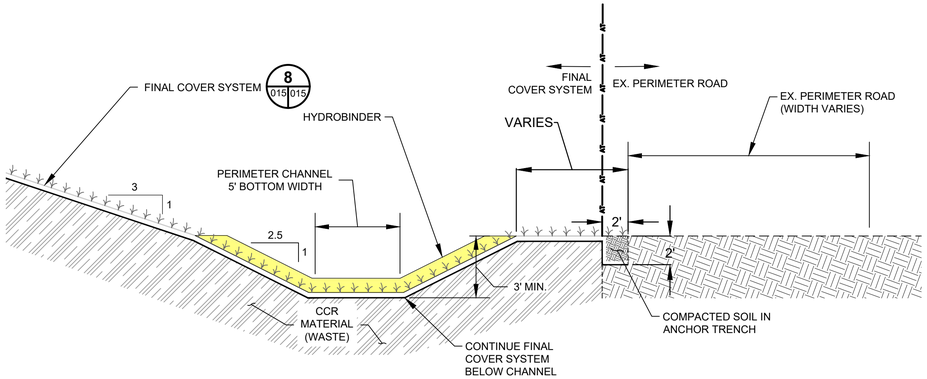
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SHEET 014 REV. 5

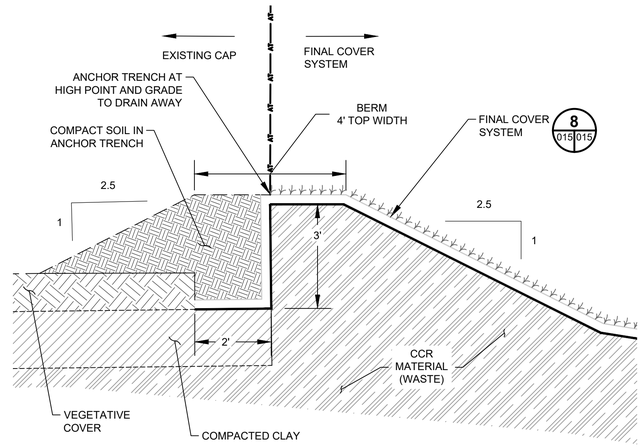
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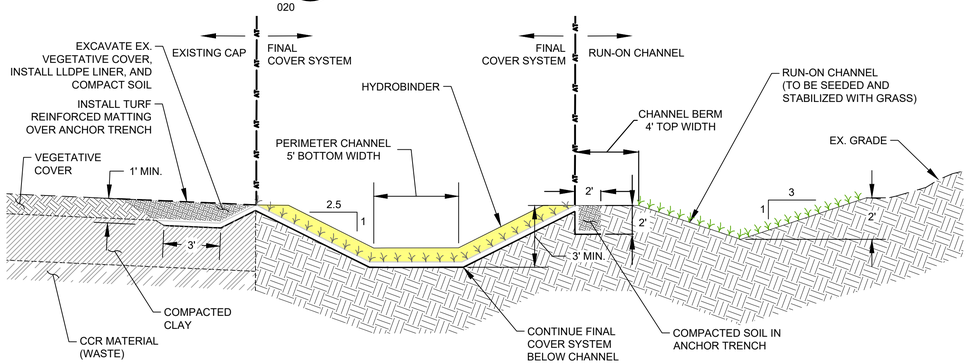
1 / 006|015 **TERMINATION TYPE 1**
NOT TO SCALE



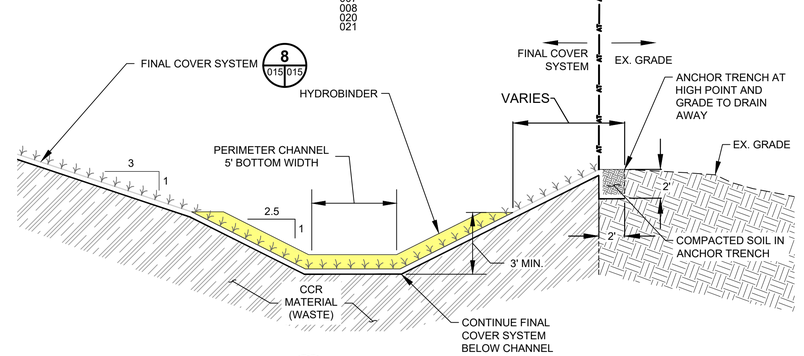
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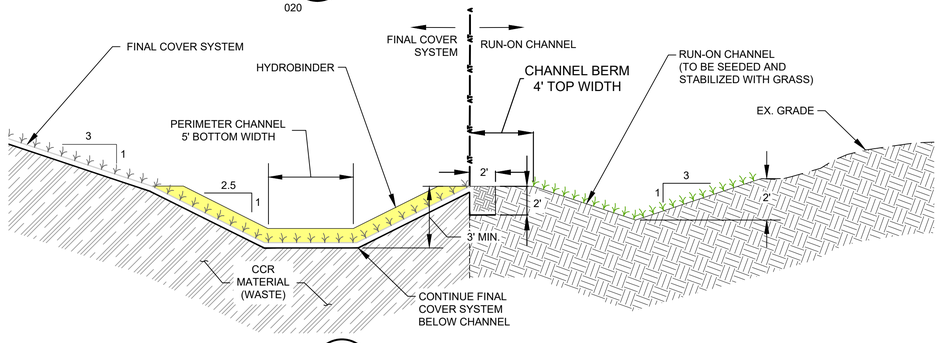
7 / 006|015 **TERMINATION TYPE 7**
NOT TO SCALE
009



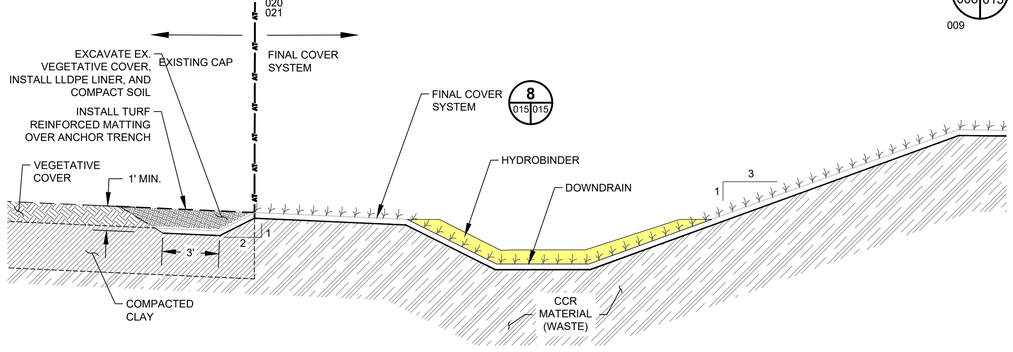
2 / 006|015 **TERMINATION TYPE 2**
NOT TO SCALE
020



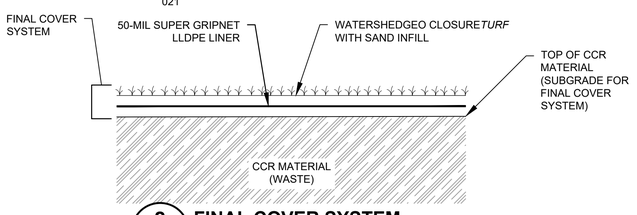
5 / 006|015 **TERMINATION TYPE 5**
NOT TO SCALE
007
008
020
021



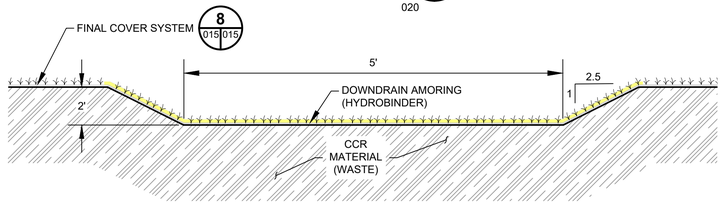
3 / 006|015 **TERMINATION TYPE 3**
NOT TO SCALE
007
008
020
021



6 / 006|015 **TERMINATION TYPE 6**
NOT TO SCALE
020



8 / 005|015 **FINAL COVER SYSTEM**
NOT TO SCALE
008
009



9 / 005|015 **DOWNDRAIN**
NOT TO SCALE

AECOM

TITLE
DETAILS I

LANDFILL CLOSURE PLANS
DOLET HILLS POWER STATION
MANSFIELD, LOUISIANA

FOR
ISSUED FOR CONSTRUCTION

SEAL
ANGELA G. HARRIGAL
License No. 46684
PROFESSIONAL ENGINEER
STATE OF LOUISIANA
4/6/2026

8555 United Plaza Blvd., Suite 300
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225/922-5700

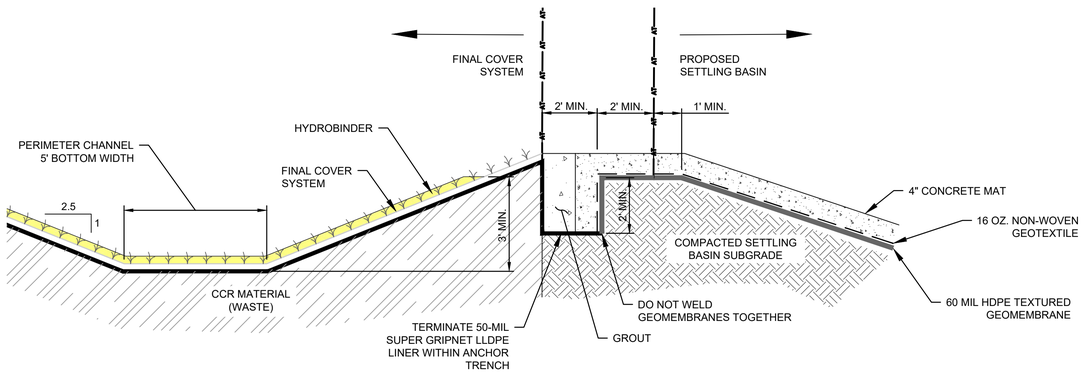
DESIGNER ANGELA G. HARRIGAL
LICENSE NUMBER 46684

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CLECO

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DATE: 4-6-2026	ENGR: AGH
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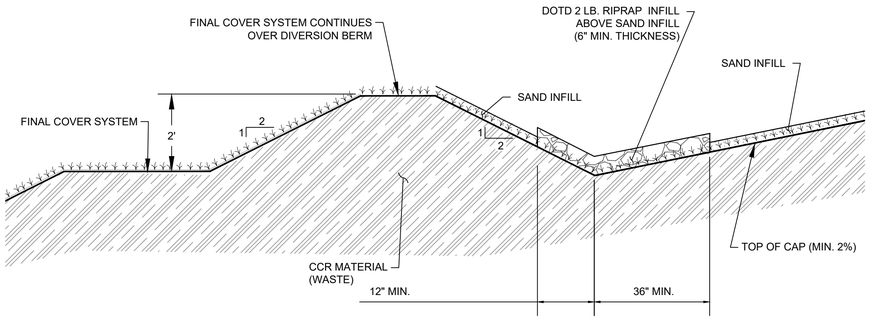
C:\Users\jared.taylor\OneDrive - AECOM\60753972_Cleco_Dolet_LF_IFC - General\900_CAD_GIS\910_CAD\05-SHEETS\015-017 - DETAILS I-III.dwg User: jared.taylor Apr 06, 2026 - 2:34pm



1 TERMINATION TYPE 8

007 | 016 NOT TO SCALE

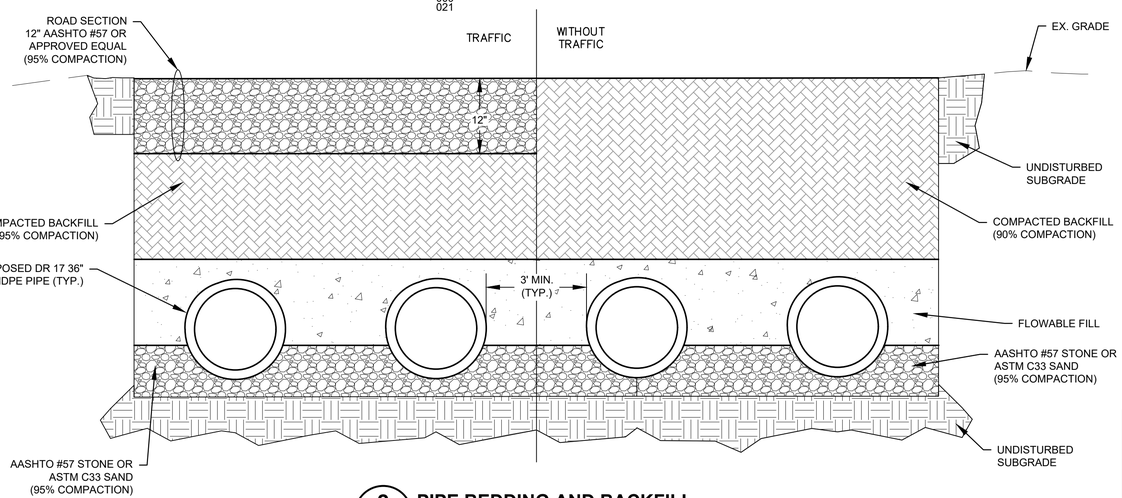
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4 DIVERSION BERM

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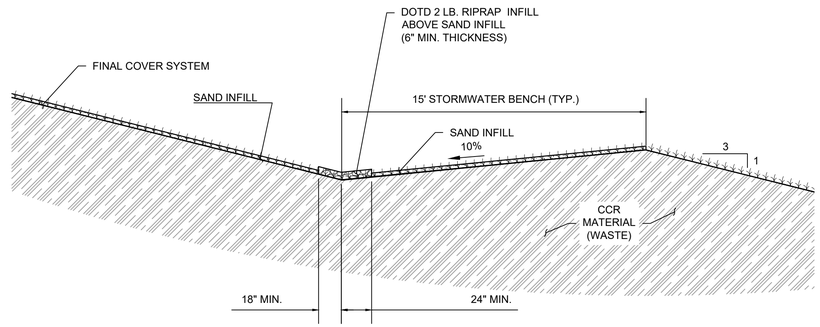
009 020



2 PIPE BEDDING AND BACKFILL

007 | 016 NOT TO SCALE

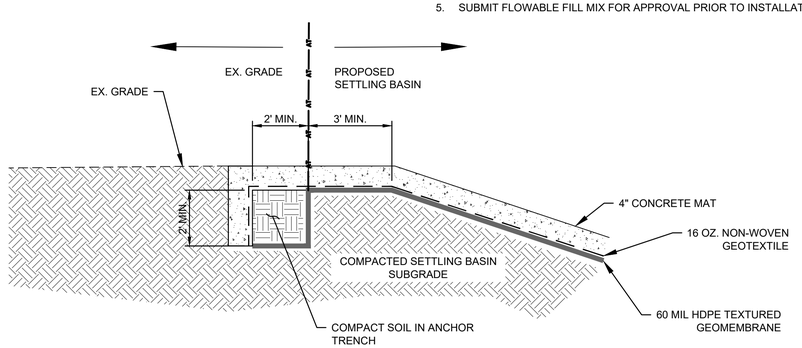
NOTES:
1. PROVIDE MATERIAL SUBMITTALS FOR APPROVAL PRIOR TO INSTALLATION.
2. COMAPCTION NOTATIONS ARE IN ACCORDANCE WITH A STANDARD PROCTOR, +/- 2% OF OPTIMUM MOISTURE.
3. IN NON-TRAFFIC AREAS, COMPACTION EFFORT MAY BE 90% OF STANDARD PROCTOR.
4. SUBMIT PROCTOR RESULTS FOR MATERIALS FOR APPROVAL PRIOR TO INSTALLATION.
5. SUBMIT FLOWABLE FILL MIX FOR APPROVAL PRIOR TO INSTALLATION.



5 STORMWATER BENCH

005 | 016 NOT TO SCALE

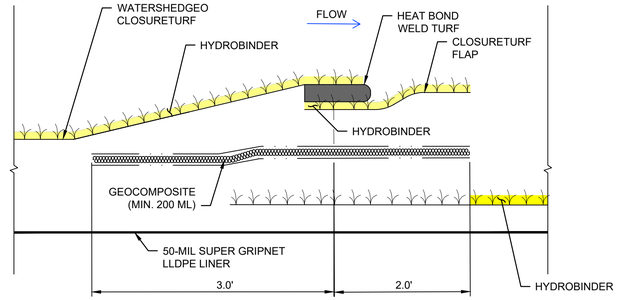
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3 TERMINATION TYPE 10

007 | 016 NOT TO SCALE

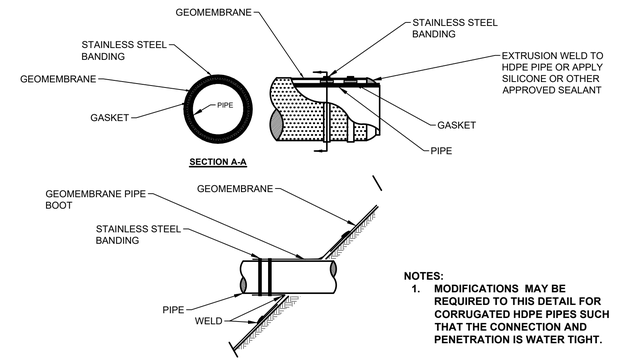
009 021



6 TYP. FINAL COVER WATER RELEASE VENT

005 | 016 NOT TO SCALE

NOTES:
1. **TYPICAL WATER RELEASE VENT FOR FINAL COVER SYSTEM SHALL BE LOCATED AT LEAST 5 FEET UPGRADIENT FROM CONFLUENCE OF DOWNDRAIN AND PERIMETER CHANNEL OR DOWNDRAIN AND BENCHES.**

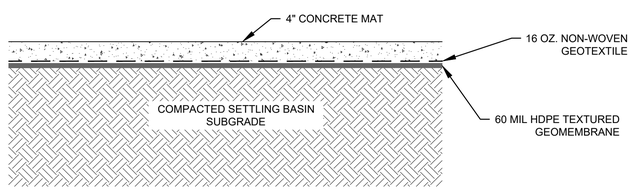


7 BOOTED LINER PENETRATION DETAIL

007 | 016 NOT TO SCALE

021

NOTES:
1. **MODIFICATIONS MAY BE REQUIRED TO THIS DETAIL FOR CORRUGATED HDPE PIPES SUCH THAT THE CONNECTION AND PENETRATION IS WATER TIGHT.**



8 SETTLING BASIN LINER SYSTEM

005 | 016 NOT TO SCALE

007

AECOM

TITLE

DETAILS II

LANDFILL CLOSURE PLANS
DOLET HILLS POWER STATION
MANSFIELD, LOUISIANA

FOR

ISSUED FOR CONSTRUCTION

SEAL

STATE OF LOUISIANA
ANGELA G. HARRIGAL
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LICENSE NUMBER 46684

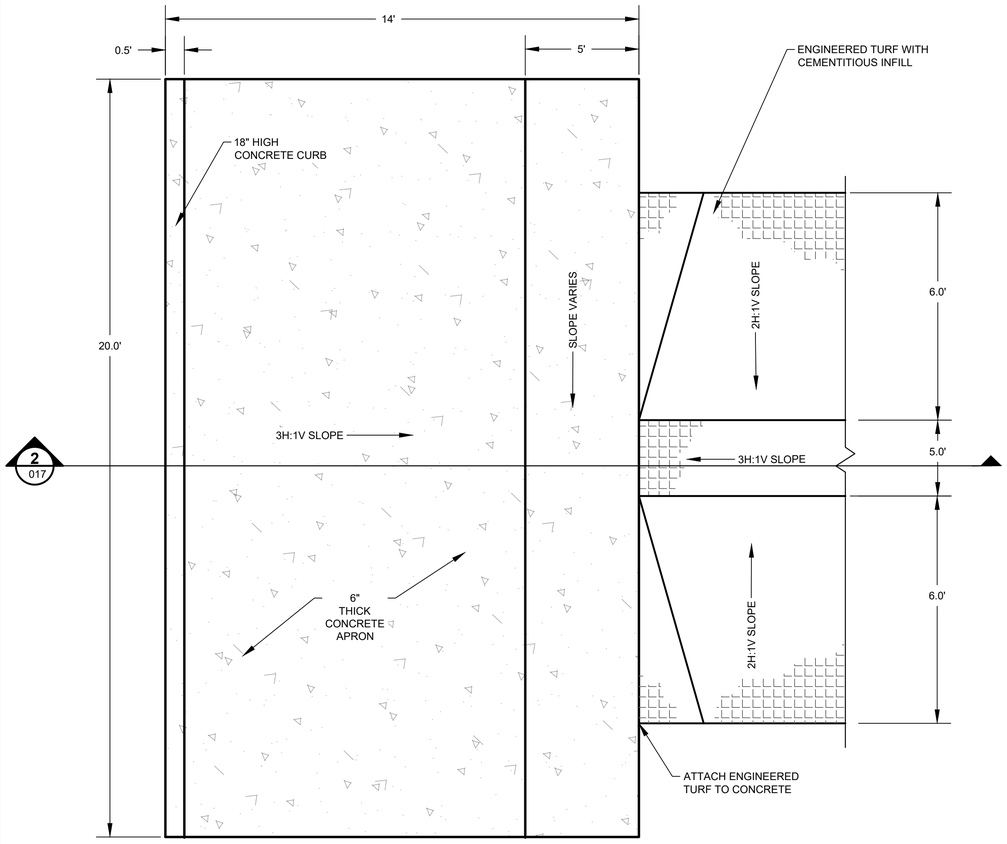
THESE DRAWINGS DO NOT INCLUDE NECESSARY COMPONENTS FOR CONSTRUCTION SAFETY.

CLECO

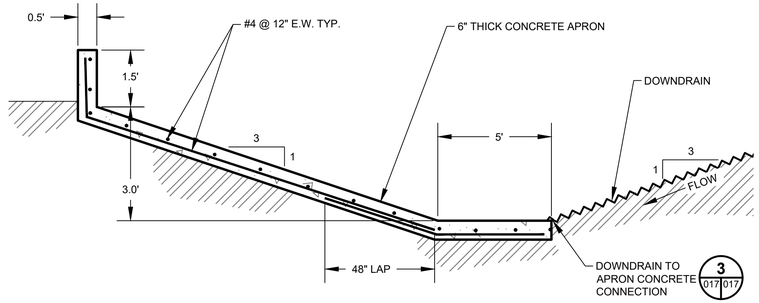
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DATE: 4-6-2026	ENGR: AGH
FILENAME: 015-017 - DETAILS I-III.DWG	APPD: AGH

DWG SIZE	DRAWING NO.	REVISION
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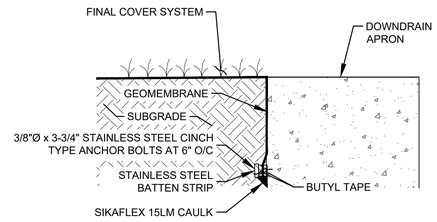
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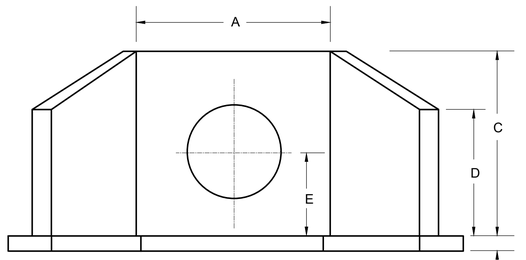
1 005|017 DOWNDRAIN APRON PLAN VIEW
NOT TO SCALE



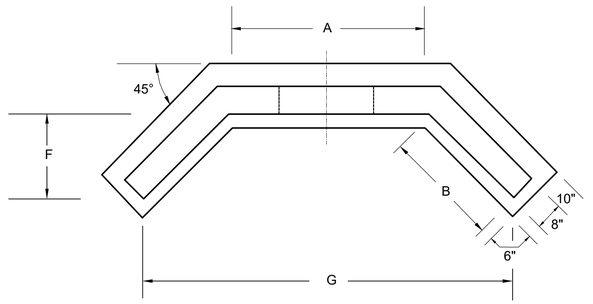
2 017|017 DOWNDRAIN APRON SECTION
NOT TO SCALE



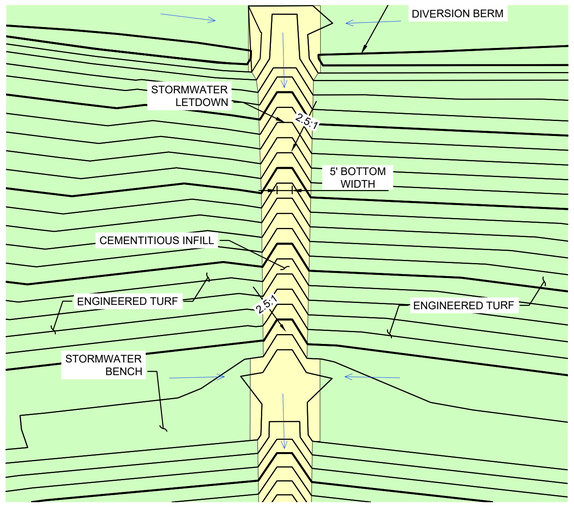
3 017|017 FINAL COVER SYSTEM TO CONCRETE HEADWALL CONNECTION
NOT TO SCALE



ELEVATION

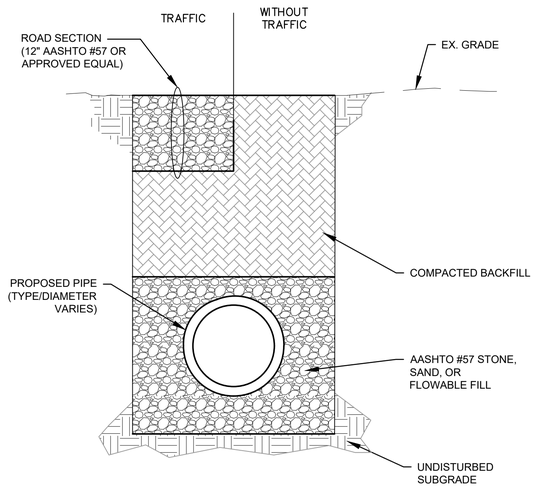


PLAN VIEW



4 009|017 FINAL COVER SYSTEM IN STORMWATER DOWNDRAINS
NOT TO SCALE

NOTES:
1. **TYPICAL WATER RELEASE VENT FOR FINAL COVER SYSTEM SHALL BE LOCATED AT LEAST 5 FEET UPGRADIENT FROM CONFLUENCE OF DOWNDRAIN AND PERIMETER CHANNEL OR DOWNDRAIN AND BENCHES.**



6 020|017 PIPE BEDDING AND BACKFILL
NOT TO SCALE

NOTES:
1. ALL EXPOSED EDGES TO HAVE 1" BEVELED CORNERS.
2. REINFORCING - NO. 4 BARS AT 12" ON CENTER EACH WAY.
3. CONCRETE TO TEST 4500 PSI @ 28 DAYS.
4. PRE-CAST STRUCTURE MAY BE USED, SUBMIT SHOP DRAWINGS FOR APPROVAL.

DIMENSION SCHEDULE								
PIPE DIA.	A	B	C	D	E	F	G	APPROX. WEIGHT
12"	36"	30"	54"	40"	36"	22"	78"	5445 LBS.
15"	48"	30"	60"	45"	37.5"	22"	92"	6400 LBS.
18"	48"	30"	60"	45"	39"	22"	92"	6400 LBS.
24"	60"	36"	64"	48"	42"	26"	112"	7250 LBS.
30"	72"	36"	72"	54"	45"	26"	124"	9276 LBS.
36"	78"	48"	78"	60"	48"	35"	144"	11623 LBS.
48"	84"	48"	90"	66"	54"	35"	150"	12402 LBS.
SPECIAL SIZES NOT LISTED CAN BE MFG. UPON REQUEST								

5 020|017 CONCRETE END SECTION
NOT TO SCALE

AECOM

TITLE
DETAILS III

LANDFILL CLOSURE PLANS
DOLET HILLS POWER STATION
MANSFIELD, LOUISIANA

FOR
ISSUED FOR CONSTRUCTION

SEAL
ANGELA G. HARRIGAL
License No. 46684
STATE OF LOUISIANA
PROFESSIONAL ENGINEER
4/6/2026

8555 United Plaza Blvd., Suite 300
Baton Rouge, Louisiana 70809
225/922-5700

DESIGNER ANGELA G. HARRIGAL
LICENSE NUMBER 46684

THESE DRAWINGS DO NOT INCLUDE NECESSARY COMPONENTS FOR CONSTRUCTION SAFETY.

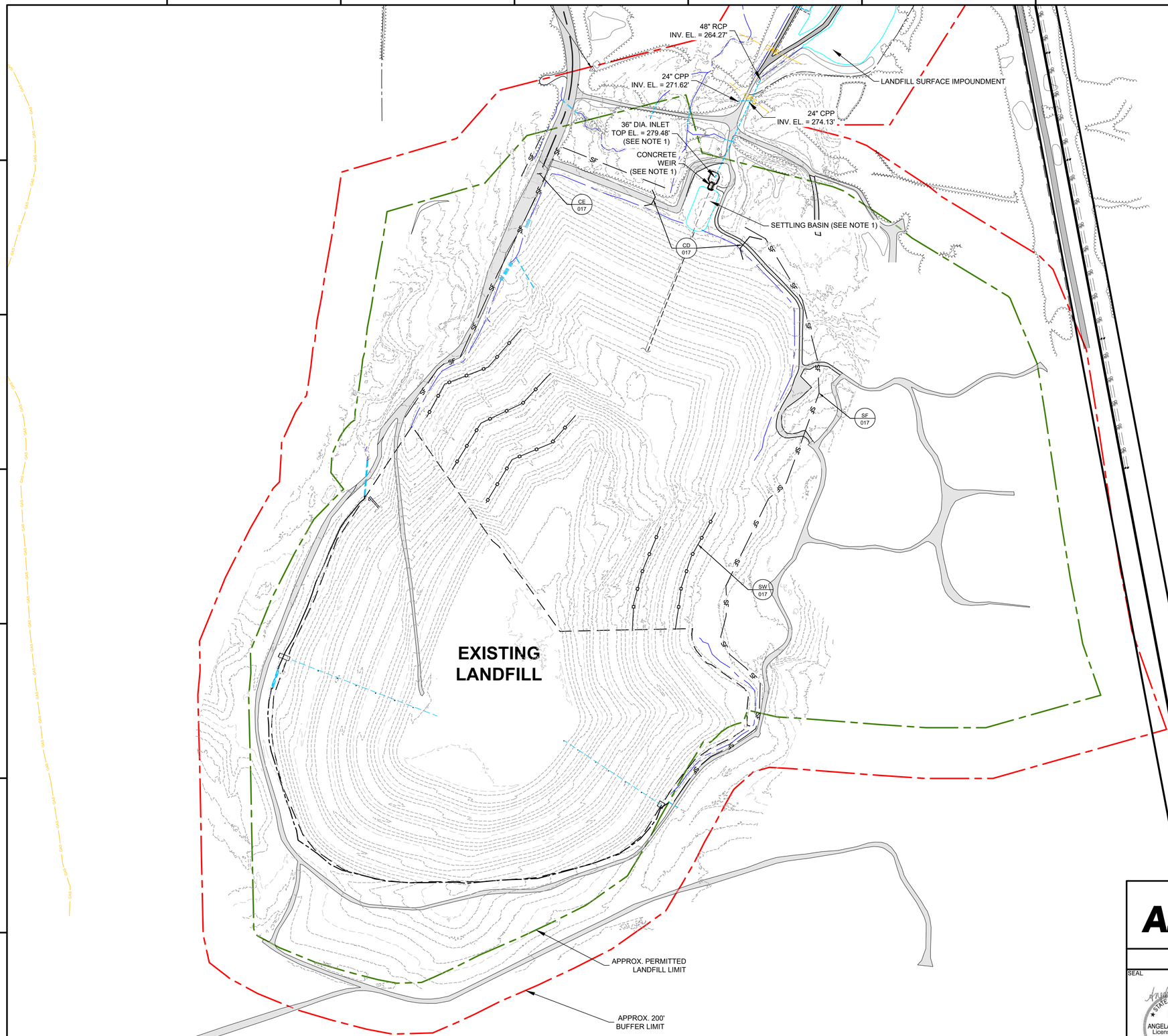
CLECO

SCALE: AS SHOWN	DES: JET
DWG TYPE: .DWG	DFTR: JET
JOB NO: 60753972	CHKD: AGH
DATE: 4-6-2026	ENGR: AGH
FILENAME: 015-017 - DETAILS I-III.DWG	APPD: AGH

DWG SIZE	DRAWING NO.	REVISION
ANSI D 22.0"x34.0"	SHEET 017	5

SHEET 017 REV. 5

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LEGEND

- EXISTING MAJOR CONTOUR (25')
- EXISTING MINOR CONTOUR (5')
- APPROXIMATE PERMITTED LANDFILL LIMIT
- APPROXIMATE 200' BUFFER LIMIT
- DITCH/STREAM CENTERLINE
- OPEN WATER
- UNPAVED ROAD
- CULVERT
- MONITORING WELL
- PIEZOMETER
- SURVEY CONTROL POINT
- SILT FENCE
- STRAW WATTLE
- CHECK DAM

NOTES:

1. EXISTING SETTLING BASIN TO BE EXCAVATED DOWN TO THE PROPOSED GRADES SHOWN ON THE PREVIOUS SHEETS AND THE EXISTING OUTLET RISER SHALL BE REMOVED AND REPLACED WITH A CONCRETE END SECTION, PRIOR TO THE PLACEMENT OF FINAL COVER AND THE INSTALLATION OF THE PIPE FROM THE SOUTHWEST SEEP AREA.
2. ADDITIONAL EROSION AND SEDIMENT CONTROLS MAY BE REQUIRED SUCH THAT STORMWATER IS MANAGED WITHIN THE PROPOSED WORK AREA. DURING OPERATION AND FINAL CLOSURE ALL STORMWATER SHALL BE MANAGED BY THE SETTLING BASIN TO THE NORTH OF THE LANDFILL.

N

0 200 400 600
GRAPHIC SCALE (IN FEET)

AECOM

TITLE
EROSION AND SEDIMENT CONTROL PLAN
LANDFILL CLOSURE PLANS
DOLET HILLS POWER STATION
MANSFIELD, LOUISIANA

FOR
ISSUED FOR CONSTRUCTION

SEAL
STATE OF LOUISIANA
ANGELA G. HARRIGAL
License No. 46684
PROFESSIONAL ENGINEER
4/6/2026

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225/922-5700

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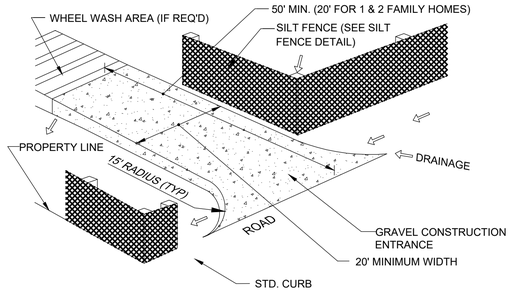
CLECO

SCALE: AS SHOWN	DES: JET
DWG TYPE: .DWG	DFTR: JET
JOB NO: 60753972	CHKD: AGH
DATE: 4-6-2026	ENGR: AGH
FILENAME: 018 - E&S CONTROL PLAN.DWG	APPD: AGH

DWG SIZE	DRAWING NO.	REVISION
ANSI D 22.0"x34.0"	SHEET 018	5

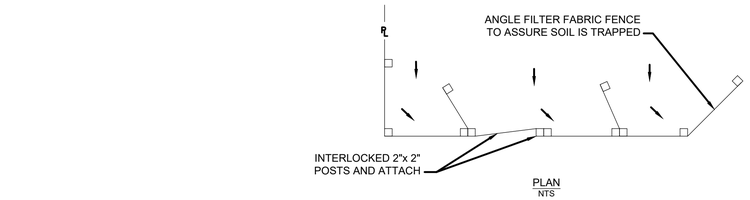
SHEET 018 REV. 5

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CE GRAVEL CONSTRUCTION ENTRANCE
016 017 NOT TO SCALE

NOTE:
1. CONSTRUCTION ENTRANCE TO BE SITUATED NEAR ACTIVE WORK LOCATIONS. CONTRACTOR SHALL FIED VERIFY AND CONFIRM LOCATION WITH OWNER.

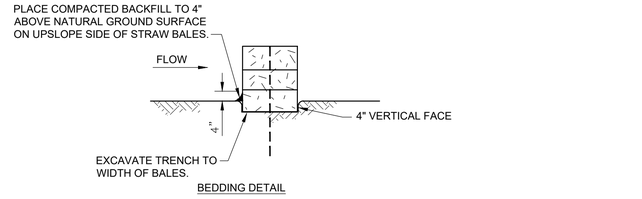


MATERIALS
POSTS: EITHER T OR U TYPE OR 2" X 2" HARDWOOD
FILTER CLOTH: MIRAFI 100X
PREFABRICATED-UNIT: ENVIROFENCE

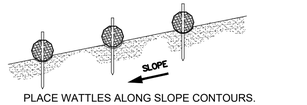
CONSTRUCTION NOTES FOR FABRICATED SILT FENCE

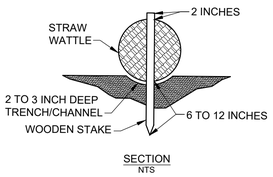
1. THE TOE ANCHOR SHALL BE BACKFILLED AND COMPACTED TO A DENSITY EQUAL TO SURROUNDING SOILS.
2. FILTER CLOTH SHALL BE FASTENED SECURELY TO UPSLOPE SIDE OF FENCE POSTS.
3. WHEN TWO SECTIONS OF FILTER CLOTH ADJOIN EACH OTHER, THEY SHALL BE OVERLAPPED 6" AND STAPLED @ 8" O.C. MAX. DIST.
4. MAINTENANCE SHALL BE PERFORMED AS NEEDED AND MATERIAL REMOVED WHEN BULGES DEVELOP IN THE SILT FENCE.

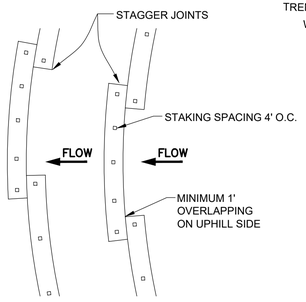
SF TYPICAL SILT FENCE DETAIL
016 017 NOT TO SCALE



SB TYPICAL STRAW BALE DETAIL
016 017 NOT TO SCALE



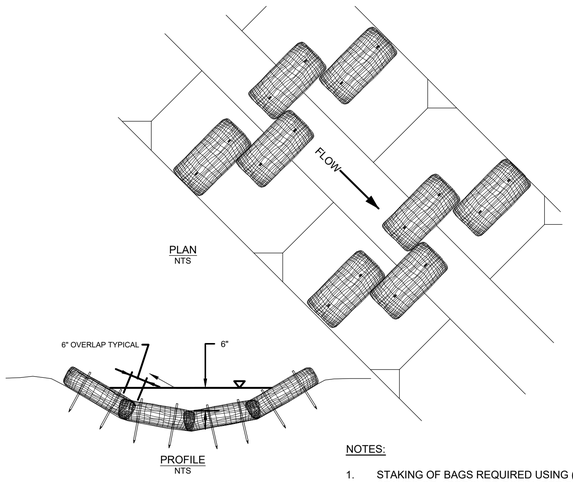




WATTLE CONSTRUCTION SPECIFICATIONS

1. PREPARE THE SLOPE BEFORE THE WATTLING PROCEDURE IS STARTED.
2. IT IS CRITICAL THAT ROLLS ARE INSTALLED PERPENDICULAR TO WATER MOVEMENT, PARALLEL TO THE SLOPE CONTOUR.
3. LAY THE ROLL ALONG THE TRENCHES FITTING IT SNUGLY AGAINST THE SOIL. MAKE SURE NO GAPS EXIST BETWEEN THE SOIL AND THE STRAW WATTLE.
4. USE A STRAIGHT BAR TO DRIVE HOLES THROUGH THE WATTLE AND INTO THE SOIL FOR THE WILLOW OR WOODEN STAKES.
5. INSTALL STAKES AT LEAST EVERY 4 FEET APART THROUGH THE WATTLE.

SW STRAW WATTLE DETAIL
016 017 NOT TO SCALE



NOTES:

1. STAKING OF BAGS REQUIRED USING (2) 1"X2" WOOD STAKES OR APPROVED EQUAL PER BAG.
2. SURFACE MUST BE SMOOTH BEFORE APPLICATION.
3. CHECK DAMS CAN BE CONSTRUCTED USING STRAW WATTLES OR OTHER MATERIALS AS APPROVED BY THE DISTRICT OR CITY.

CD CHECK DAM BIO-FILTER BAG
016 017 NOT TO SCALE

AECOM

TITLE
EROSION AND SEDIMENT CONTROL DETAILS
LANDFILL CLOSURE PLANS
DOLET HILLS POWER STATION
MANSFIELD, LOUISIANA

FOR
ISSUED FOR CONSTRUCTION

SEAL
STATE OF LOUISIANA
ANGELA G. HARRIGAL
License No. 46684
PROFESSIONAL ENGINEER
4/6/2026

8555 United Plaza Blvd., Suite 300
Baton Rouge, Louisiana 70809
225/922-5700

DESIGNER ANGELA G. HARRIGAL
LICENSE NUMBER 46684

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CLECO

SCALE: AS SHOWN	DES: JET
DWG TYPE:.DWG	DFTR: JET
JOB NO: 60753972	CHKD: AGH
DATE: 4-6-2026	ENGR: AGH
FILENAME: 019 - E&S CONTROL DETAILS.DWG	APPD: AGH

DWG SIZE	DRAWING NO.	REVISION
ANSI D 22.0"x34.0"	SHEET 019	5

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PROP. (2) 36" HDPE CULVERTS
PIPE INVERTS TO MATCH EX.
36" CULVERT

PROP. CONCRETE
MAT LINING

PROP. LA DOTD 10 LB. RIPRAP
OUTLET PROTECTION
30' UPSTREAM WIDTH
50' DOWNSTREAM WIDTH
40' LONG
18" THICK MIN.

7 020|016 EX. 6" DRAIN LINE
(TO BE EXTENDED
INTO PROPOSED
SEDIMENT BASIN)

1 TO 6 020|015 CENTERLINE OF PROP.
PERIMETER DITCH

PROP. LA DOTD 55 LB.
RIPRAP OUTLET
PROTECTION
18" THICK MIN.
20' WIDE X 20' LONG MIN.

RUNOFF
COLLECTION
POND

PROP. LA DOTD 10 LB.
RIPRAP OUTLET
PROTECTION
18" THICK MIN.
40' WIDE X 40' LONG MIN.

EX. 36" CULVERT
TO BE REPLACED WITH
(2) 36" CULVERTS

EX. 48" CULVERT
APPROX. INV. 268.4'
(EX. RISER TO BE REMOVED)

7 020|016 5 020|017

CENTERLINE OF PROP.
UPHILL EASTERN DITCH

EX. LANDFILL RUNOFF
SUMP TO BE EXCAVATED AND
CONVERTED TO STORMWATER
DETENTION BASIN

PROP. LA DOTD 10 LB.
RIPRAP OUTLET
PROTECTION
18" THICK MIN.
10' WIDE X 30' LONG MIN.

CENTERLINE OF PROP.
PERIMETER DITCH

1 TO 6 020|015

STORMWATER
BENCH (TYP)

5 020|016

DOWNDRAIN (TYP)

DOWNDRAIN (TYP) 9 020|015

PROP. CLOSURE
TURF LIMIT - WEST

4 020|016 TYPICAL DIVERSION BERM
ON TOP OF LANDFILL

CENTERLINE OF PROP.
UPHILL EASTERN DITCH

LIMITS OF EXISTING LANDFILL
CAPPED AREA

PROP. CLOSURE
TURF LIMIT - EAST

P-11
378.46

EX. LANDFILL LETDOWN
OUTFALL STRUCTURE

P-12
378.45

EX. LANDFILL LETDOWN
ELEV. = 491.89'

EX. LANDFILL LETDOWN
ELEV. = 495.27'

P-5
382.07

MW-1B
382.06

P-2
362.16

P-3
361.68

EX. LANDFILL LETDOWN
OUTFALL STRUCTURE

APPROX. PERMITTED
LANDFILL LIMIT

P-1
367.16

MW-2A
367.20

P-13
367.17

APPROX. 200'
BUFFER LIMIT

LEGEND

- EXISTING MAJOR CONTOUR (25')
- EXISTING HISTORIC MINOR CONTOUR (5')
- PROPOSED CONTOUR (25')
- PROPOSED CONTOUR (5')
- UNPAVED ROAD
- APPROXIMATE PERMITTED LANDFILL LIMIT
- APPROXIMATE 200' BUFFER LIMIT
- LIMITS OF EXISTING LANDFILL CAPPED AREA
- CENTERLINE OF EXISTING PERIMETER DITCH
- DITCH/STREAM CENTERLINE
- OPEN WATER
- EXISTING CULVERT
- PROPOSED CULVERT
- MONITORING WELL
- PIEZOMETER
- SURVEY CONTROL POINT
- RUNOFF DIRECTION
- RIPRAP (TYP.)
- CONCRETE MAT LINING (TYP.)

N

0 200 400 600
GRAPHIC SCALE (IN FEET)

0 INCHES 1 2 3 TENTHS 10 20 30

C:\Users\jared.taylor\OneDrive - AECOM\60753972_Cleco_Dolet_LF_IFC - General\900_CAD_GIS\910_CAD\05-SHEETS\020 - STORMWATER MANAGEMENT PLAN.dwg User: jared.taylor Apr 06, 2026 - 2:40pm

AECOM

TITLE
STORMWATER MANAGEMENT PLAN

LANDFILL CLOSURE PLANS
DOLET HILLS POWER STATION
MANSFIELD, LOUISIANA

FOR
ISSUED FOR CONSTRUCTION

SEAL
ANGELA G. HARRIGAL
STATE OF LOUISIANA
License No. 46684
PROFESSIONAL ENGINEER
4/6/2026

8555 United Plaza Blvd., Suite 300
Baton Rouge, Louisiana 70809
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DESIGNER ANGELA G. HARRIGAL
LICENSE NUMBER 46684

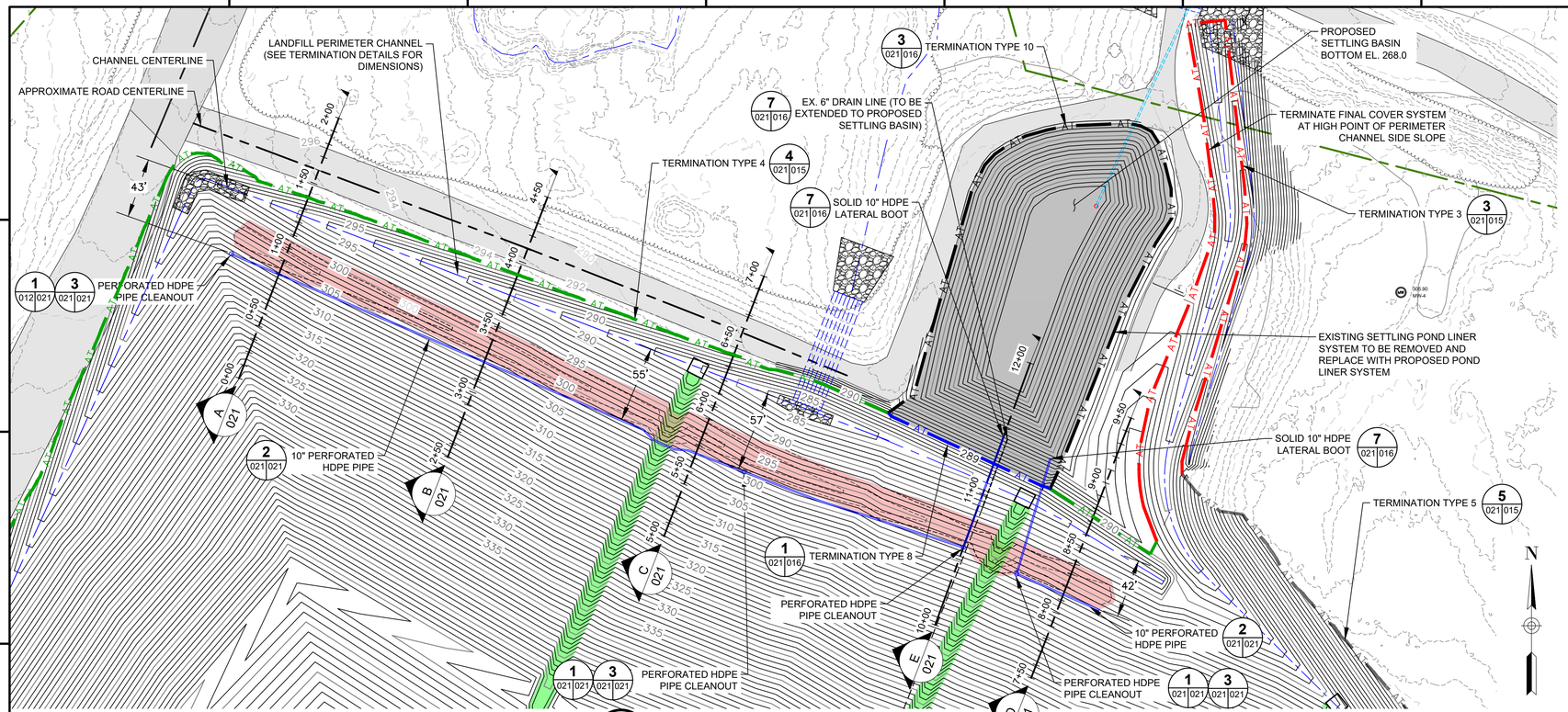
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CLECO

SCALE: AS SHOWN	DES: JET
DWG TYPE: .DWG	DFTR: JET
JOB NO: 60753972	CHKD: AGH
DATE: 4-6-2026	ENGR: AGH
FILENAME: 020 - STORMWATER MANAGEMENT PLAN.DWG	APPD: AGH

DWG SIZE	DRAWING NO.	REVISION
ANSI D 22.0"x34.0"	SHEET 020	5

SHEET 020 REV. 5



SEEP PLAN

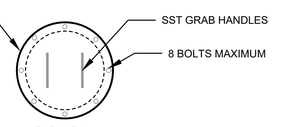
1" = 50'-0" 0' 50' 100'

LEGEND

- EXISTING MAJOR CONTOUR (10')
- EXISTING HISTORIC MINOR CONTOUR (2')
- PROPOSED TOP OF WASTE MAJOR CONTOUR (10')
- PROPOSED TOP OF WASTE MINOR CONTOUR (2')
- UNPAVED ROAD
- APPROXIMATE PERMITTED LANDFILL LIMIT
- APPROXIMATE 200' BUFFER LIMIT
- CHANNEL/STREAM CENTERLINE
- OPEN WATER
- EXISTING CULVERT
- PROPOSED CULVERT
- DOWNDRAIN
- CONCRETE MAT
- APPROXIMATE ROAD CENTERLINE
- 10" PERFORATED HDPE PIPE
- TOP OF CLAY LINER ASBUILT (PROVIDED BY PICKETT JAN. 2025)
- MONITORING WELL
- PIEZOMETER
- SURVEY CONTROL POINT
- PROPOSED CLAY TERMINATION BERM

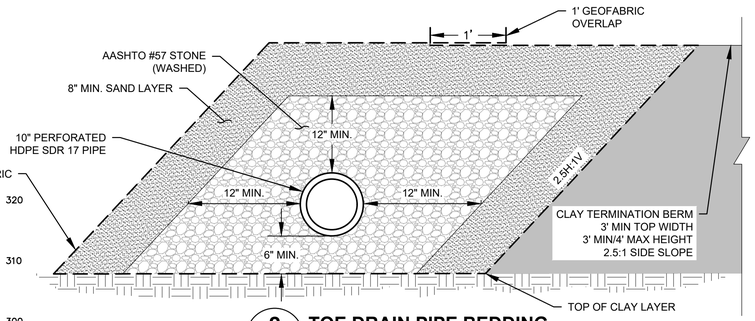
ANCHOR TRENCH TERMINATIONS

- TERMINATION TYPE 1
- TERMINATION TYPE 2
- TERMINATION TYPE 3
- TERMINATION TYPE 4
- TERMINATION TYPE 5
- TERMINATION TYPE 6
- TERMINATION TYPE 7
- TERMINATION TYPE 8
- (NOT USED) TERMINATION TYPE 9
- TERMINATION TYPE 10

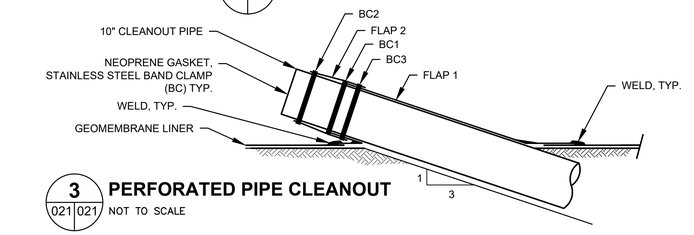


1 PIPE CLEANOUT COVER
021 021 NOT TO SCALE

NOTES:
1. ALL HARDWARE SHALL BE 316 SST.
2. INSTALL MIN. 1/2" TK. NEOPRENE GASKET BEHIND PLATE. HAND TIGHTEN BOLTS TO COMPRESS GASKET.

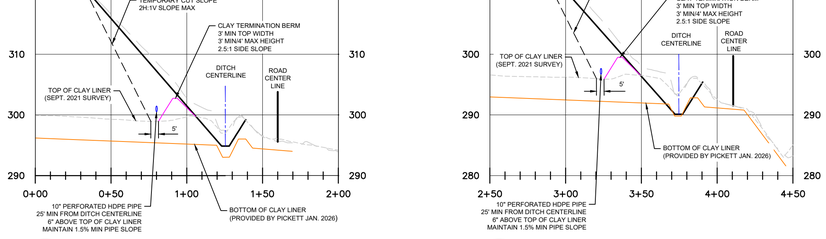


2 TOE DRAIN PIPE BEDDING
021 021 NOT TO SCALE



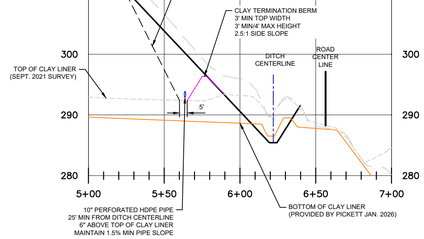
3 PERFORATED PIPE CLEANOUT
021 021 NOT TO SCALE

NOTES:
1. PLACE BAND CLAMPS (BC) 1 AND 2 TO SECURE HDPE FLAP 1 TO PIPE. THEN INSTALL HDPE FLAP 2 IN OPPOSITE DIRECTION OVERTOP BC1 AND BC2. PLACE BC 3 TO SECURE HDPE FLAP 2.
2. SILICON CAULK, OR SIMILAR, SHALL BE USED AT END OF HDPE FLAPS ON OUTSIDE OF ALL BAND CLAMPS AND BETWEEN HDPE FLAPS 1 AND 2.

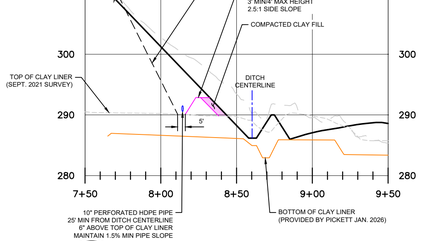


A 021 TYPICAL SEEP REPAIR

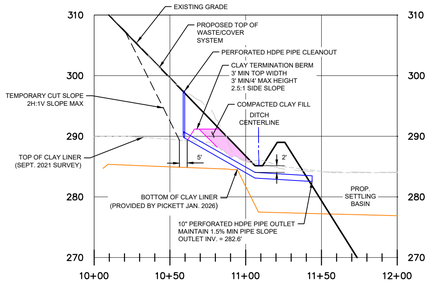
B 021 TYPICAL SEEP REPAIR



C 021 TYPICAL SEEP REPAIR



D 021 TYPICAL SEEP REPAIR



E 021 TOE DRAIN OUTLET (TYP.)

1" = 50'-0" 0' 50' 100'
HORIZONTAL SCALE

1" = 12'-6" 0' 12.5' 25'
VERTICAL SCALE

REV	DATE	JOB NO.	PROJECT TYPE	DES	DFTR	CHKD	ENG	APPD	DESCRIPTION
B1	2-20-2026	60753972	LANDFILL CLOSURE	AGH	AGH	VKG	AGH	AGH	BULLETIN 1 - TOE DRAIN
2	3-27-2026	60753972	LANDFILL CLOSURE	JET	JET	AGH	AGH	AGH	BULLETIN 2 - NORTH DITCH & SEDIMENT BASIN
3	4-6-2026	60753972	LANDFILL CLOSURE	JET	JET	AGH	AGH	AGH	BULLETIN 2 - NORTH DITCH & SEDIMENT BASIN

AECOM

TITLE
TOE DRAIN
LANDFILL CLOSURE PLANS
DOLET HILLS POWER STATION
MANSFIELD, LOUISIANA

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License No. 46684
PROFESSIONAL ENGINEER
4/6/2026

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DESIGNER ANGELA G. HARRIGAL
LICENSE NUMBER 46684

THESE DRAWINGS DO NOT INCLUDE NECESSARY COMPONENTS FOR CONSTRUCTION SAFETY.

CLECO

SCALE: AS SHOWN	DES: JET
DWG TYPE:.DWG	DFTR: JET
JOB NO: 60753972	CHKD: AGH
DATE: 4-6-2026	ENGR: AGH
FILENAME: SEEP REMEDIATION DESIGN 2.DWG	APPD: AGH

DWG SIZE	DRAWING NO.	REVISION
ANSI D 22.0"x34.0"	SHEET 021	3

C:\Users\jared.taylor\OneDrive - AECOM\General - CLECO Dolet Demolition Project\602-Construction Support - LF Closure\605-O&M\Seep Remediation Design\Seep Remediation Design 2.dwg User: jared.taylor Apr 06, 2026 - 2:44pm

APPENDIX C – ENGINEERING CALCULATIONS

WORKSHEET TITLE:	Run-off / Run-off - Dolet Hills Landfill	CALCULATION NO.:	202248 - C - 002
CREATED:	9/2/2026	REVISION:	A
PERFORMED BY:	A. MYERS	REVIEWED BY:	J. Eichenberger
OBJECTIVE:	Determine adequacy of existing run-on and run-off controls		

REFERENCES:

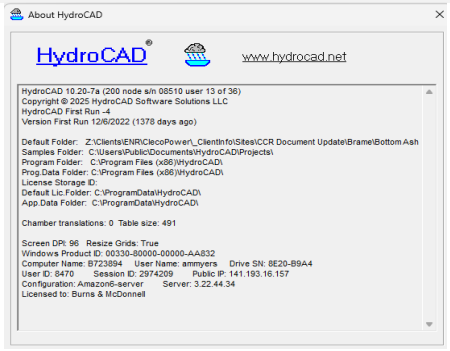
1 Natural Resources Conservation Service (June 1986). TR-55: Urban Hydrology for Small Watersheds.
Retrieved from : www.nrcs.usda.gov/Internet/FSE_DOCUMENTS/stelprdb1044171.pdf

2 US Department of Agriculture. (no date). Custom soils resouces report for DeSoto Parish, LA.
Retrieved from : http://websoilsurvey.nrcs.usda.gov/app/WebSoilSurvey.aspx

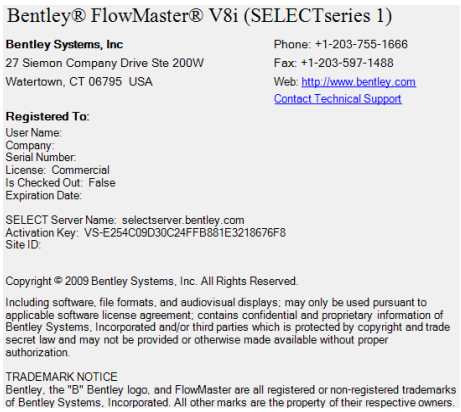
3 National Oceanic and Atmospheric Administration. (2021). NOAA Atlas 14, Volume 9, Version 2. [Point precipitation frequency estimates for Mansfield, LA, Station Mansfield (16-5874), US]. Retrieved from http://hdsc.nws.noaa.gov/hdsc/pfds/pfds_map_cont.html?bkmrk=la

SOFTWARE:

1



2



ASSUMPTIONS:

1 Design storm is 25-yr, 24-hr (per CCR Rule)

2 Soils (existing and cover soils) are generally fine sandy loam, Reference 2
Hydrologic Soil Group D

3 The Curve Number for artificial turf is assumed to be 95.

CALCULATE PEAK DISCHARGE AND CHECK POND CAPACITIES:

1 Establish drainage areas - see SK-CIVIL-001 in Appendix A and HydroCAD Report in Appendix B. Areas meaured in Civil 3D.

2 Establish rainfall data (assume SCS Type III distribution)

SCS Storm	Depth (in)
25yr, 24hr	8.16

Reference 3

3 Establish CN and Time of Concentration - see HydroCAD Report.

4 Establish stage-storage volume for ponds - see HydroCAD Report.

5 Run HydroCAD with input parameters.

RESULTS:

See HydroCAD report.

CONCLUSION:

Under the modeled conditions, the landfill ponds can accept and control inflows from the design flood event without overtopping.

CHECK DITCH CAPACITIES

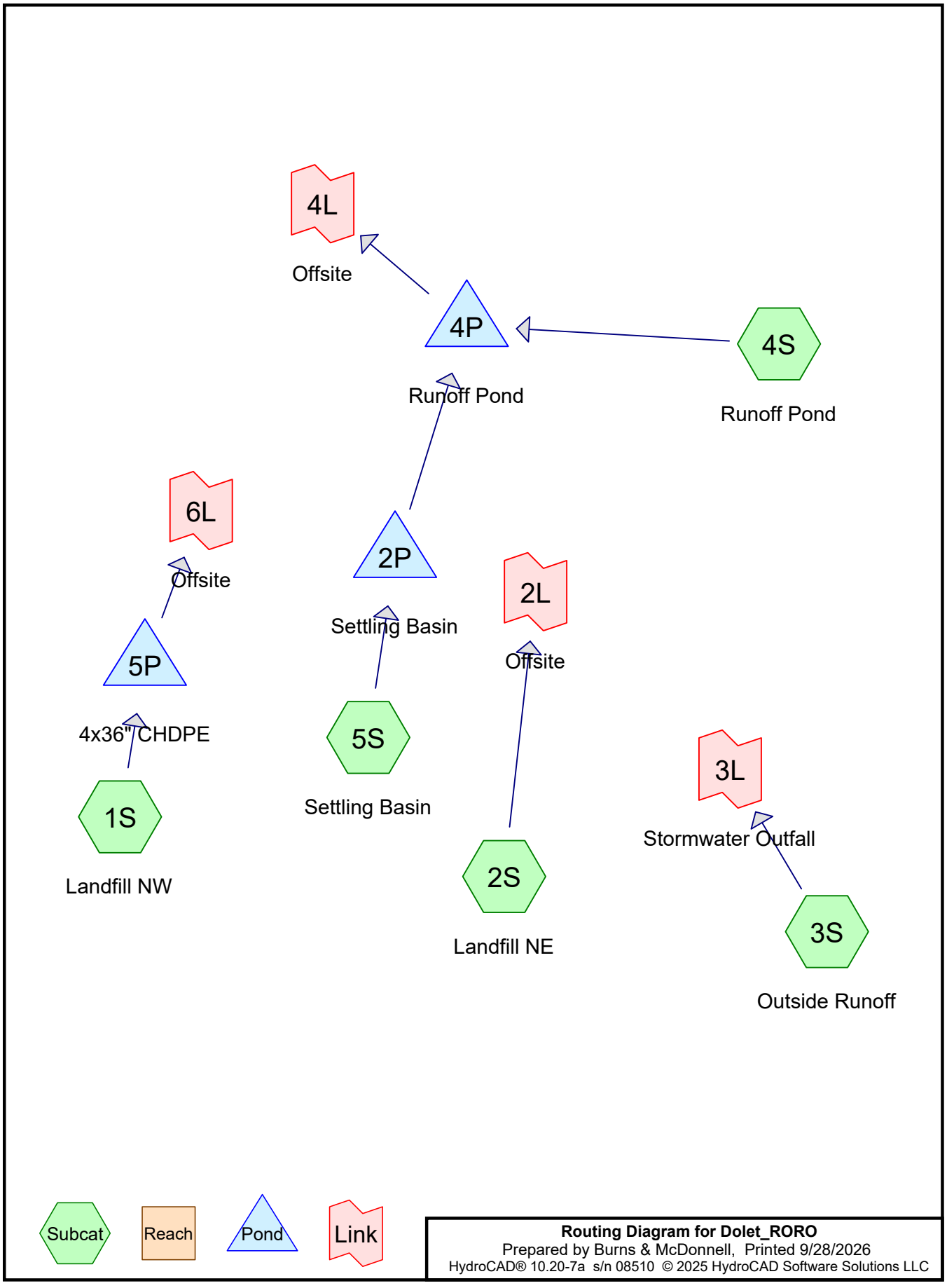
1 Use FlowMaster to determine peak flow depth in the channels using peak discharge calculated from HydroCAD. Minimum channel depth is 2ft.
Landfill Run-off: Assume a trapezoidal channel with roughness coefficient of 0.020 (Hydrobinder), channel slope 5%, bottom width of 5ft, and 3:1 side slopes.
Outside Run-off: Assume a triangular channel with roughness coefficient of 0.025 (earth, clean and winding), channel slope 2.9%, and 3:1 side slopes.

RESULTS:

See FlowMaster worksheets.

CONCLUSION:

Under the modeled conditions, the landfill diversion ditches can accept and control inflows from the design flood event without overtopping.



Routing Diagram for Dolet_RORO
Prepared by Burns & McDonnell, Printed 9/28/2026
HydroCAD® 10.20-7a s/n 08510 © 2025 HydroCAD Software Solutions LLC

Area Listing (all nodes)

Area (acres)	CN	Description (subcatchment-numbers)
18.111	80	>75% Grass cover, Good, HSG D (1S, 2S, 3S, 4S, 5S)
44.401	95	Artificial Turf (1S, 2S, 5S)
0.474	96	Gravel surface, HSG D (1S, 2S, 3S, 4S, 5S)
2.844	98	Water Surface, HSG D (4S)
3.876	77	Woods, Good, HSG D (3S, 4S)
69.706	**90**	**TOTAL AREA**

Time span=0.00-24.00 hrs, dt=0.01 hrs, 2401 points x 3
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment1S: Landfill NW Runoff Area=1,193,723 sf 0.00% Impervious Runoff Depth>7.31"
Flow Length=2,550' Tc=6.8 min CN=93 Runoff=208.48 cfs 16.704 af

Subcatchment2S: Landfill NE Runoff Area=1,230,711 sf 0.00% Impervious Runoff Depth>7.07"
Flow Length=3,180' Tc=11.8 min CN=91 Runoff=180.77 cfs 16.646 af

Subcatchment3S: Outside Runoff Runoff Area=243,941 sf 0.00% Impervious Runoff Depth>5.65"
Flow Length=1,678' Slope=0.0286 '/' Tc=7.3 min CN=79 Runoff=35.02 cfs 2.637 af

Subcatchment4S: Runoff Pond Runoff Area=319,978 sf 38.71% Impervious Runoff Depth>6.31"
Flow Length=217' Tc=147.3 min CN=86 Runoff=11.94 cfs 3.860 af

Subcatchment5S: Settling Basin Runoff Area=48,042 sf 0.00% Impervious Runoff Depth>7.20"
Flow Length=138' Tc=6.0 min CN=92 Runoff=8.56 cfs 0.661 af

Pond 2P: Settling Basin Peak Elev=269.20' Storage=8,390 cf Inflow=8.56 cfs 0.661 af
48.0" Round Culvert n=0.012 L=423.6' S=0.0097 '/' Outflow=5.41 cfs 0.646 af

Pond 4P: Runoff Pond Peak Elev=261.25' Storage=16.057 af Inflow=12.55 cfs 4.506 af
Outflow=0.00 cfs 0.000 af

Pond 5P: 4x36" CHDPE Peak Elev=287.54' Storage=16,159 cf Inflow=208.48 cfs 16.704 af
36.0" Round Culvert x 4.00 n=0.012 L=84.0' S=0.1310 '/' Outflow=187.51 cfs 16.703 af

Link 2L: Offsite Inflow=180.77 cfs 16.646 af
Primary=180.77 cfs 16.646 af

Link 3L: Stormwater Outfall Inflow=35.02 cfs 2.637 af
Primary=35.02 cfs 2.637 af

Link 4L: Offsite Inflow=0.00 cfs 0.000 af
Primary=0.00 cfs 0.000 af

Link 6L: Offsite Inflow=187.51 cfs 16.703 af
Primary=187.51 cfs 16.703 af

Total Runoff Area = 69.706 ac Runoff Volume = 40.508 af Average Runoff Depth = 6.97"
95.92% Pervious = 66.862 ac 4.08% Impervious = 2.844 ac

Summary for Subcatchment 1S: Landfill NW

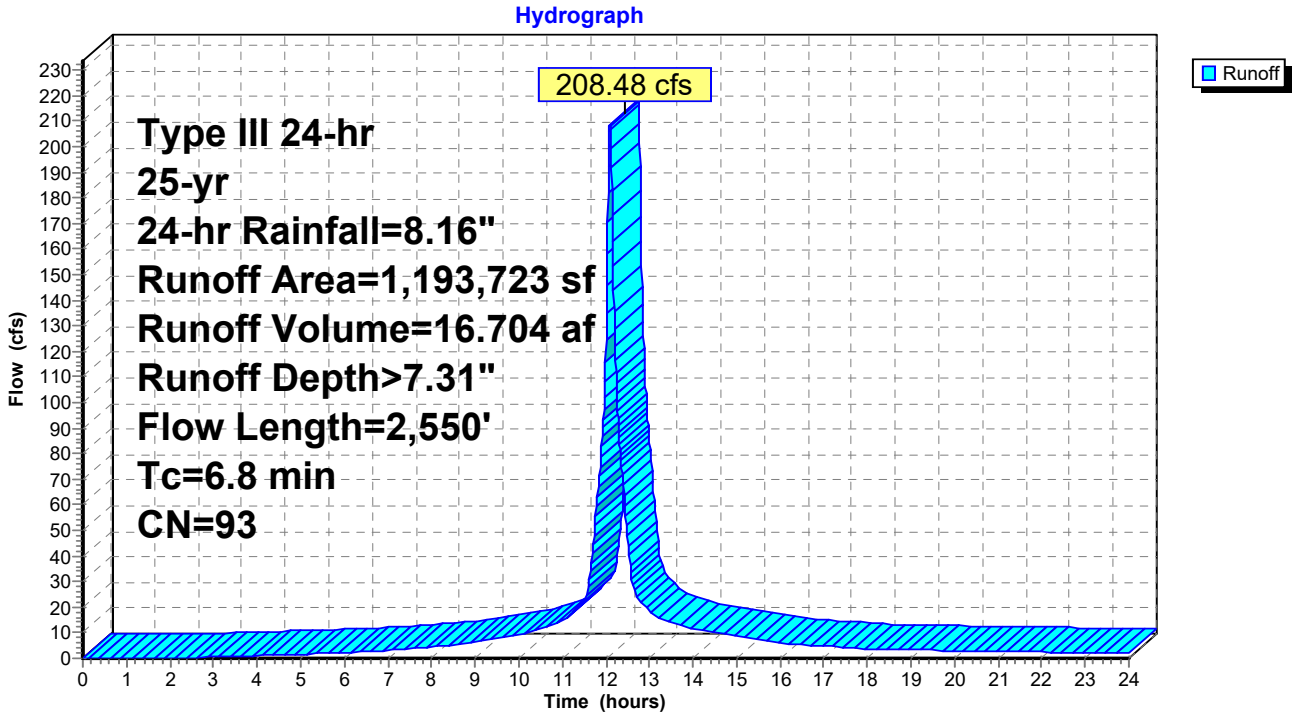
Runoff = 208.48 cfs @ 12.09 hrs, Volume= 16.704 af, Depth> 7.31"
Routed to Pond 5P : 4x36" CHDPE

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs
Type III 24-hr 25-yr, 24-hr Rainfall=8.16"

	Area (sf)	CN	Description
*	1,010,105	95	Artificial Turf
	180,694	80	>75% Grass cover, Good, HSG D
	2,924	96	Gravel surface, HSG D
	1,193,723	93	Weighted Average
	1,193,723		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
2.2	67	0.2500	0.51		**Sheet Flow,** Grass: Short n= 0.150 P2= 4.39"
2.3	756	0.0833	5.56	5.56	**Trap/Vee/Rect Channel Flow,** Bot.W=0.00' D=0.50' Z= 4.0 '/' Top.W=4.00' n= 0.030 Stream, clean & straight
0.2	93	0.0500	6.70	41.18	**Trap/Vee/Rect Channel Flow,** Bot.W=0.00' D=1.00' Z= 0.3 & 12.0 '/' Top.W=12.30' n= 0.030 Stream, clean & straight
2.1	1,634	0.0500	13.18	105.42	**Trap/Vee/Rect Channel Flow,** Bot.W=5.00' D=1.00' Z= 3.0 '/' Top.W=11.00' n= 0.020
6.8	2,550	Total			

Subcatchment 1S: Landfill NW



Summary for Subcatchment 2S: Landfill NE

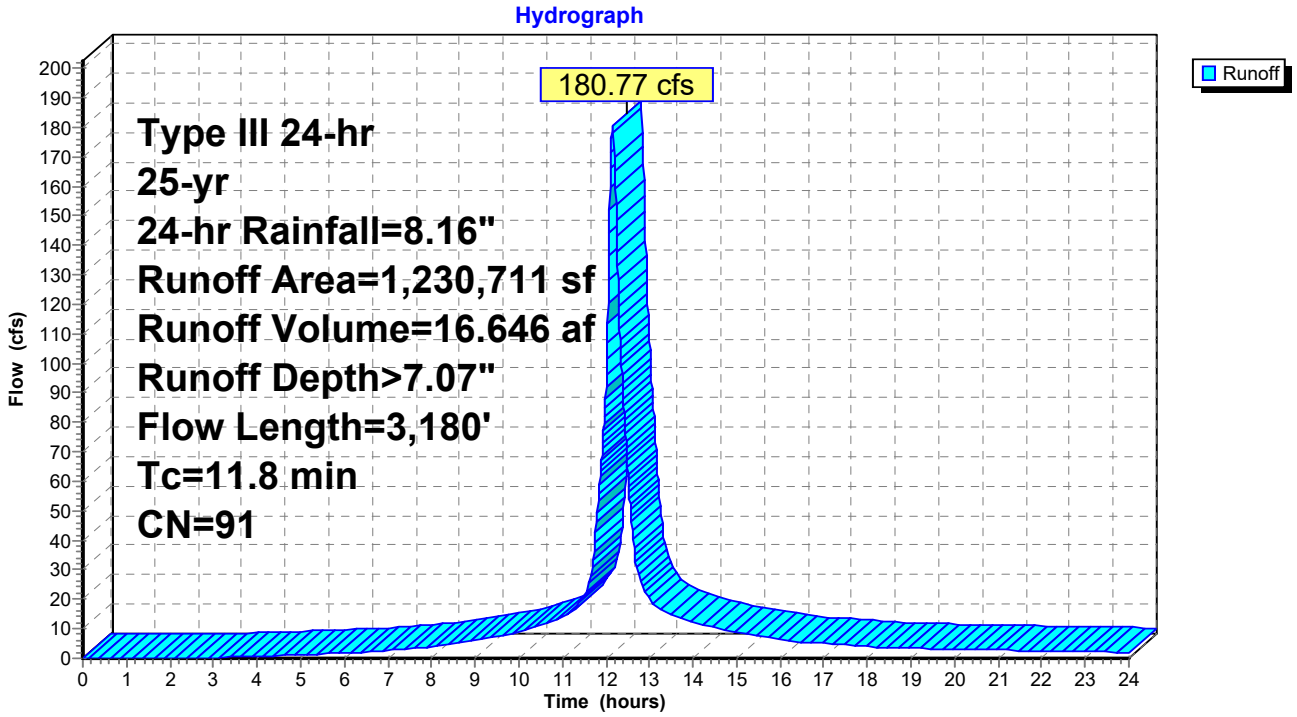
Runoff = 180.77 cfs @ 12.16 hrs, Volume= 16.646 af, Depth> 7.07"
Routed to Link 2L : Offsite

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs
Type III 24-hr 25-yr, 24-hr Rainfall=8.16"

	Area (sf)	CN	Description
*	892,612	95	Artificial Turf
	335,276	80	>75% Grass cover, Good, HSG D
	2,823	96	Gravel surface, HSG D
	1,230,711	91	Weighted Average
	1,230,711		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
2.1	63	0.2500	0.50		**Sheet Flow,** Grass: Short n= 0.150 P2= 4.39"
1.4	23	0.0870	0.27		**Sheet Flow,** Grass: Short n= 0.150 P2= 4.39"
2.2	66	0.2500	0.50		**Sheet Flow,** Grass: Short n= 0.150 P2= 4.39"
0.1	20	0.1500	5.81		**Shallow Concentrated Flow,** Grassed Waterway Kv= 15.0 fps
1.2	438	0.0100	5.89	47.15	**Trap/Vee/Rect Channel Flow,** Bot.W=5.00' D=1.00' Z= 3.0 '/' Top.W=11.00' n= 0.020
0.2	137	0.0280	9.86	78.89	**Trap/Vee/Rect Channel Flow,** Bot.W=5.00' D=1.00' Z= 3.0 '/' Top.W=11.00' n= 0.020
2.7	942	0.0100	5.89	47.15	**Trap/Vee/Rect Channel Flow,** Bot.W=5.00' D=1.00' Z= 3.0 '/' Top.W=11.00' n= 0.020
0.5	517	0.0800	16.67	133.35	**Trap/Vee/Rect Channel Flow,** Bot.W=5.00' D=1.00' Z= 3.0 '/' Top.W=11.00' n= 0.020
0.6	565	0.0740	16.03	128.25	**Trap/Vee/Rect Channel Flow,** Bot.W=5.00' D=1.00' Z= 3.0 '/' Top.W=11.00' n= 0.020
0.8	409	0.0200	8.33	66.68	**Trap/Vee/Rect Channel Flow,** Bot.W=5.00' D=1.00' Z= 3.0 '/' Top.W=11.00' n= 0.020
11.8	3,180	Total			

Subcatchment 2S: Landfill NE



Summary for Subcatchment 3S: Outside Runoff

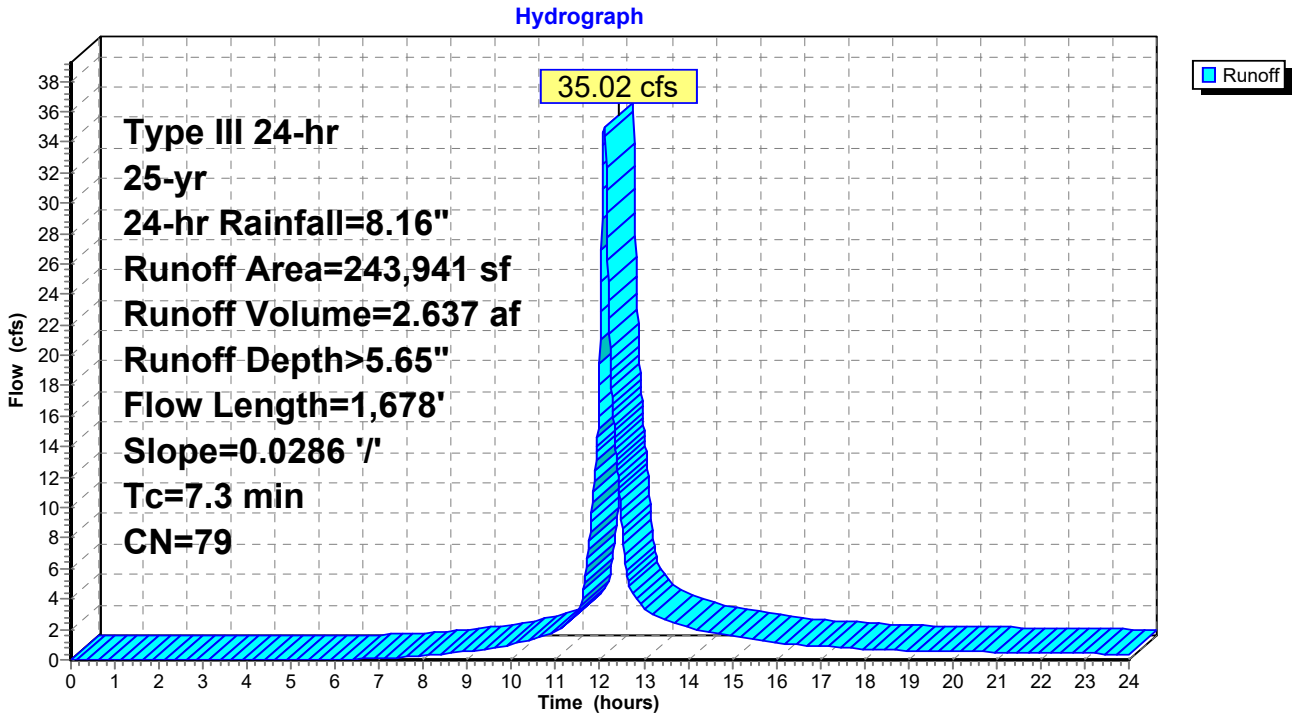
Runoff = 35.02 cfs @ 12.10 hrs, Volume= 2.637 af, Depth> 5.65"
Routed to Link 3L : Stormwater Outfall

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs
Type III 24-hr 25-yr, 24-hr Rainfall=8.16"

Area (sf)	CN	Description
136,736	80	>75% Grass cover, Good, HSG D
100,467	77	Woods, Good, HSG D
6,738	96	Gravel surface, HSG D
243,941	79	Weighted Average
243,941		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
7.3	1,678	0.0286	3.85	2.89	**Trap/Vee/Rect Channel Flow,** Bot.W=0.00' D=0.50' Z= 3.0 '/' Top.W=3.00' n= 0.025 Earth, clean & winding

Subcatchment 3S: Outside Runoff



Summary for Subcatchment 4S: Runoff Pond

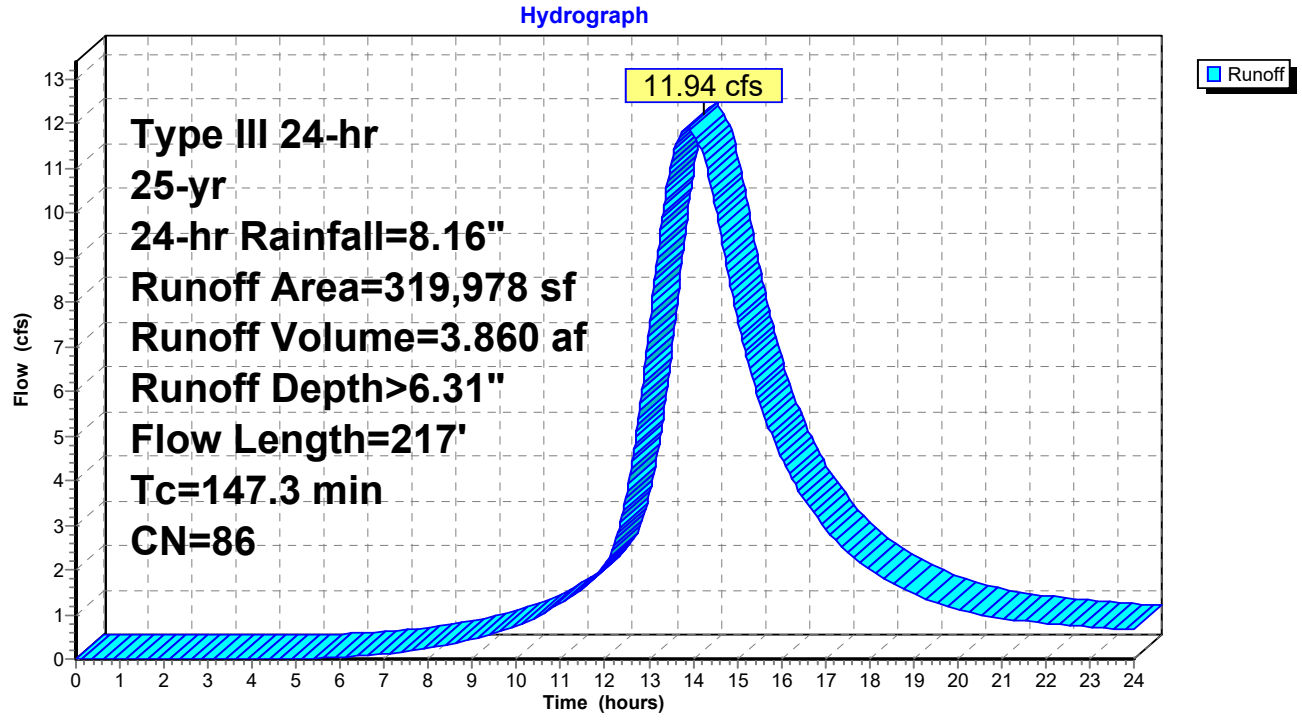
Runoff = 11.94 cfs @ 13.91 hrs, Volume= 3.860 af, Depth> 6.31"
Routed to Pond 4P : Runoff Pond

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs
Type III 24-hr 25-yr, 24-hr Rainfall=8.16"

Area (sf)	CN	Description
125,803	80	>75% Grass cover, Good, HSG D
68,351	77	Woods, Good, HSG D
123,877	98	Water Surface, HSG D
1,947	96	Gravel surface, HSG D
319,978	86	Weighted Average
196,101		61.29% Pervious Area
123,877		38.71% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.9	94	0.2000	0.23		**Sheet Flow,** Woods: Light underbrush n= 0.400 P2= 4.39"
140.3	90	0.0001	0.01		**Sheet Flow,** Woods: Light underbrush n= 0.400 P2= 4.39"
0.1	33	0.3300	4.02		**Shallow Concentrated Flow,** Short Grass Pasture Kv= 7.0 fps
147.3	217	Total			

Subcatchment 4S: Runoff Pond



Summary for Subcatchment 5S: Settling Basin

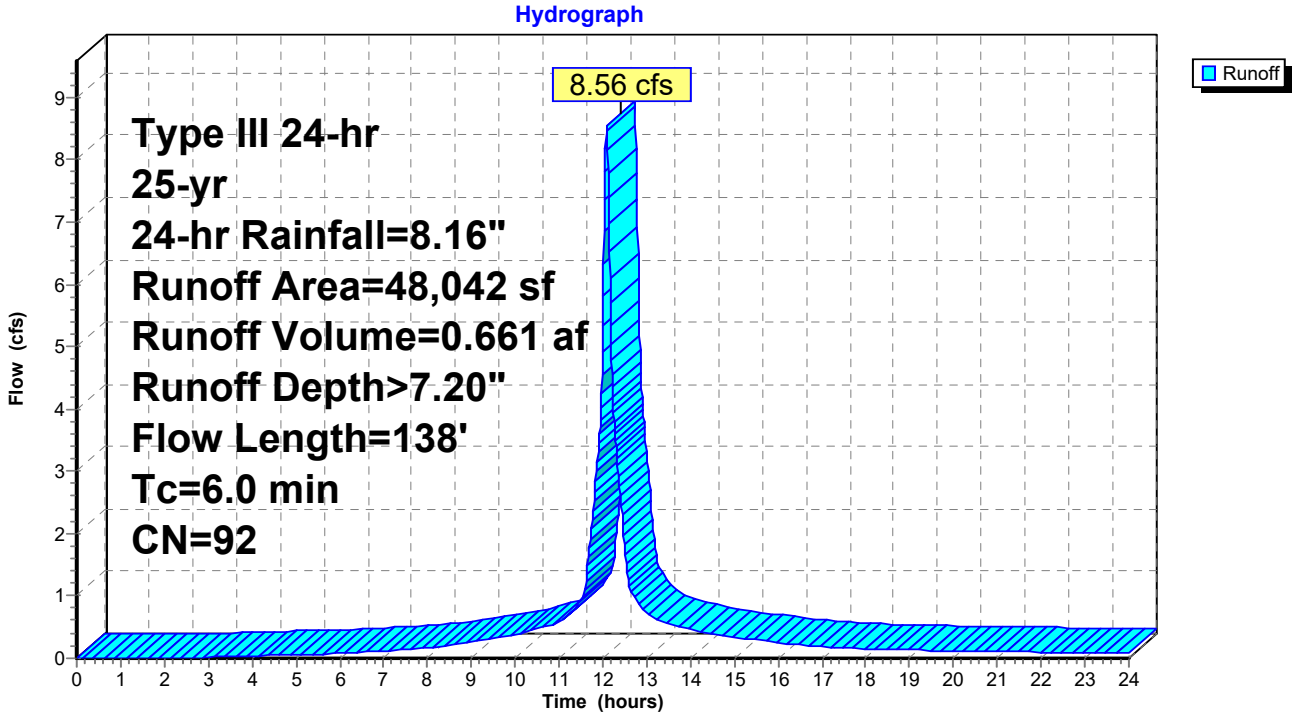
Runoff = 8.56 cfs @ 12.08 hrs, Volume= 0.661 af, Depth> 7.20"
Routed to Pond 2P : Settling Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs
Type III 24-hr 25-yr, 24-hr Rainfall=8.16"

	Area (sf)	CN	Description
*	31,411	95	Artificial Turf
	6,230	96	Gravel surface, HSG D
	10,401	80	>75% Grass cover, Good, HSG D
	48,042	92	Weighted Average
	48,042		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
3.4	82	0.1250	0.40		**Sheet Flow,** Grass: Short n= 0.150 P2= 4.39"
1.7	56	0.3300	0.54		**Sheet Flow,** Grass: Short n= 0.150 P2= 4.39"
5.1	138	Total, Increased to minimum Tc = 6.0 min			

Subcatchment 5S: Settling Basin



Summary for Pond 2P: Settling Basin

Inflow Area = 1.103 ac, 0.00% Impervious, Inflow Depth > 7.20" for 25-yr, 24-hr event
Inflow = 8.56 cfs @ 12.08 hrs, Volume= 0.661 af
Outflow = 5.41 cfs @ 12.18 hrs, Volume= 0.646 af, Atten= 37%, Lag= 5.7 min
Primary = 5.41 cfs @ 12.18 hrs, Volume= 0.646 af
Routed to Pond 4P : Runoff Pond

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3
Starting Elev= 268.40' Surf.Area= 6,733 sf Storage= 2,582 cf
Peak Elev= 269.20' @ 12.18 hrs Surf.Area= 7,841 sf Storage= 8,390 cf (5,808 cf above start)
Flood Elev= 282.00' Surf.Area= 30,364 sf Storage= 241,218 cf (238,636 cf above start)

Plug-Flow detention time= 114.0 min calculated for 0.586 af (89% of inflow)
Center-of-Mass det. time= 34.0 min (802.0 - 768.0)

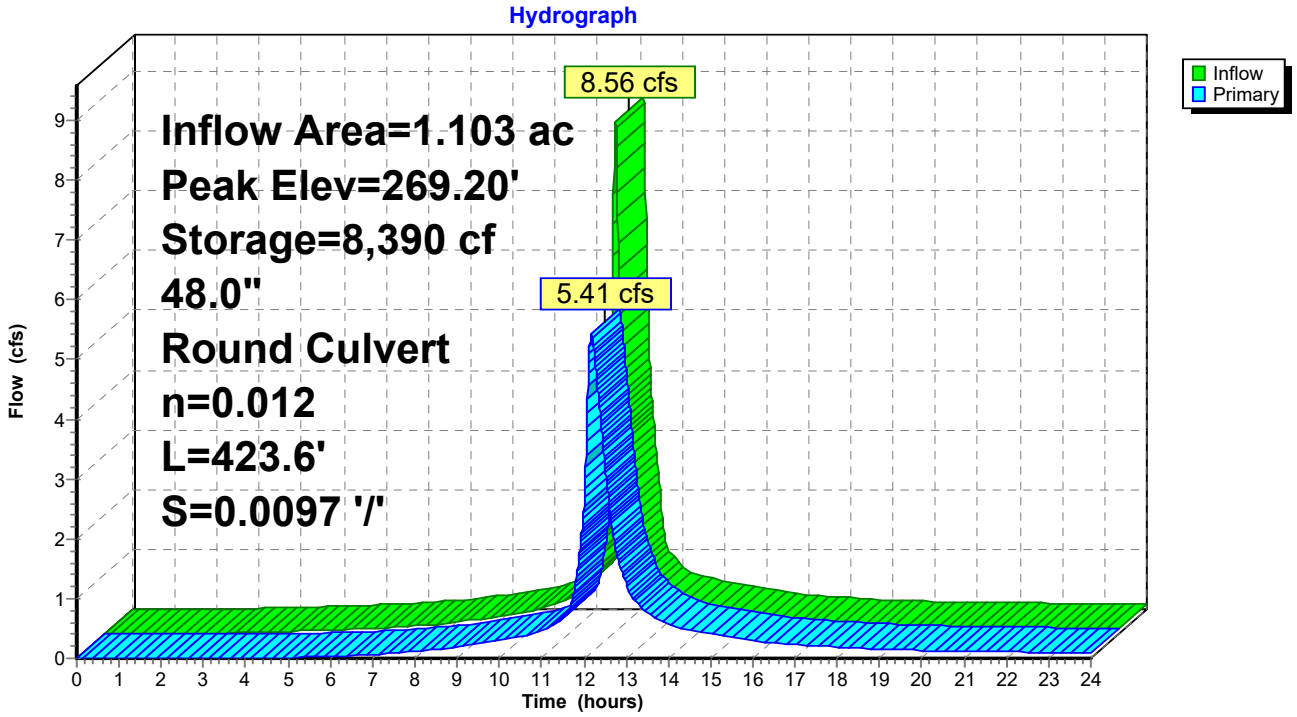
Volume	Invert	Avail.Storage	Storage Description
#1	268.00'	241,218 cf	**Custom Stage Data (Prismatic)** Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
268.00	6,177	0	0
272.00	11,739	35,832	35,832
276.00	18,344	60,166	95,998
280.00	26,056	88,800	184,798
282.00	30,364	56,420	241,218

Device	Routing	Invert	Outlet Devices
#1	Primary	268.40'	**48.0" Round Culvert** L= 423.6' RCP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 268.40' / 264.27' S= 0.0097 '/' Cc= 0.900 n= 0.012 Concrete pipe, finished, Flow Area= 12.57 sf

Primary OutFlow Max=5.41 cfs @ 12.18 hrs HW=269.20' TW=259.31' (Dynamic Tailwater)
1=Culvert (Inlet Controls 5.41 cfs @ 3.04 fps)

Pond 2P: Settling Basin



Summary for Pond 4P: Runoff Pond

Inflow Area = 8.449 ac, 33.66% Impervious, Inflow Depth > 6.40" for 25-yr, 24-hr event
Inflow = 12.55 cfs @ 13.91 hrs, Volume= 4.506 af
Outflow = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af, Atten= 100%, Lag= 0.0 min
Primary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af
Routed to Link 4L : Offsite

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3
Starting Elev= 259.00' Surf.Area= 1.780 ac Storage= 11.551 af
Peak Elev= 261.25' @ 24.00 hrs Surf.Area= 2.359 ac Storage= 16.057 af (4.505 af above start)
Flood Elev= 273.00' Surf.Area= 4.525 ac Storage= 57.824 af (46.273 af above start)

Plug-Flow detention time= (not calculated: initial storage exceeds outflow)
Center-of-Mass det. time= (not calculated: no outflow)

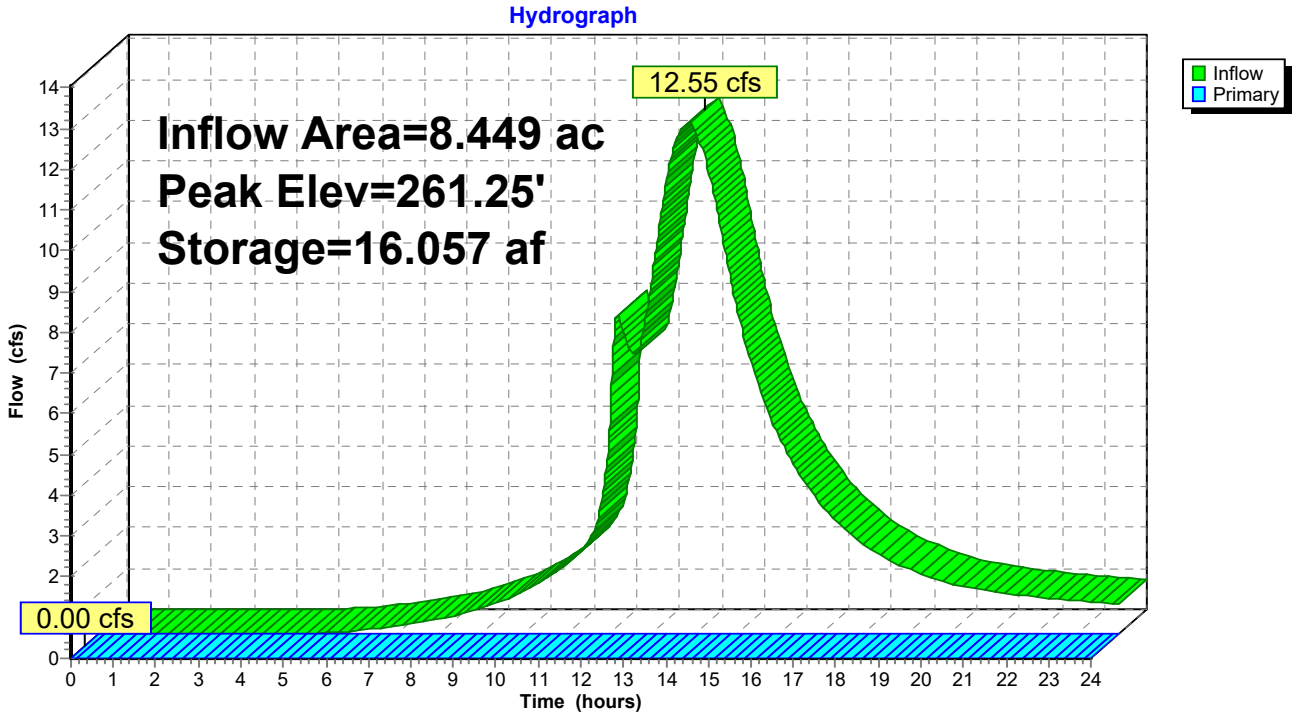
Volume	Invert	Avail.Storage	Storage Description
#1	250.00'	57.824 af	**Custom Stage Data (Prismatic)** Listed below (Recalc)

Elevation (feet)	Surf.Area (acres)	Inc.Store (acre-feet)	Cum.Store (acre-feet)
250.00	0.832	0.000	0.000
252.00	1.016	1.848	1.848
254.00	1.219	2.235	4.083
256.00	1.434	2.653	6.736
258.00	1.661	3.095	9.831
260.00	1.898	3.559	13.390
261.00	2.259	2.078	15.468
262.00	2.652	2.455	17.924
264.00	3.003	5.655	23.579
266.00	3.334	6.337	29.916
268.00	3.716	7.050	36.966
270.00	4.143	7.859	44.825
272.00	4.396	8.539	53.364
273.00	4.525	4.460	57.824

Device	Routing	Invert	Outlet Devices
#1	Primary	270.00'	**Custom Weir/Orifice, Cv= 2.62 (C= 3.28)**
			Head (feet) 0.00 3.00
			Width (feet) 30.00 30.00

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=259.00' TW=0.00' (Dynamic Tailwater)
↑—**1=Custom Weir/Orifice** (Controls 0.00 cfs)

Pond 4P: Runoff Pond



Summary for Pond 5P: 4x36" CHDPE

Inflow Area = 27.404 ac, 0.00% Impervious, Inflow Depth > 7.31" for 25-yr, 24-hr event
Inflow = 208.48 cfs @ 12.09 hrs, Volume= 16.704 af
Outflow = 187.51 cfs @ 12.13 hrs, Volume= 16.703 af, Atten= 10%, Lag= 2.4 min
Primary = 187.51 cfs @ 12.13 hrs, Volume= 16.703 af
Routed to Link 6L : Offsite

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 287.54' @ 12.13 hrs Surf.Area= 8,614 sf Storage= 16,159 cf
Flood Elev= 290.00' Surf.Area= 16,063 sf Storage= 46,466 cf

Plug-Flow detention time= 0.5 min calculated for 16.703 af (100% of inflow)
Center-of-Mass det. time= 0.5 min (765.4 - 765.0)

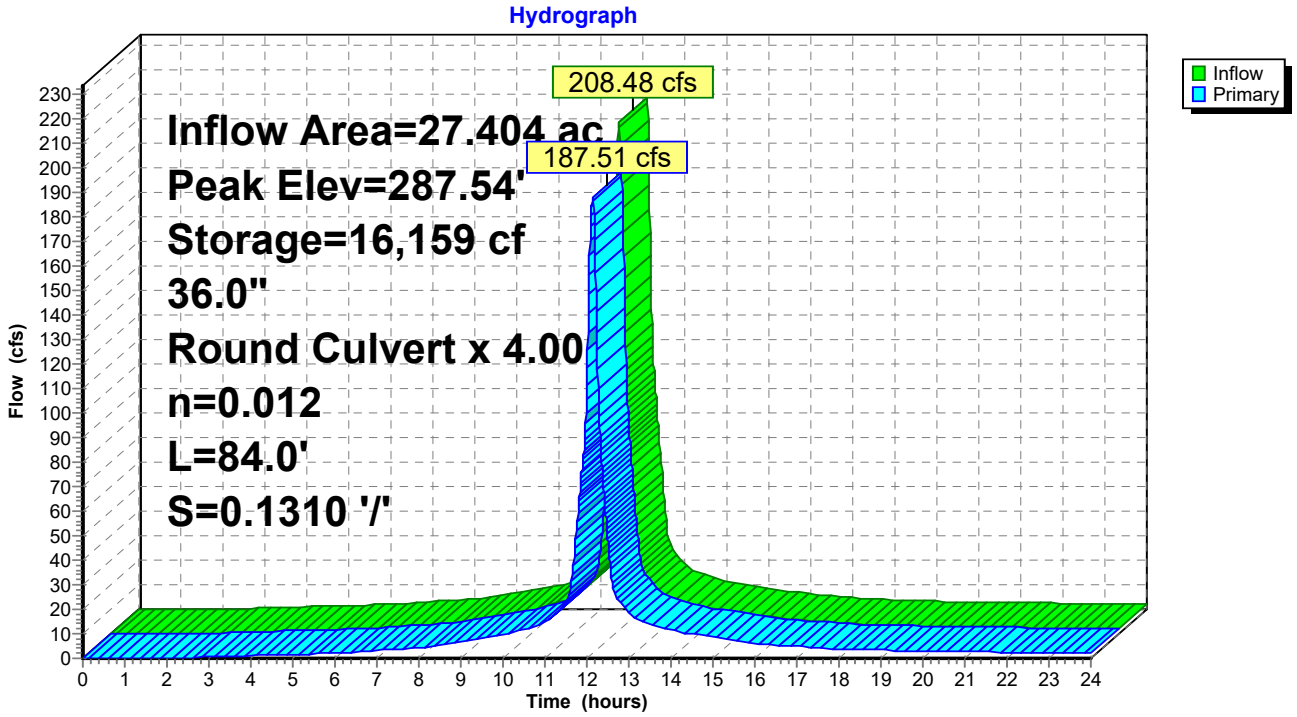
Volume	Invert	Avail.Storage	Storage Description
#1	283.00'	46,466 cf	**Custom Stage Data (Prismatic)** Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
283.00	65	0	0
284.00	722	394	394
287.00	6,965	11,531	11,924
290.00	16,063	34,542	46,466

Device	Routing	Invert	Outlet Devices
#1	Primary	283.00'	**36.0" Round Culvert X 4.00** L= 84.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 283.00' / 272.00' S= 0.1310 '/' Cc= 0.900 n= 0.012, Flow Area= 7.07 sf

Primary OutFlow Max=187.37 cfs @ 12.13 hrs HW=287.54' TW=0.00' (Dynamic Tailwater)
↑—**1=Culvert** (Inlet Controls 187.37 cfs @ 6.63 fps)

Pond 5P: 4x36" CHDPE

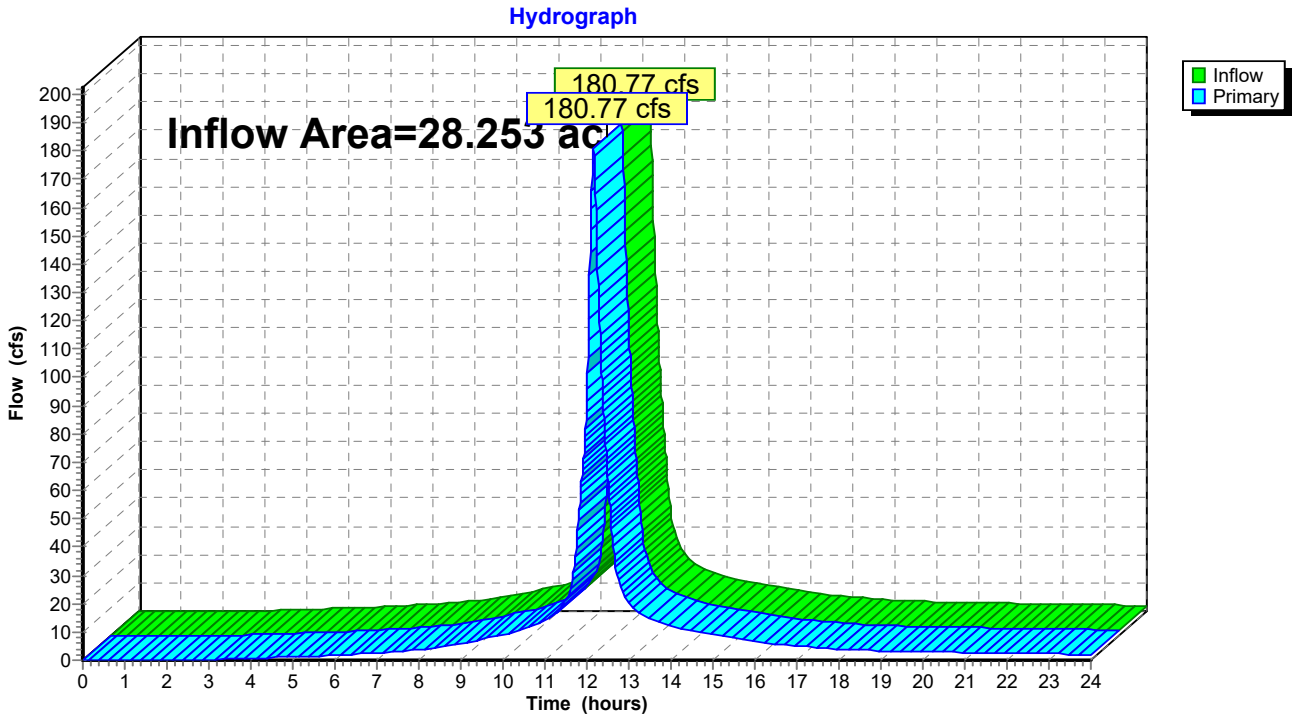


Summary for Link 2L: Offsite

Inflow Area = 28.253 ac, 0.00% Impervious, Inflow Depth > 7.07" for 25-yr, 24-hr event
Inflow = 180.77 cfs @ 12.16 hrs, Volume= 16.646 af
Primary = 180.77 cfs @ 12.16 hrs, Volume= 16.646 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Link 2L: Offsite

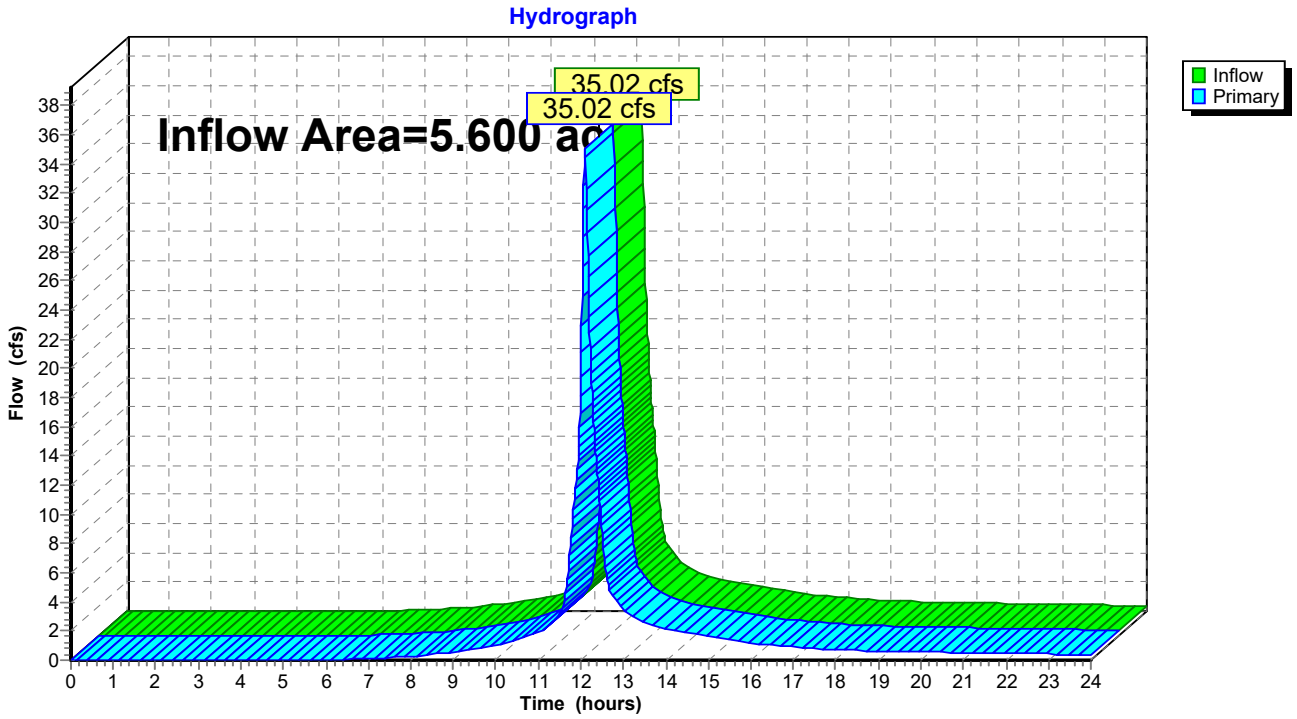


Summary for Link 3L: Stormwater Outfall

Inflow Area = 5.600 ac, 0.00% Impervious, Inflow Depth > 5.65" for 25-yr, 24-hr event
Inflow = 35.02 cfs @ 12.10 hrs, Volume= 2.637 af
Primary = 35.02 cfs @ 12.10 hrs, Volume= 2.637 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Link 3L: Stormwater Outfall

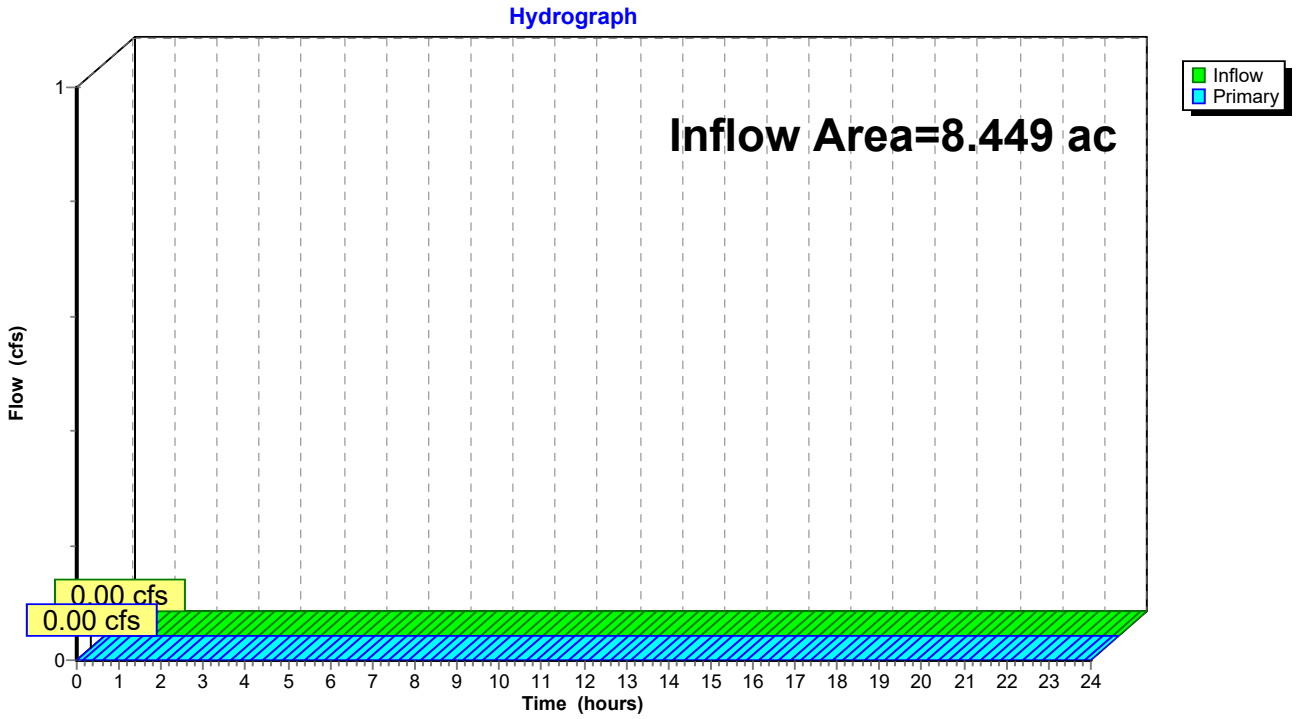


Summary for Link 4L: Offsite

Inflow Area = 8.449 ac, 33.66% Impervious, Inflow Depth = 0.00" for 25-yr, 24-hr event
Inflow = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af
Primary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Link 4L: Offsite

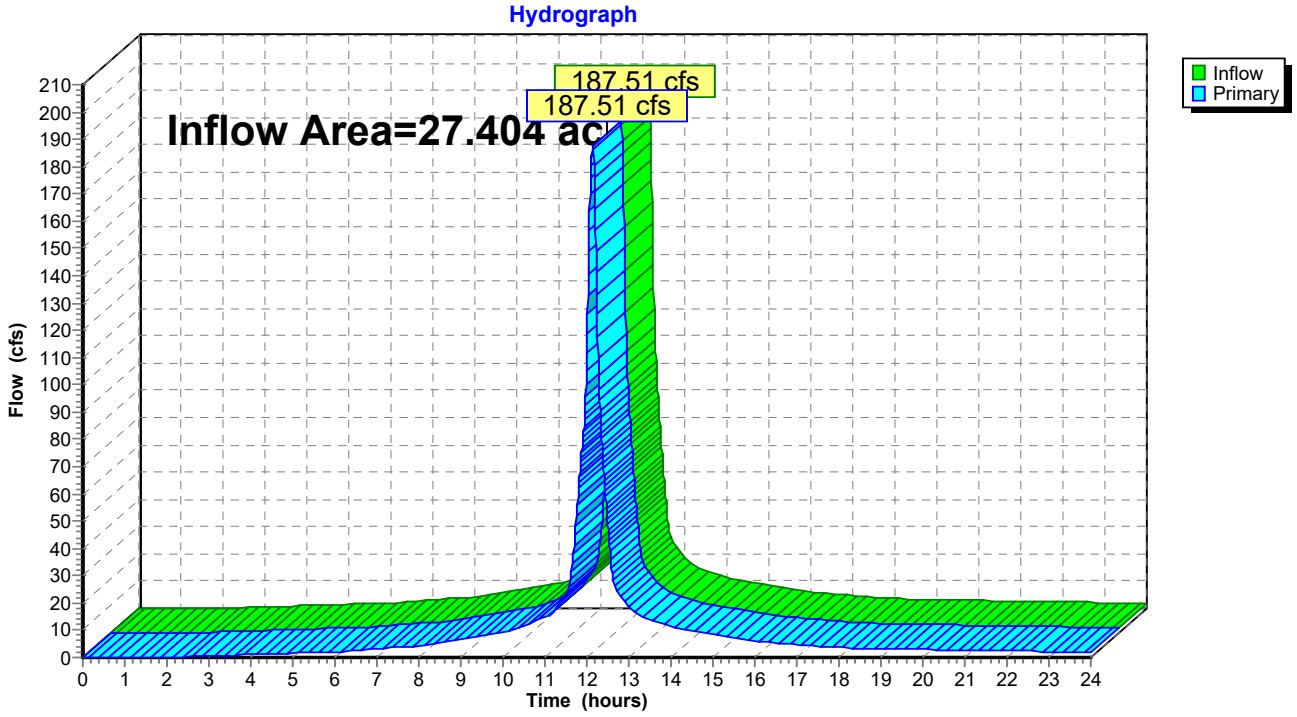


Summary for Link 6L: Offsite

Inflow Area = 27.404 ac, 0.00% Impervious, Inflow Depth > 7.31" for 25-yr, 24-hr event
Inflow = 187.51 cfs @ 12.13 hrs, Volume= 16.703 af
Primary = 187.51 cfs @ 12.13 hrs, Volume= 16.703 af, Atten= 0%, Lag= 0.0 min
Routed to nonexistent node 1L

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Link 6L: Offsite



Worksheet for Landfill Runoff - Trapezoidal Channel

Project Description	
Friction Method	Manning Formula
Solve For	Normal Depth

Input Data	
Roughness Coefficient	0.020
Channel Slope	0.050 ft/ft
Left Side Slope	3.000 H:V
Right Side Slope	3.000 H:V
Bottom Width	5.00 ft
Discharge	208.48 cfs

Results	
Normal Depth	17.0 in
Flow Area	13.1 ft²
Wetted Perimeter	14.0 ft
Hydraulic Radius	11.3 in
Top Width	13.50 ft
Critical Depth	29.1 in
Critical Slope	0.005 ft/ft
Velocity	15.92 ft/s
Velocity Head	3.94 ft
Specific Energy	5.36 ft
Froude Number	2.849
Flow Type	Supercritical

GVF Input Data	
Downstream Depth	0.0 in
Length	0.0 ft
Number Of Steps	0

GVF Output Data	
Upstream Depth	0.0 in
Profile Description	N/A
Profile Headloss	0.00 ft
Downstream Velocity	Infinity ft/s
Upstream Velocity	Infinity ft/s
Normal Depth	17.0 in
Critical Depth	29.1 in
Channel Slope	0.050 ft/ft
Critical Slope	0.005 ft/ft

Worksheet for Outside Runoff - Triangular Channel

Project Description	
Friction Method	Manning Formula
Solve For	Normal Depth

Input Data	
Roughness Coefficient	0.025
Channel Slope	0.020 ft/ft
Left Side Slope	3.000 H:V
Right Side Slope	3.000 H:V
Discharge	35.02 cfs

Results	
Normal Depth	16.4 in
Flow Area	5.6 ft²
Wetted Perimeter	8.6 ft
Hydraulic Radius	7.8 in
Top Width	8.18 ft
Critical Depth	18.4 in
Critical Slope	0.011 ft/ft
Velocity	6.28 ft/s
Velocity Head	0.61 ft
Specific Energy	1.98 ft
Froude Number	1.342
Flow Type	Supercritical

GVF Input Data	
Downstream Depth	0.0 in
Length	0.0 ft
Number Of Steps	0

GVF Output Data	
Upstream Depth	0.0 in
Profile Description	N/A
Profile Headloss	0.00 ft
Downstream Velocity	Infinity ft/s
Upstream Velocity	Infinity ft/s
Normal Depth	16.4 in
Critical Depth	18.4 in
Channel Slope	0.020 ft/ft
Critical Slope	0.011 ft/ft

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