



History of Construction

Brame Fly Ash Pond



CLECO Corporation

Rodemacher Unit 2
Project No. 90965

Revision 0
10/14/2016



History of Construction

Brame Fly Ash Pond

prepared for

**CLECO Corporation
Rodemacher Unit 2
Rapides Parish, Louisiana**

Project No. 90965

**Revision 0
10/14/2016**

prepared by

**Burns & McDonnell Engineering Company, Inc.
Kansas City, Missouri**

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INDEX AND CERTIFICATION

CLECO Corporation
History of Construction
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Report Index

<u>Chapter Number</u>	<u>Chapter Title</u>	<u>Number of Pages</u>
1.0	Introduction	1
2.0	Plan Objectives	1
3.0	History of Construction	2

Certification

I hereby certify, as a Professional Engineer in the state of Louisiana, that the information in this document was assembled under my direct personal charge. This report is not intended or represented to be suitable for reuse by the CLECO Corporation or others without specific verification or adaptation by the Engineer.

Randell L Sedlacek
Randell L Sedlacek, P.E.
Louisiana License #38408

Date: 10/14/16

TABLE OF CONTENTS

	<u>Page No.</u>
1.0 INTRODUCTION.....	1-1
2.0 PLAN OBJECTIVES	2-1
3.0 HISTORY OF CONSTRUCTION.....	3-1
APPENDIX A - USGS TOPOGRAPHICAL MAP	
APPENDIX B - AREA-CAPACITY CURVE	
APPENDIX C - GEOLOGIC DATA	
APPENDIX D - DESIGN DOCUMENTS	
APPENDIX E - INSTRUMENTATION	

LIST OF ABBREVIATIONS

<u>Abbreviation</u>	<u>Term/Phrase/Name</u>
BMcD	Burns & McDonnell
Brame	Brame Energy Center
CCR	Coal Combustion Residual
CFR	Code of Federal Regulations
CLECO	CLECO Corporation
CY	Cubic Yards
EPA	Environmental Protection Agency
GPM	Gallons per Minute
HUC	Hydrologic Unit Code
LSU	Louisiana State University
U.S.C	United States Code
USGS	United States Geological Survey

1.0 INTRODUCTION

On April 17, 2015, the Environmental Protection Agency (EPA) issued the final version of the federal Coal Combustion Residual (CCR) Rule to regulate the disposal of CCR materials generated at coal-fired units. The rule will be administered as part of the Resource Conservation and Recovery Act [RCRA, 42 United States Code (U.S.C.) §6901 et seq.], using the Subtitle D approach.

The existing CCR impoundments at CLECO Corporation's (CLECO's) Brame Energy Center (Brame) are subject to the CCR Rule and as such CLECO must compile a History of Construction for each CCR surface impoundment per 40 CFR §257.73(c). This report serves as the History of Construction for the Fly Ash Pond at Brame.

This history of construction is in addition to, not in place of, any other applicable site permits, environmental standards, or work safety practices.

2.0 PLAN OBJECTIVES

Per 40 CFR §257.73(c), the History of Construction must contain, to the extent feasible, the following items:

- The name and address of the owner/operator of the CCR unit, the name of the CCR unit, and the identification number of the CCR unit
- The location of the CCR unit on most recent United States Geological Survey (USGS) 7½ minute or 15 minute topographical map
- A statement of the purpose for which the CCR unit is being used
- The name and size of watershed within which the CCR unit is located
- A description of the physical and engineering properties of the foundation and abutment materials on which the CCR unit is constructed
- A statement of the type, size, range, and physical and engineering properties of the materials used in constructing each zone or stage of the CCR unit; the method of site preparation and construction of each zone or stage of the CCR unit; and the approximate dates of construction of each successive stage of construction of the CCR unit.
- Detailed dimensional drawings of the CCR Unit, including the following:
 - Plan view and cross sections of the length and width of the CCR unit
 - Foundation improvements
 - Drainage provisions, spillways, diversion ditches, outlets
 - Instrumentation locations
 - Slope protection
 - Normal operating pool surface elevation
 - Maximum pool surface elevation following peak discharge from the inflow design flood
 - Expected maximum depth of CCR within the unit
 - Any identifiable natural or manmade features that could adversely affect operation of the CCR unit due to malfunction or mis-operation
- A description of the type, purpose, and location of existing instrumentation
- Area-capacity curves for the CCR unit
- A description of each spillway and diversion design features and capacities and calculations used in their determination
- The construction specifications and provisions for surveillance, maintenance, and repair of the CCR unit
- Any record or knowledge of structural instability of the CCR unit

3.0 HISTORY OF CONSTRUCTION

Section	CCR Rule Description	Included	Information	Source
40 CFR §257.73 (c)(1) (i)	Name and address of the owner/operator of the CCR unit	Y	Cleco Corporation 2030 Donahue Ferry Road P.O. Box 5000 Pineville, LA 71361-5000	Owner
	Name of the CCR units Identification number of the CCR unit	Y Y	Fly Ash Pond P-0005 (permit number)	Owner Owner
40 CFR §257.73 (c)(1) (ii)	Location of the CCR unit on most recent United States Geological Survey (USGS) 7½ minute or 15 minute topographical map	Y	See Appendix A	USGS Map
40 CFR §257.73 (c)(1) (iii)	Statement of the purpose for which the CCR unit is being used	Y	The pond receives fly ash from the dry fly ash silos that is loaded onto trucks, hauled, and placed in the pond for disposal.	Owner
40 CFR §257.73 (c)(1) (iv)	Name and size of watershed within which the CCR unit is located	Y	HUC12 = 111402070703 Jacks Creek Watershed Area = 9,407 acres Only approximately 39 acres of this overall watershed actually contributes runoff to the Fly Ash Pond.	HUC 12 Mapping - LSU AGCenter.com
40 CFR §257.73 (c)(1) (v)	Description of the physical and engineering properties of the foundation and abutment materials on which the CCR unit is constructed	Y	Foundation Soils include soft to medium stiff clay and silty clay. See Appendix C for Geologic Data	Permit
40 CFR §257.73 (c)(1) (vi)	Statement of the type, size, range, and physical and engineering properties of the materials used in constructing each zone or stage of the CCR unit	Y	Embankment constructed of on-site material. Embankment contains 3-ft compacted clay liner with average Plasticity Index of 29 and average Liquid Limit of 49. See Appendix C permit data	Permit / Correspondence between CLECO and Louisiana DNR
	The method of site preparation and construction of each zone or stage of the CCR unit	Y	Original embankment constructed with 3:1 side slopes with 3-ft compacted clay liner. Liner was constructed in 8-10 in. horizontal lifts compacted with sheepfoot compaction. Post-construction inspection in 1982 indicated in-situ liner insufficient in large areas of pond (allegedly due to excessive borrow during construction of Bottom Ash Pond). Footprint / dikes reconfigured to 30 acre area (from original 104 acre area) with compacted liner placed in insufficient areas. See Appendix C for original permit data	Correspondence between CLECO and Louisiana DNR
	The approximate dates of construction of each successive stage of construction of the CCR unit	Y	Began construction in 1978. Post-construction inspection in 1982, footprint / dikes reconfigured to 30 acre area (from 104 acre area) with compacted liner placed in insufficient areas. Construction completed in 1984.	Correspondence between CLECO and Louisiana DNR

40 CFR §257.73 (c)(1) (vii)	Detailed dimensional drawings including the following: Plan view and cross sections of the length and width of the CCR unit Foundation improvements Drainage provisions, spillways, diversion ditches, outlets Instrumentation locations Slope protection Normal operating pool surface elevation Maximum pool surface elevation following peak discharge from the inflow design flood Expected maximum depth of CCR within the unit Any identifiable natural or manmade features that could adversely affect operation of the CCR unit due to malfunction or mis-operation	Y N Y Y Y Y Y Y	See Appendix D N/A 6" HDPE pipe (discharges to adjacent Bottom Ash Pond via manually started pump only) Pump info not found in records review See Appendix E for groundwater monitoring well locations. No other instruments are installed at this time. Exterior: 3:1, vegetated Interior: 3:1, vegetated Normal operating level at EL 86 Maximum pool surface level at EL 90.6 following peak discharge from the inflow design flood, assuming pond is 50% full of ash to the top of the dike and initial water level of EL 86. At EL 105, depth of CCR is approximately 21 feet. 250 GPM pump - manually operated to control pond overflow	Permit Owner Permit Owner Inflow Design Flood Control System Plan, Oct. 2016 Measured in CAD based on Construction Drawings / aerial imagery Owner
40 CFR §257.73 (c)(1) (viii)	Description of the type, purpose, and location of existing instrumentation	Y	Monitoring wells - monitor uppermost aquifer (compliance and background monitoring). See Appendix E for locations. Surface water level gauge (measured manually)	Permit
40 CFR §257.73 (c)(1) (ix)	Area-capacity curves for the CCR unit	Y	See Appendix B	Measured in CAD based on Construction Drawings / aerial imagery
40 CFR §257.73 (c)(1) (x)	Description of each spillway and diversion design features and capacities and calculations used in their determination	Y	Pump - controls pond overflow 0.086 MGD to Bottom Ash Pond (when pumping)	Owner
40 CFR §257.73 (c)(1) (xi)	Construction specifications Provisions for surveillance, maintenance, and repair of the CCR unit	N Y	Not found in records review Weekly and Annual Inspections per 40 CFR §257.83.	
40 CFR §257.73 (c)(1) (xii)	Any record or knowledge of structural instability of the CCR unit	N	N/A	

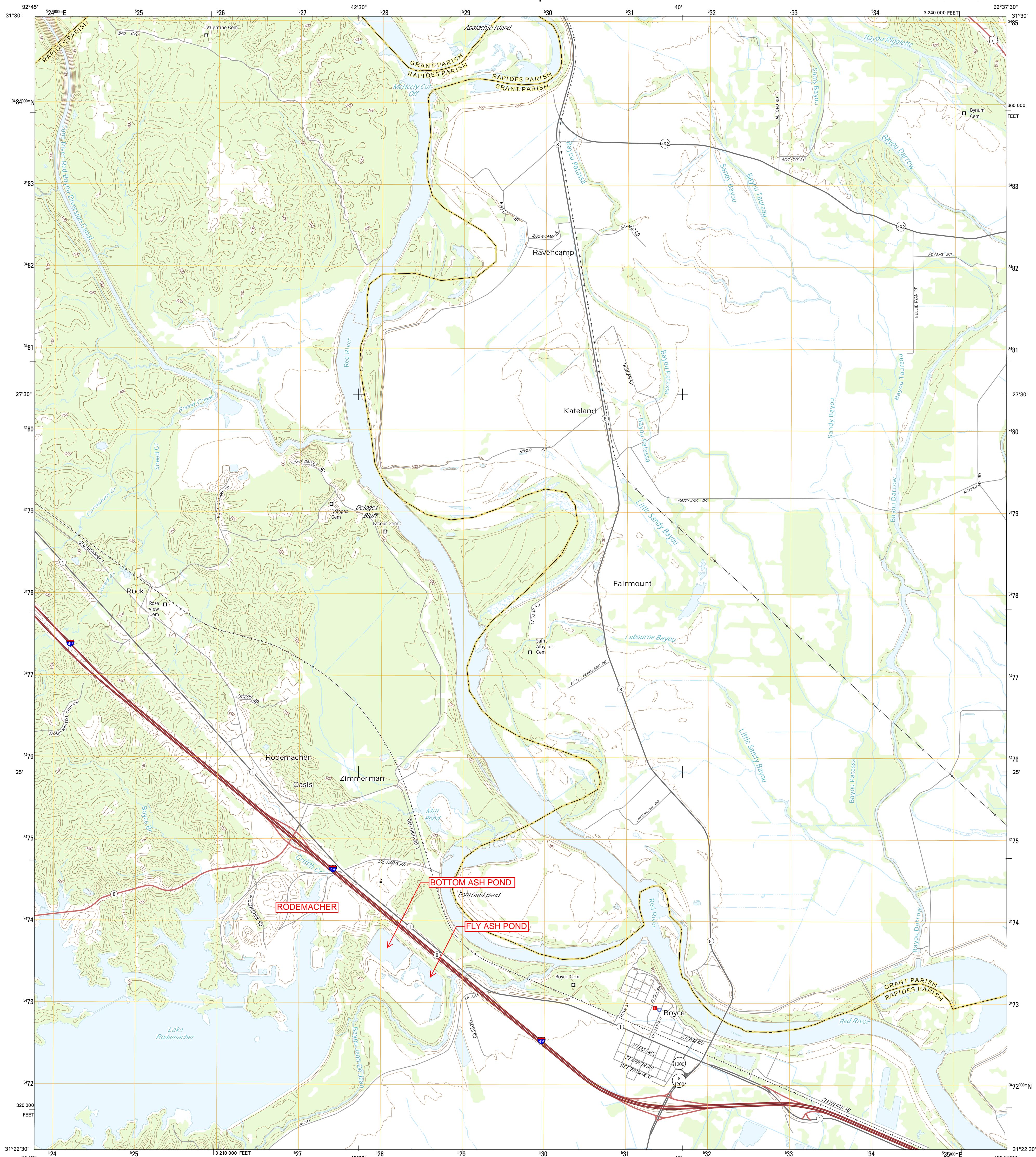
APPENDIX A - USGS TOPOGRAPHICAL MAP



U.S. DEPARTMENT OF THE INTERIOR
U.S. GEOLOGICAL SURVEY



BOYCE QUADRANGLE
LOUISIANA
7.5-MINUTE SERIES



Produced by the United States Geological Survey

North American Datum of 1983 (NAD83)
World Geodetic System of 1984 (WGS84). Projection and
1000-meter grid: Universal Transverse Mercator, Zone 15R
10,000-foot ticks: Louisiana Coordinate System of 1983 (north
zone)

This map is not a legal document. Boundaries may be
generalized for this map scale. Private lands within government
reservations may not be shown. Obtain permission before
entering private lands.

Imagery: NAIP, June 2013
Roads: HERE, ©2013 - 2014
Names: GNIS, 2015
Hydrography: National Hydrography Dataset, 2013
Contour: National Elevation Dataset, Ellipsoid Height, 10' (15' grid)
Boundaries: Multiple sources; see metadata file 1972 - 2015
Public Land Survey System: BLM, 2015

NORTH AMERICAN VERTICAL DATUM OF 1988
DECLINATION AT CENTER OF SHEET
0° 10' 19.5" N
0° 10' 3 MILES

UTM GRID AND 2015 MAGNETIC NORTH
DECLINATION AT CENTER OF SHEET

U.S. National Grid	100,000 m Square ID
WO	Grid Zone Designation 15R

SCALE 1:24 000
1 0.5 0 1 2
KILOMETERS
1000 500 0 1000 2000
1 0.5 0 1 2
MILES
1000 0 1000 2000 3000 4000 5000 6000 7000 8000 9000 10000

CONTOUR INTERVAL 10 FEET

NORTH AMERICAN VERTICAL DATUM OF 1988

This map was produced to conform with the
National Geospatial Program US Topo Product Standard, 2011.
A metadata file associated with this product is draft version 0.6.18



QUADRANGLE LOCATION

1	2	3
4		5
6	7	8

ADJOINING QUADRANGLES

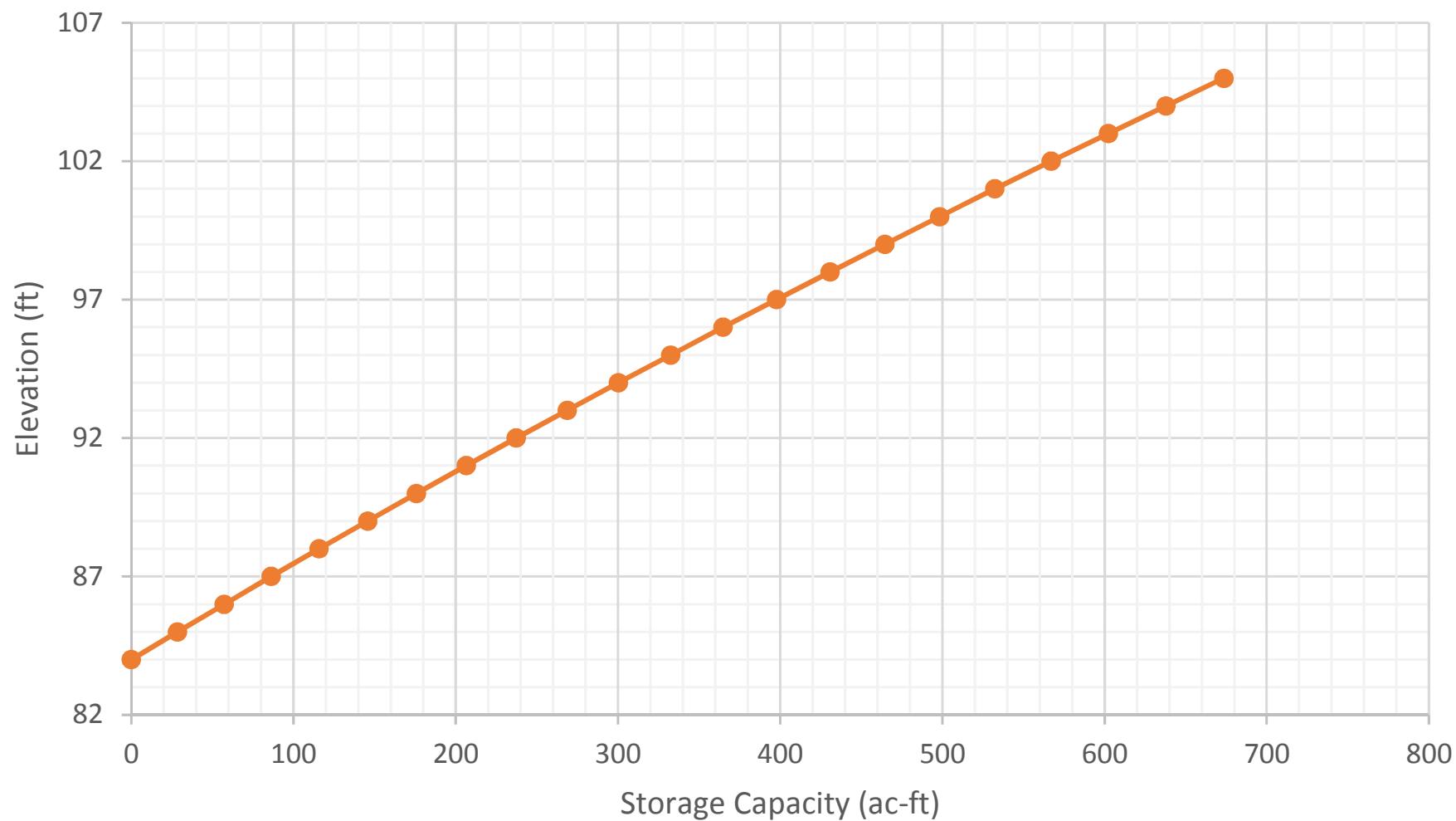
ROAD CLASSIFICATION
Expressway
Secondary Hwy
Ramp
4WD
Interstate Route
US Route
State Route

BOYCE, LA
2015

NSN 76-3016368754
NGA REF NO. USGS X 24 K 5 12

APPENDIX B - AREA-CAPACITY CURVE

Brame - Fly Ash Pond



APPENDIX C - GEOLOGIC DATA

521.D. FACILITY GEOLOGY

1. INFORMATION REQUIRED FOR TYPE I AND II FACILITIES

Exhibits 20 through 24 present representative soil conditions for the Fly Ash Pond. These borings were done before excavation of clay materials used to form a three-foot thick compacted liner for the dikes. After excavation, the Fly Ash Pond will be underlain by a 3-foot to 13-foot top layer of reddish-brown clay to gray clay which grades in some areas to a silt/sandy silt. Beneath most of the Fly Ash Pond and underlying the top clay stratum is approximately 25 to 40 feet of clay below which are sands and clayey sands 10 feet or more in thickness. Laboratory permeability of the clay tested at 1.1×10^{-8} cm/sec. The Atterburg Limit values for Plasticity Index averaged 29 with an average liquid limit of 49 which classifies the clay as a CH according to the Unified Soil Classification System.

Compaction and Atterburg Limits tests were being performed on the clay liner of the Fly Ash Pond dike. The minimum specified Plasticity Index is 15 with 60 percent passing the #200 sieve and compaction is 95 percent of modified proctor test. The clay liner is being placed in 8- to 10-inch layers and compacted with "sheep's foot" equipment. Given these criteria and knowing the in situ characteristics of the clay which is being excavated (Average Plasticity Index 29), an effective protective layer should be present for the ground water.

After clay liner material is borrowed from the Fly Ash Pond area, hand auger borings will be performed to confirm that a 3-foot thickness of clay remains over the bottom of the Fly Ash Pond.

1.a. Isometric Profiles and Cross-sections

The locations of the 20 borings used to develop the soil profiles of the Fly Ash Pond are shown on Exhibit 25. Borings along the profile of the area of the dike were spaced 200 to 700 feet apart and extended to an average depth of 50 feet below grade. Borings inside the Fly Ash Pond were spaced from 100 to 400 feet apart and varied from a minimum depth of 15 feet to a maximum depth of 50 feet. Five cross sections of the subsurface conditions are illustrated on Exhibits 20 through 24.

Section A-A covers the most northwestern portion of the Fly Ash Pond from a southwest to northeast direction. Section B-B is near the central portion of the pond in a southwest to northeast direction and section C-C is in the lower southeast corner of the pond with the profile covering a southwest to northeast section.

Two profiles, Sections D-D and E-E, cover the center area of the pond and both sections are drawn to cover a northwest to southeast direction.

1.b. Soil Boring Logs

Representations of the boring logs are included on Exhibits 20 through 24.

521.D (cont'd)

1.c. **Soil Test Results**

Appendix F is a summary of representative laboratory tests on soils from the Fly Ash Pond area.

1.d. **Geologic Cross-sections**

Exhibit 5 contains geologic cross-sections which extend to a depth greater than 200 feet below ground surface.

1.e. **Faults**

The review of available published information did not reveal information on faulting near the facility and faults have not been noted within the facility itself.

1.f. **Seismic Impact Zone**

The facility is not located in a seismic impact zone.

1.g. **Unstable Areas**

The facility is not located in an unstable area.

2. **INFORMATION REQUIRED FOR TYPE III WOODWASTE AND CONSTRUCTION/DEMOLITION-DEBRIS FACILITIES.**

These sections are not applicable because the facility is not a Type III woodwaste or construction/demolition-debris facility.

Pond, the Coal Sedimentation Pond, and the Unit 2 Metal Cleaning Waste Pond are situated on the Terrace aquifer, while the Fly Ash Pond, Bottom Ash Pond, and Ash Management Area are situated on the aquifer recharge area for the Alluvial aquifer.

- e. if the facility is located in a flood plain, a plan to ensure that the facility does not restrict the flow of the 100-year base flood or significantly reduce the temporary water-storage capacity of the flood plain, and documentation indicating that the design of the facility is such that the flooding does not affect the integrity of the facility or result in the washout of solid waste.

The Fly Ash Pond is the only facility that is constructed within an area previously permitted through the Army Corps of Engineers (COE) under permit number LMNOD-SP dated March 29, 1977. Cleco currently holds a solid waste permit from the LDEQ for this area to manage ash for future plant expansions.

- D. Facility Geology. Standards governing facility geology are contained in LAC 33:VII.709.C (Type I and II facilities), LAC 33:VII.717.D (Type I-A and II-A facilities), and LAC 33:VII.719.D (Type III facilities).
 - I. The following information regarding geology is required for Type I and Type II facilities:
 - a. isometric profile and cross-sections of soils, by type, thickness, and permeability;

Isometric soil profiles and geologic cross sections have been constructed for the facilities from available data. The locations of the profiles for the cross sections are shown in Appendix G. Five (5) isometric soil profiles and sixteen (16) geologic cross sections, A-A' through P-P', were constructed from available data and are included in this appendix. Historical geologic cross sections constructed for the facilities are included in Appendix H.

- b. logs of all known soil borings taken on the facility and a description of the methods used to seal abandoned soil borings;

A copy of the logs of soil borings is included in Appendix I. Please note that the soil boring logs performed by Aquaterra (2004) and Eagle (2005) are included in this appendix. Soil boring logs were not available for the drilling activities performed by Sargent & Lundy (1981); however, geologic cross sections illustrating these logs are available and are included. Design and construction of the units began before the Louisiana Solid Waste Rules and Regulations were established and the units were initially under interim status prior to the standard permits being issued.

Abandoned soil borings were sealed in accordance with applicable methods at the time of drilling according to available records reviewed. Since May 1993, soil

borings have been sealed in accordance with applicable portions of "Construction of Geotechnical Boreholes and Groundwater Monitoring Systems Handbook" dated December 2000 (LDEQ and LDOTD, 2000).

- c. **results of tests for classifying soils (moisture contents, Atterberg limits, gradation, etc.), measuring soil strength, and determining the coefficients of permeability, and other applicable geotechnical tests;**

A copy of available geotechnical testing used for soil classification is included in Appendix I.

- d. **geologic cross-section from available published information depicting the stratigraphy to a depth of at least 200 feet below the ground surface;**

A fence diagram illustrating Rapides Parish geology to approximately 3,000 feet below ground surface is included as Appendix F. (Plate 4, Water Resources of Rapides Parish, Louisiana, Water Resources Bulletin No. 8, Department of Conservation, Louisiana Geological Survey, and Louisiana Department of Public Works, April 1966).

- e. **for faults mapped as existing through the facility, verification of their presence by geophysical mapping or stratigraphic correlation of boring logs. If the plane of the fault is verified within the facility's boundaries, a discussion of measures that will be taken to mitigate adverse effects on the facility and the environment;**

There are no known faults within the facility or within one mile of the perimeter of the facility. The review of available published information did not reveal information on faulting near the facility.

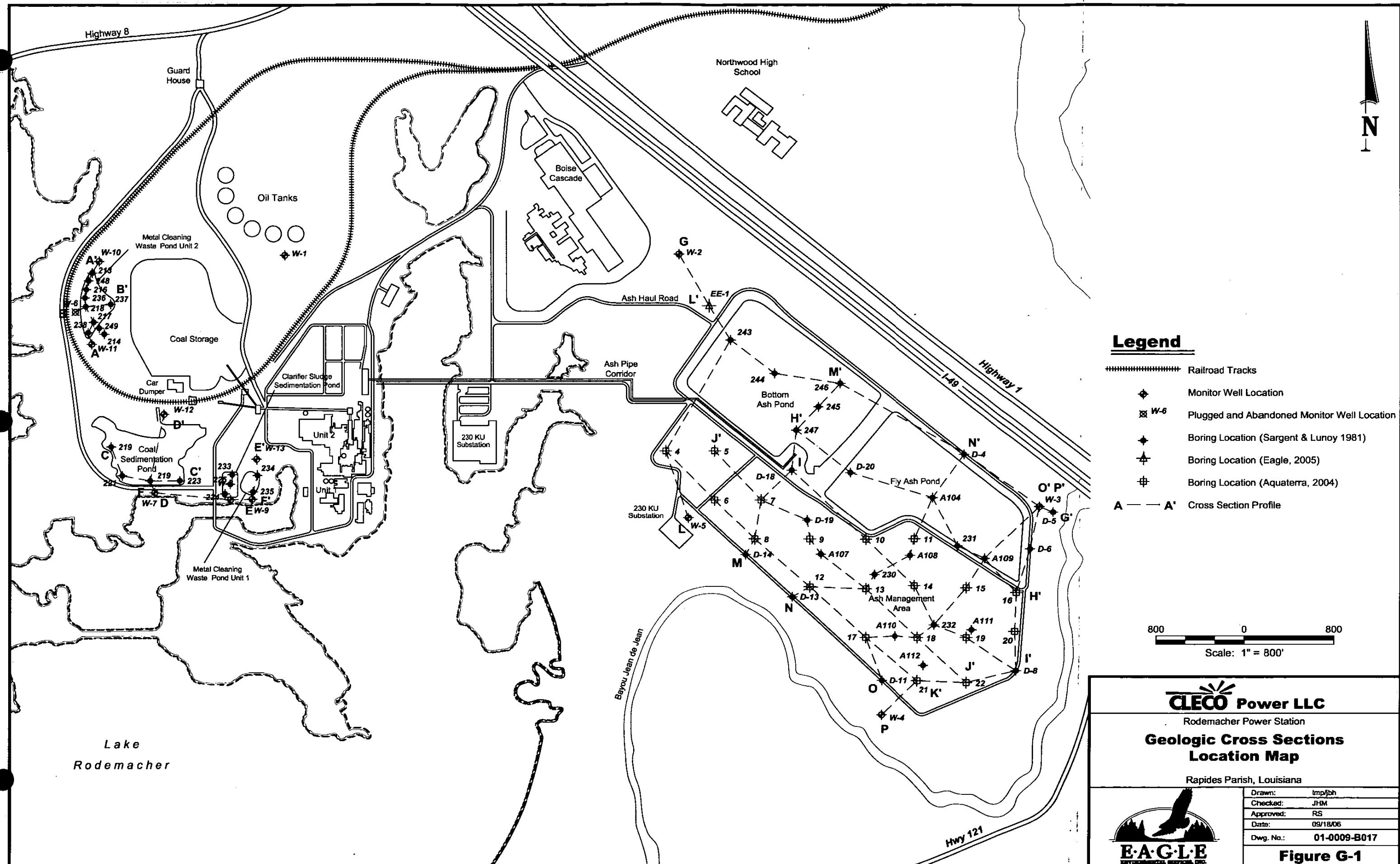
- f. **for a facility located in a seismic impact zone, a report with calculations demonstrating that the facility will be designed and operated so that it can withstand the stresses caused by the maximum ground motion, as provided in LAC 33:VII.709.C.2; and**

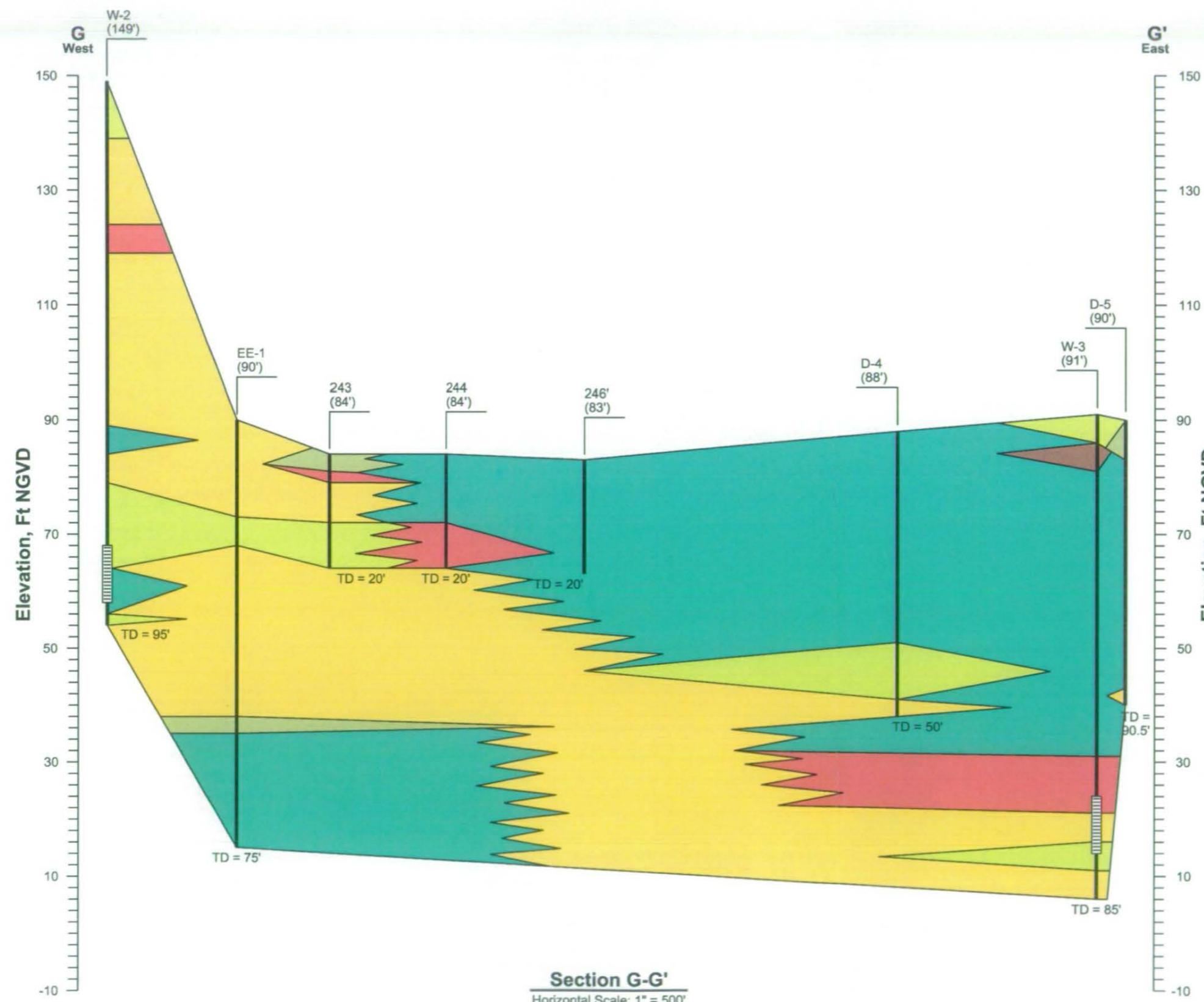
Not applicable. Review of the "Seismicity Map of the State of Louisiana" (Map MF-1081, Stover and others, United States Geological Survey, 1987) does not indicate seismic activity in the area of the Rodemacher Power Station.

- g. **for a facility located in an unstable area, a demonstration of facility design as provided in LAC 33:VII.709.C.3.**

Not applicable. The RPS is not located in an unstable area.

2. **The following information regarding geology is required by Type III wood waste, and construction/demolition-debris facilities:**



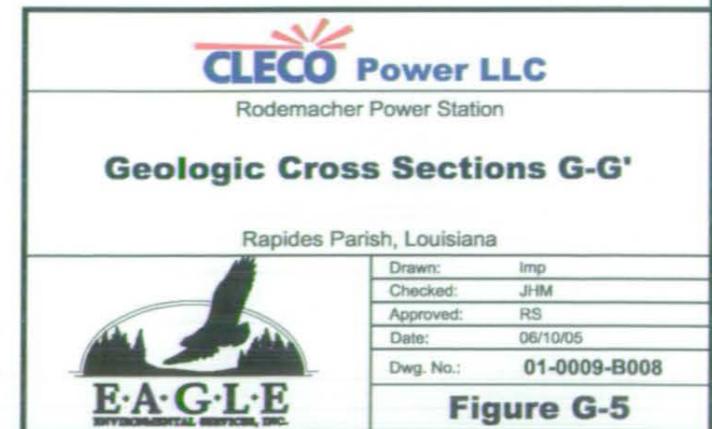


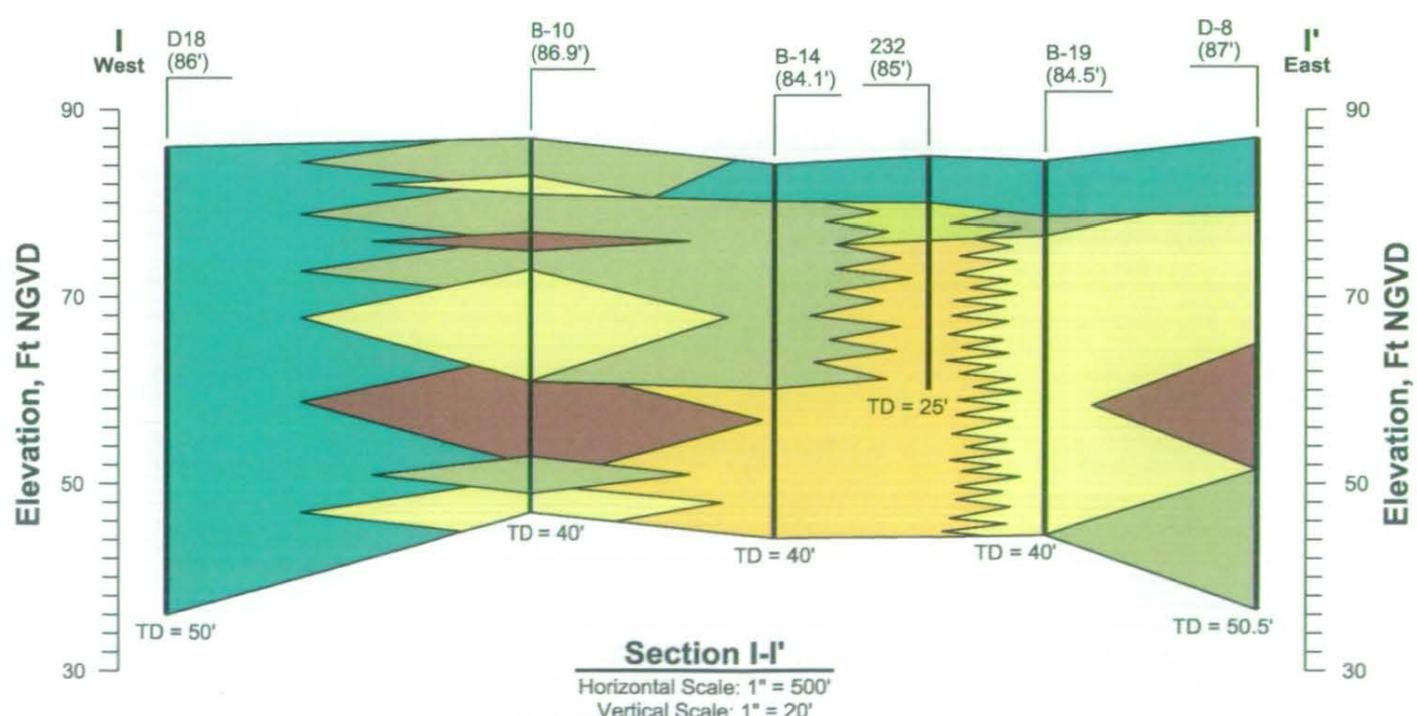
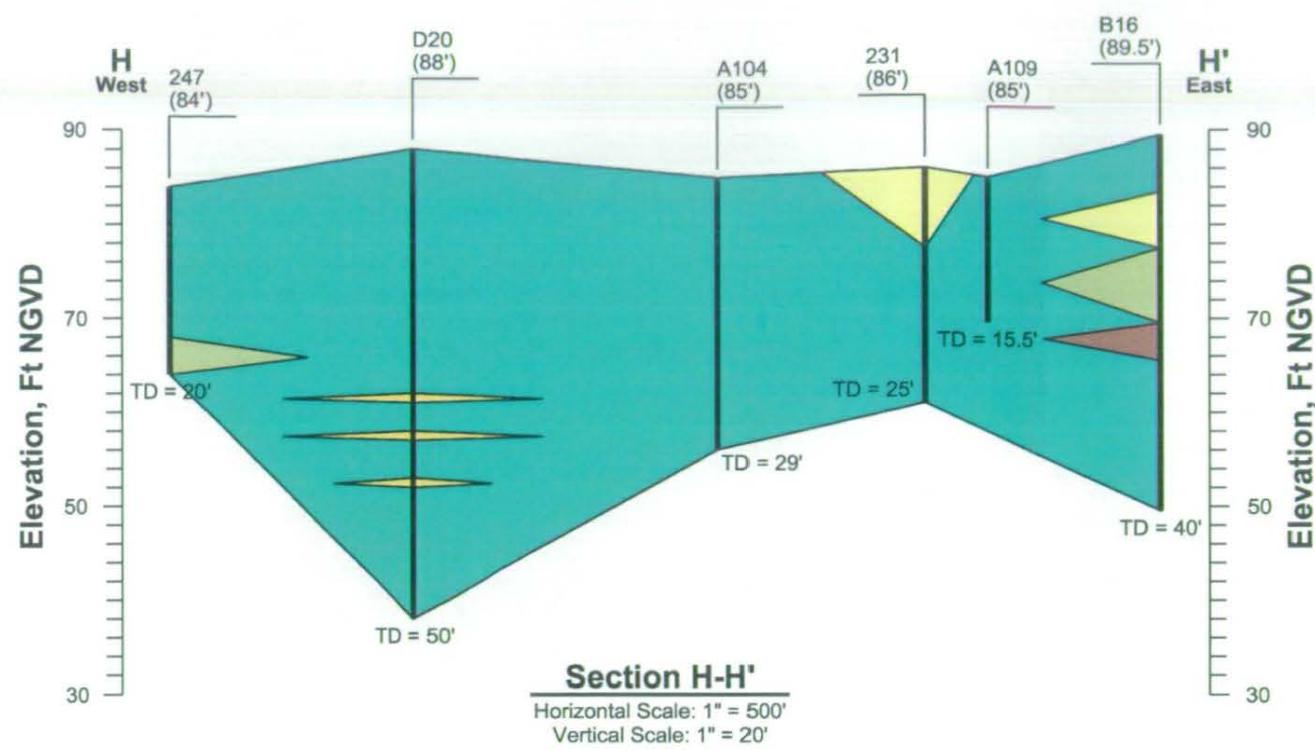
Legend

- Sand
- Silty Sand / Sandy Silt / Silt
- Sandy Clay
- Silty Clay
- Clay
- Clayey Silt
- Clayey Sand
- Screen Interval
- (114') Elevation, Ft NGVD
- TD Total Depth

Note:

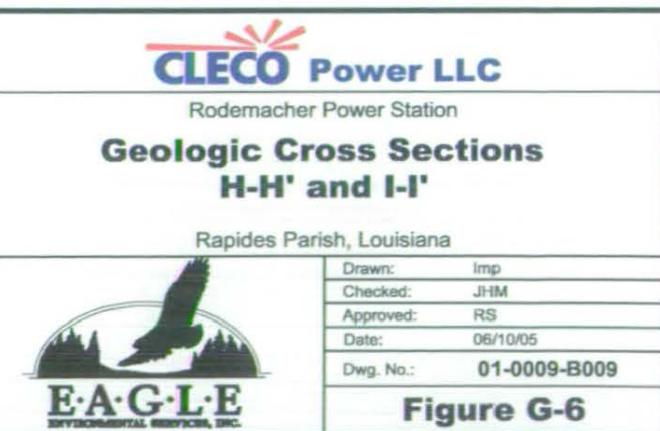
Stratigraphy between boring are inferred. Actual conditions may vary.

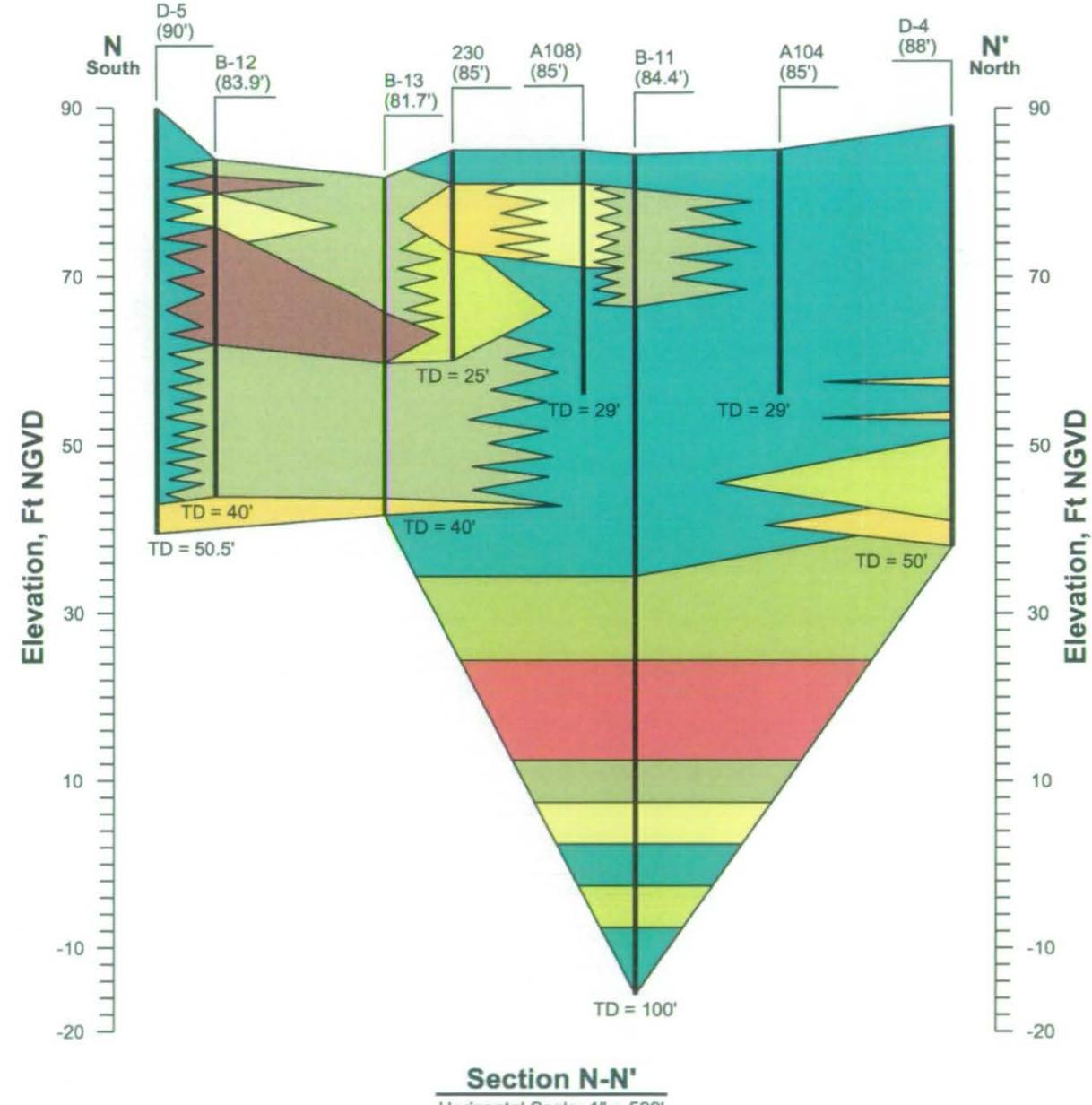




Legend	
Sand	(Yellow)
Silty Sand / Sandy Silt / Silt	(Light Yellow)
Sandy Clay	(Light Green)
Silty Clay	(Green)
Clay	(Teal)
Clayey Silt	(Brown)
Clayey Sand	(Red)
Screen Interval	(Hatched)
(114') Elevation, Ft NGVD	
TD Total Depth	

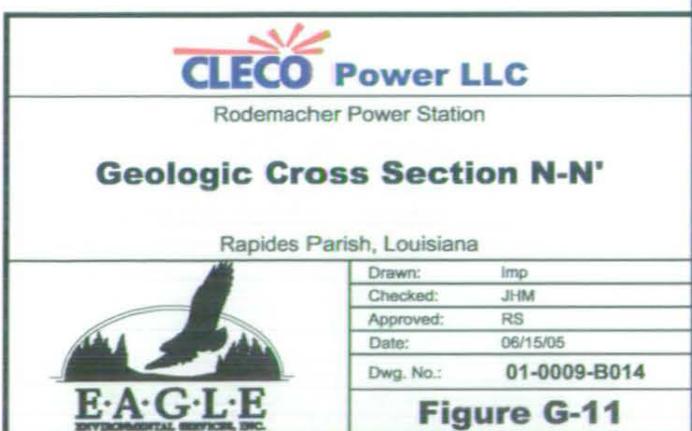
Note:
Stratigraphy between boring are inferred. Actual conditions may vary.

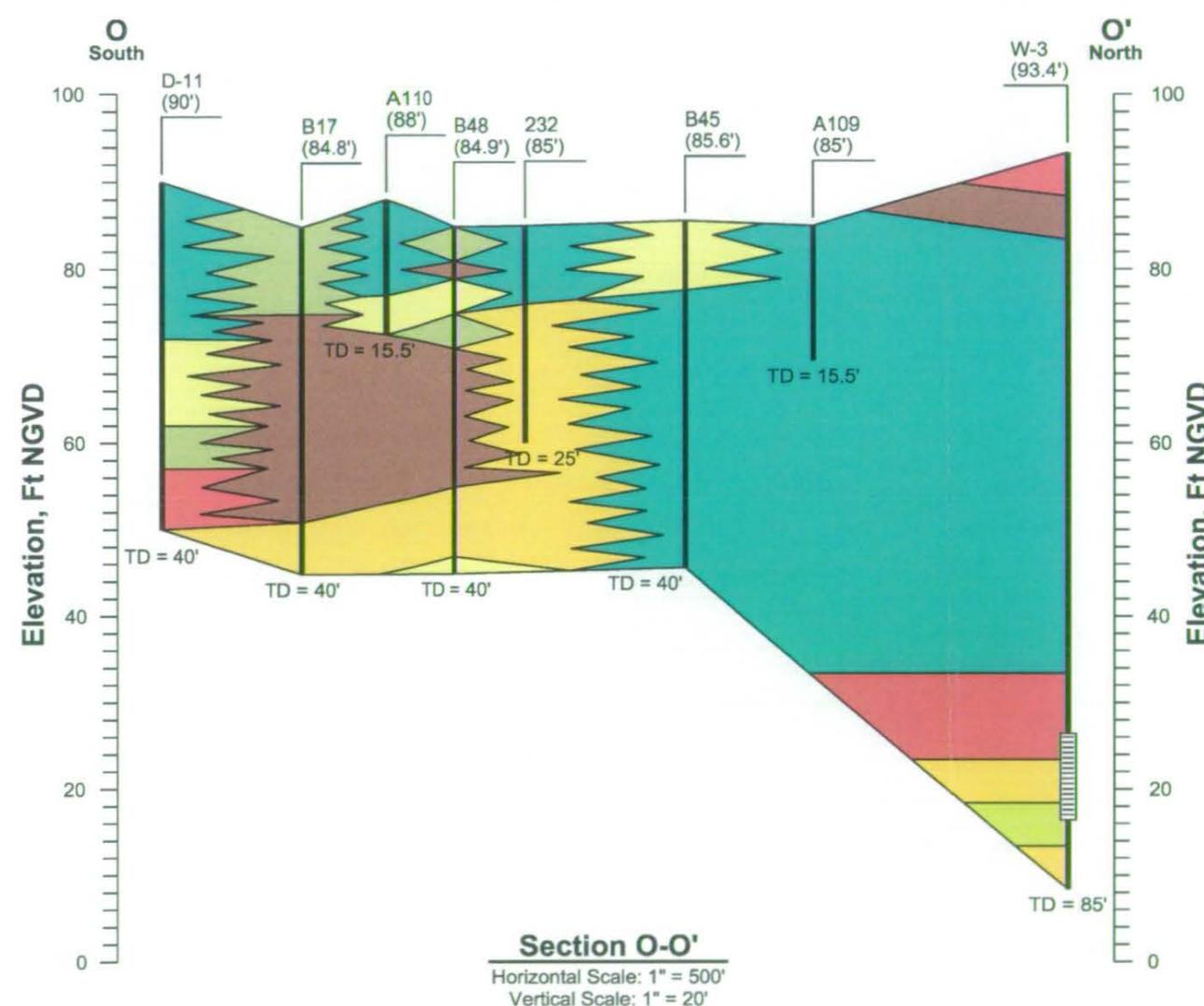




Legend	
Sand	
Silty Sand / Sandy Silt / Silt	
Sandy Clay	
Silty Clay	
Clay	
Clayey Silt	
Clayey Sand	
Wood / Clay	
Screen Interval	
(114')	Elevation, Ft NGVD
TD	Total Depth

Note:
Stratigraphy between boring are inferred. Actual conditions may vary.



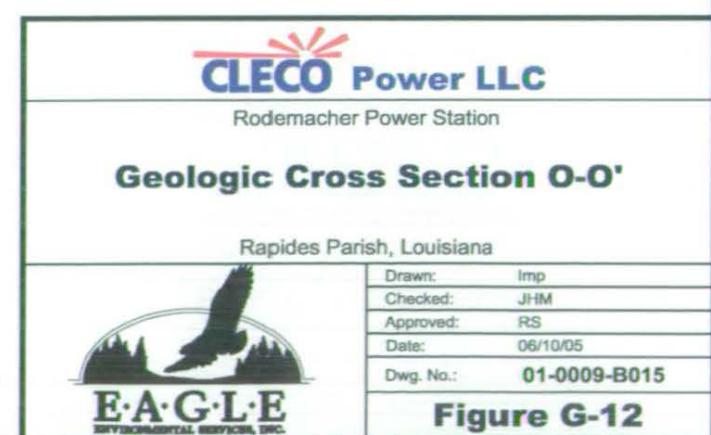


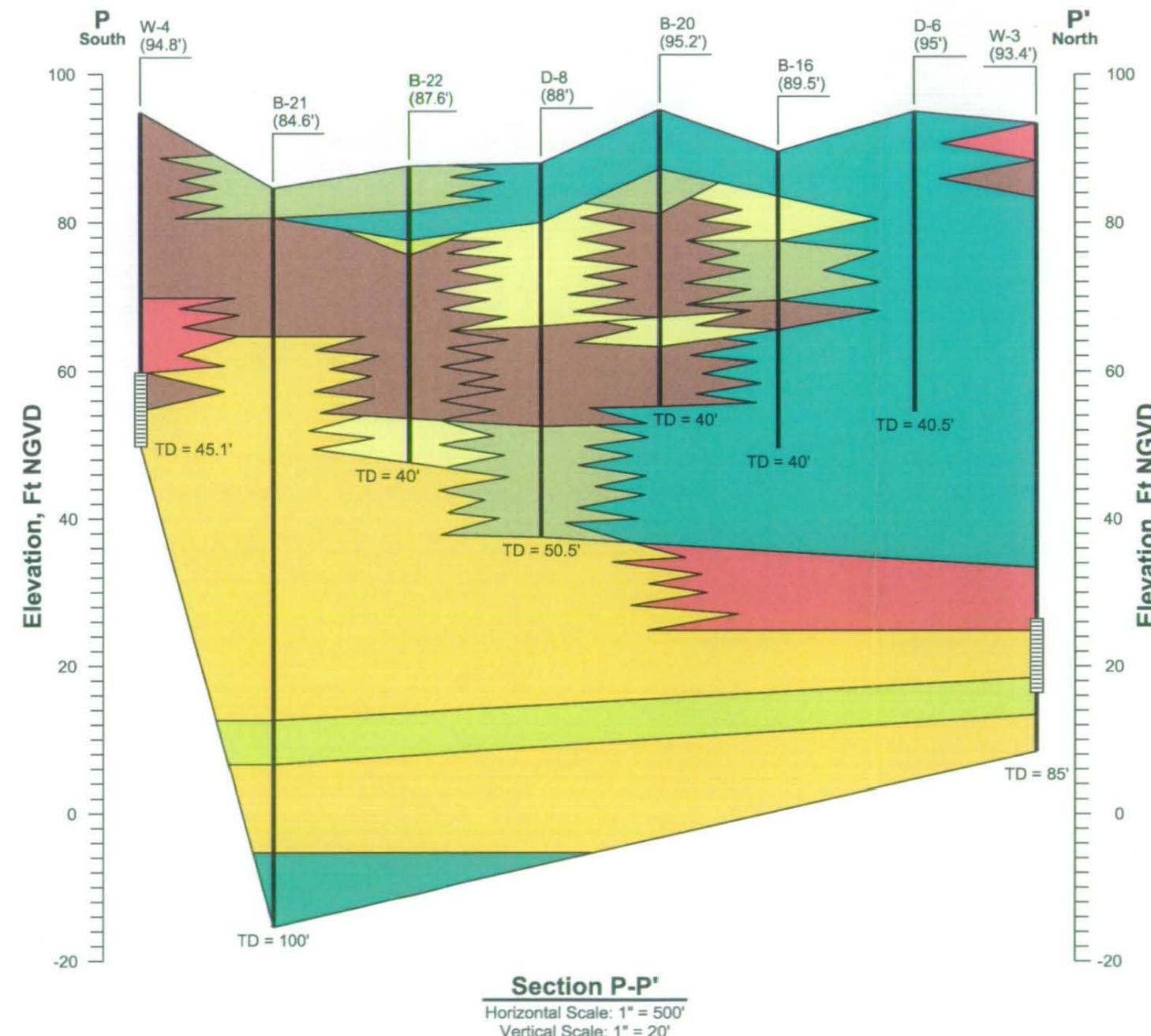
Legend

- Sand
 - Silty Sand / Sandy Silt / Silt
 - Sandy Clay
 - Silty Clay
 - Clay
 - Clayey Silt
 - Clayey Sand
 - Screen Interval
- (114') Elevation, Ft NGVD
- TD Total Depth

Note:

Stratigraphy between boring are inferred. Actual conditions may vary.

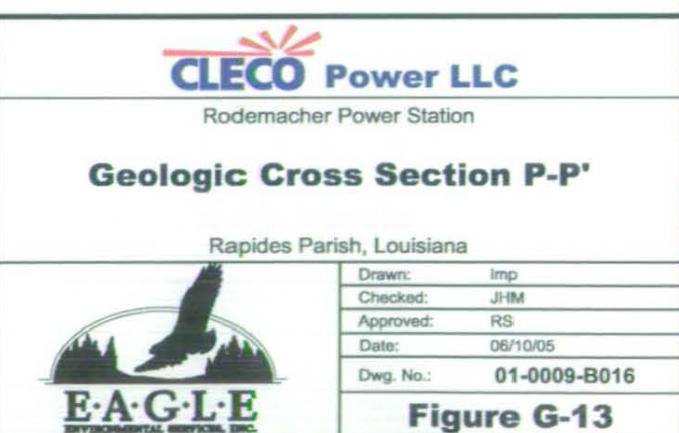


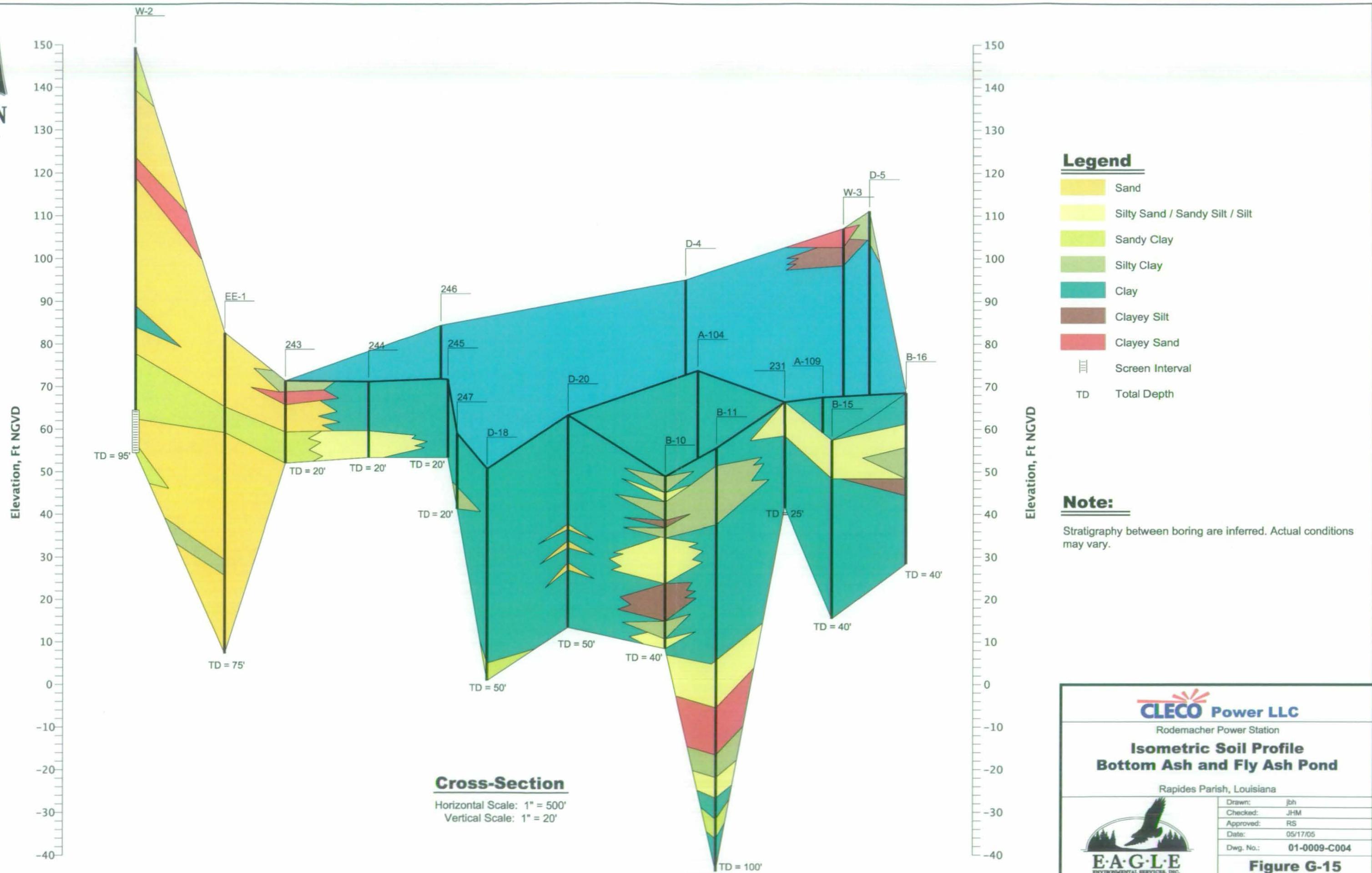


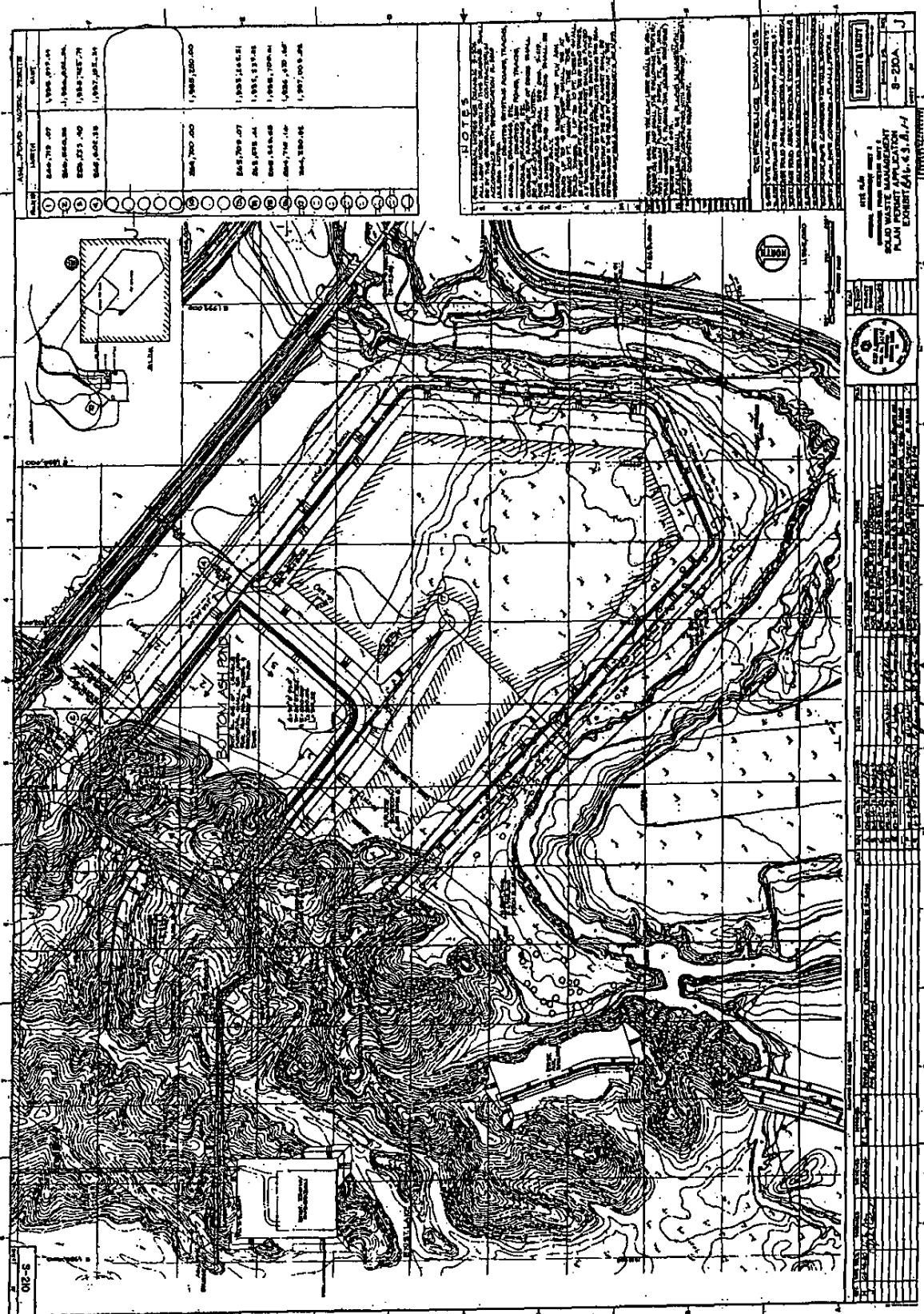
Legend	
Sand	
Silty Sand / Sandy Silt / Silt	
Sandy Clay	
Silty Clay	
Clay	
Clayey Silt	
Clayey Sand	
Screen Interval	
(114') Elevation, Ft NGVD	
TD Total Depth	

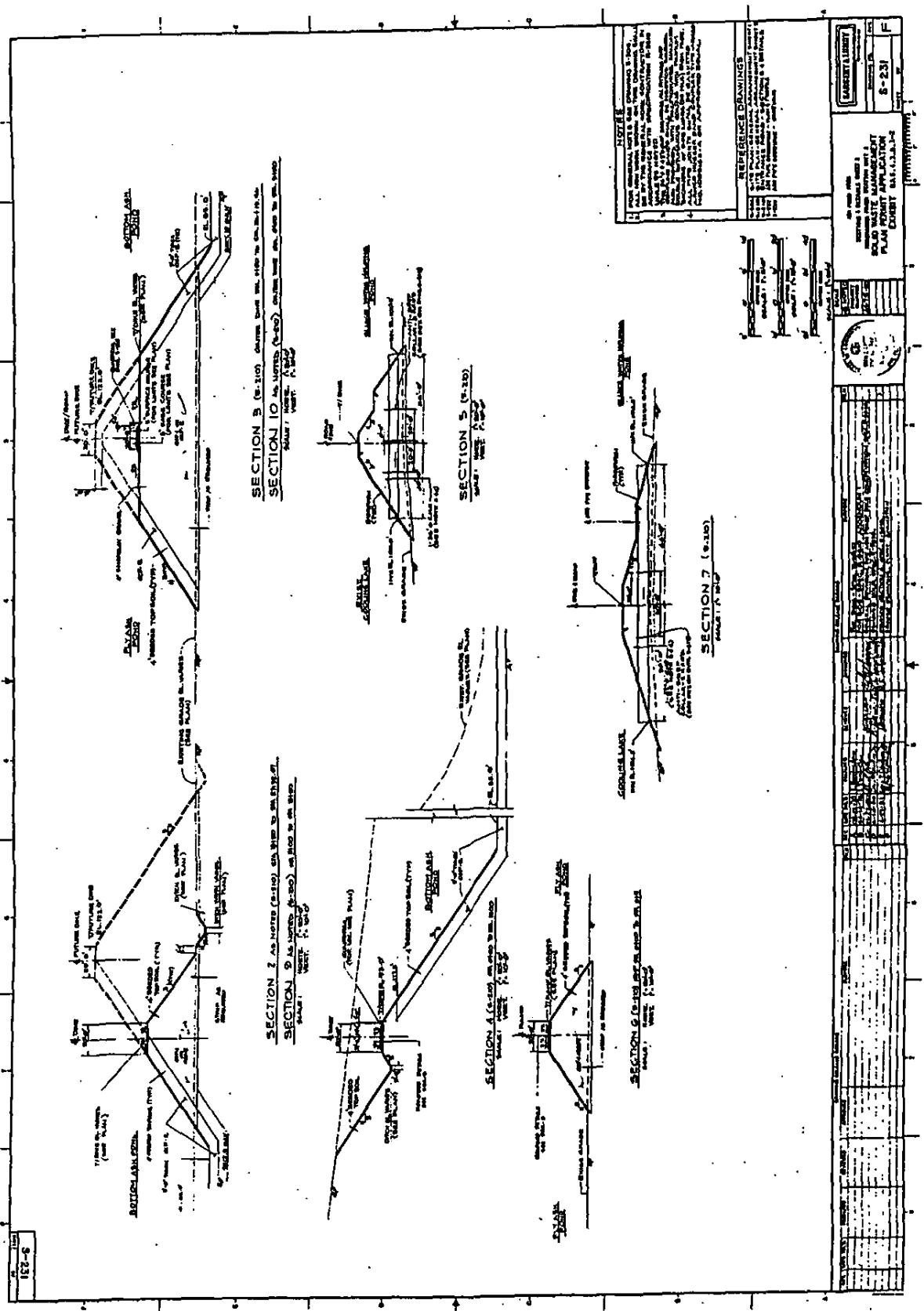
Note:

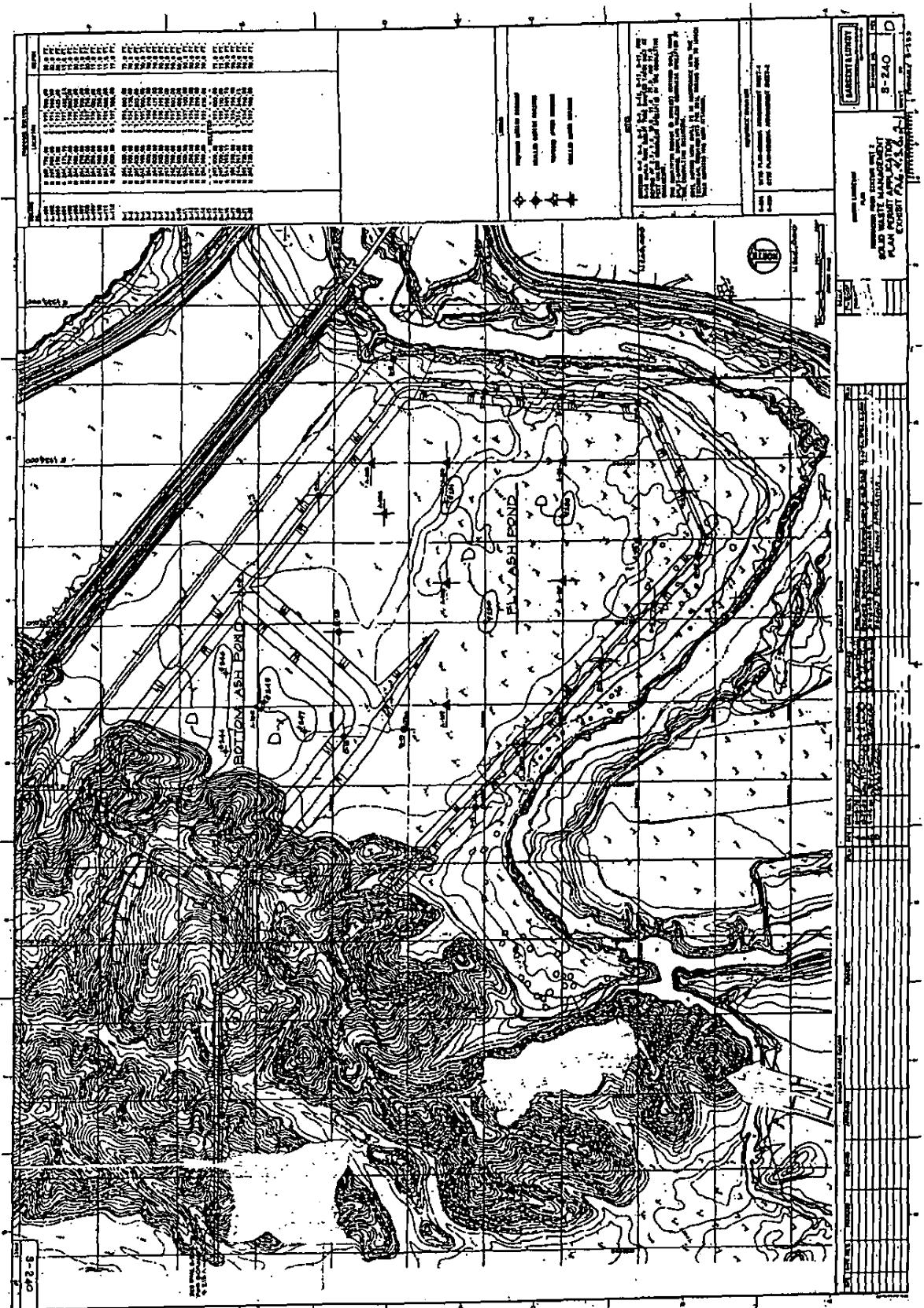
Stratigraphy between boring are inferred. Actual conditions may vary.

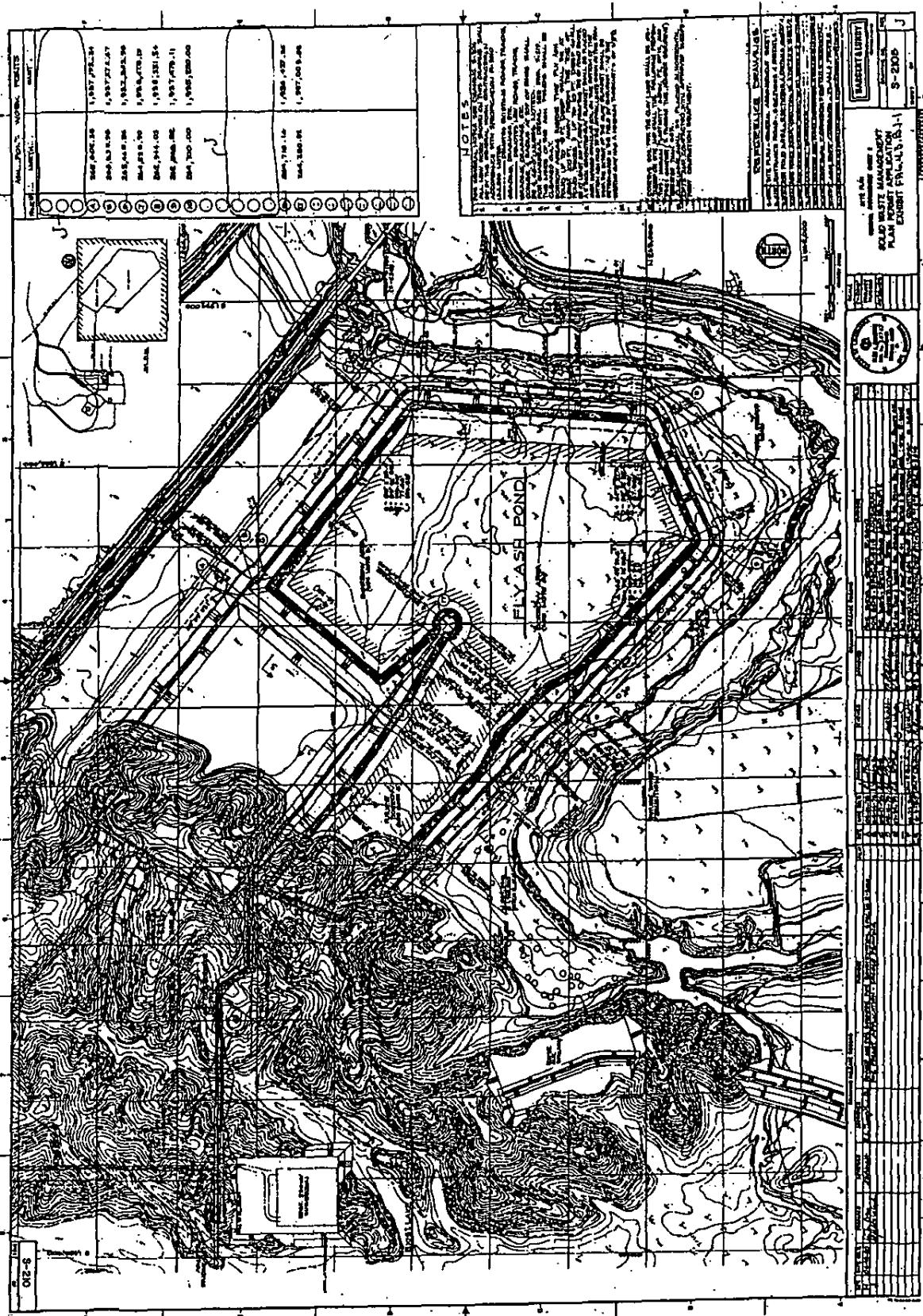


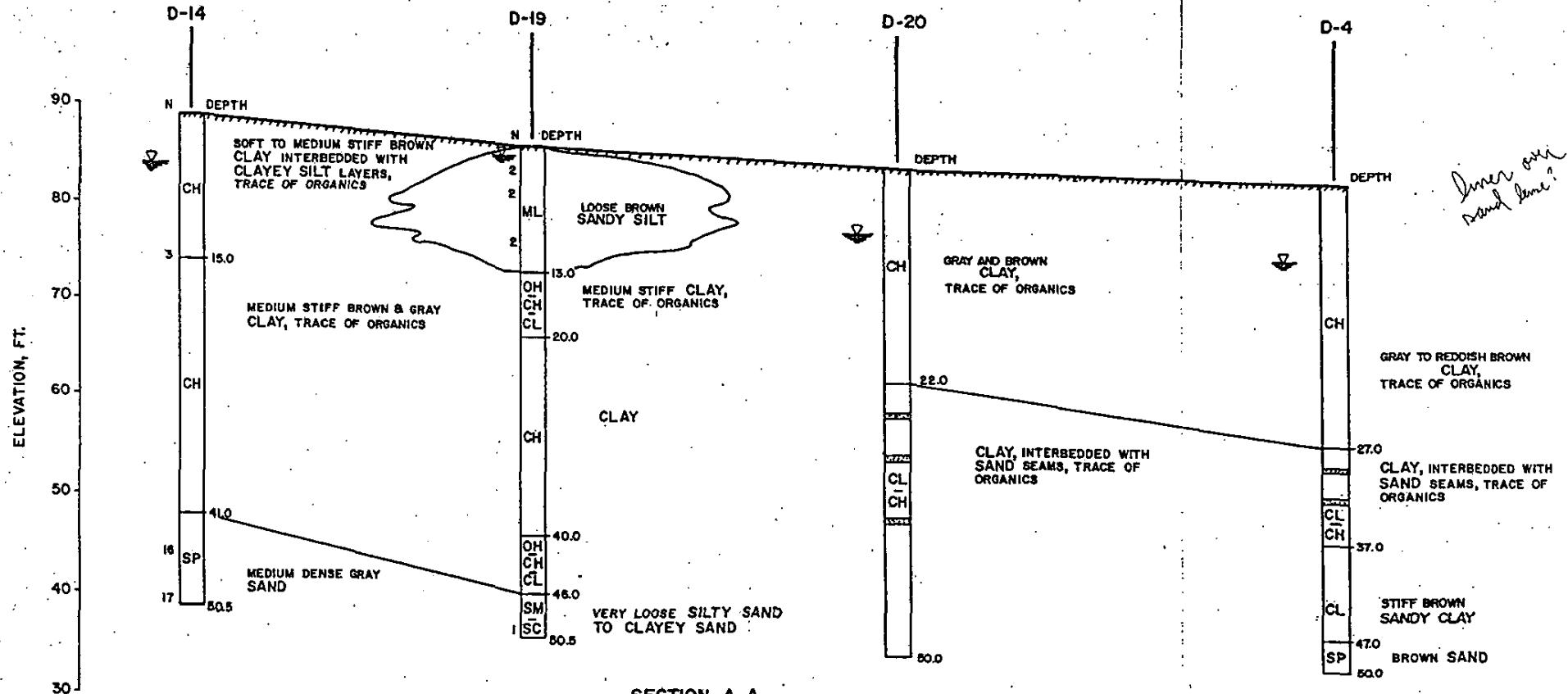


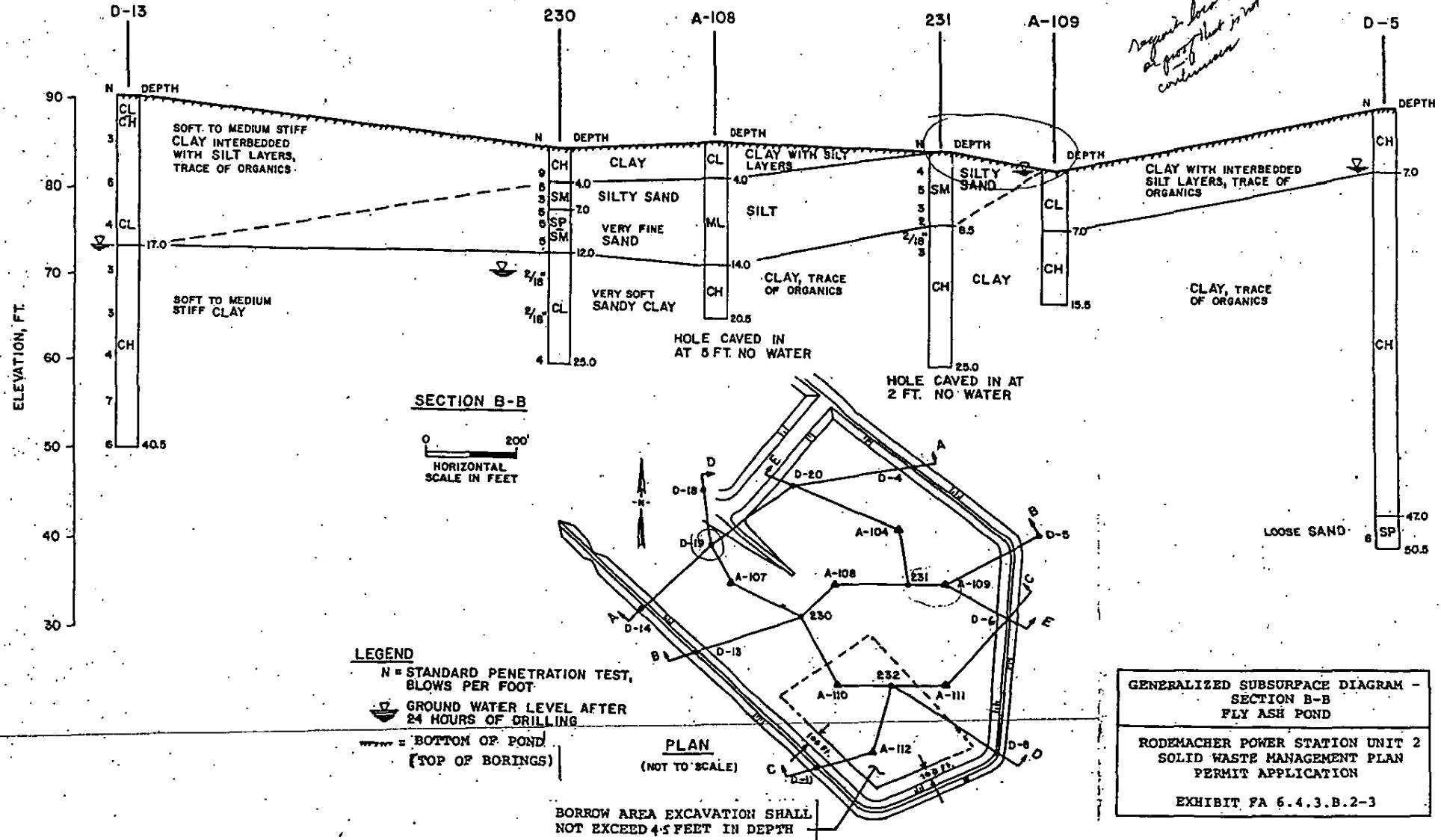


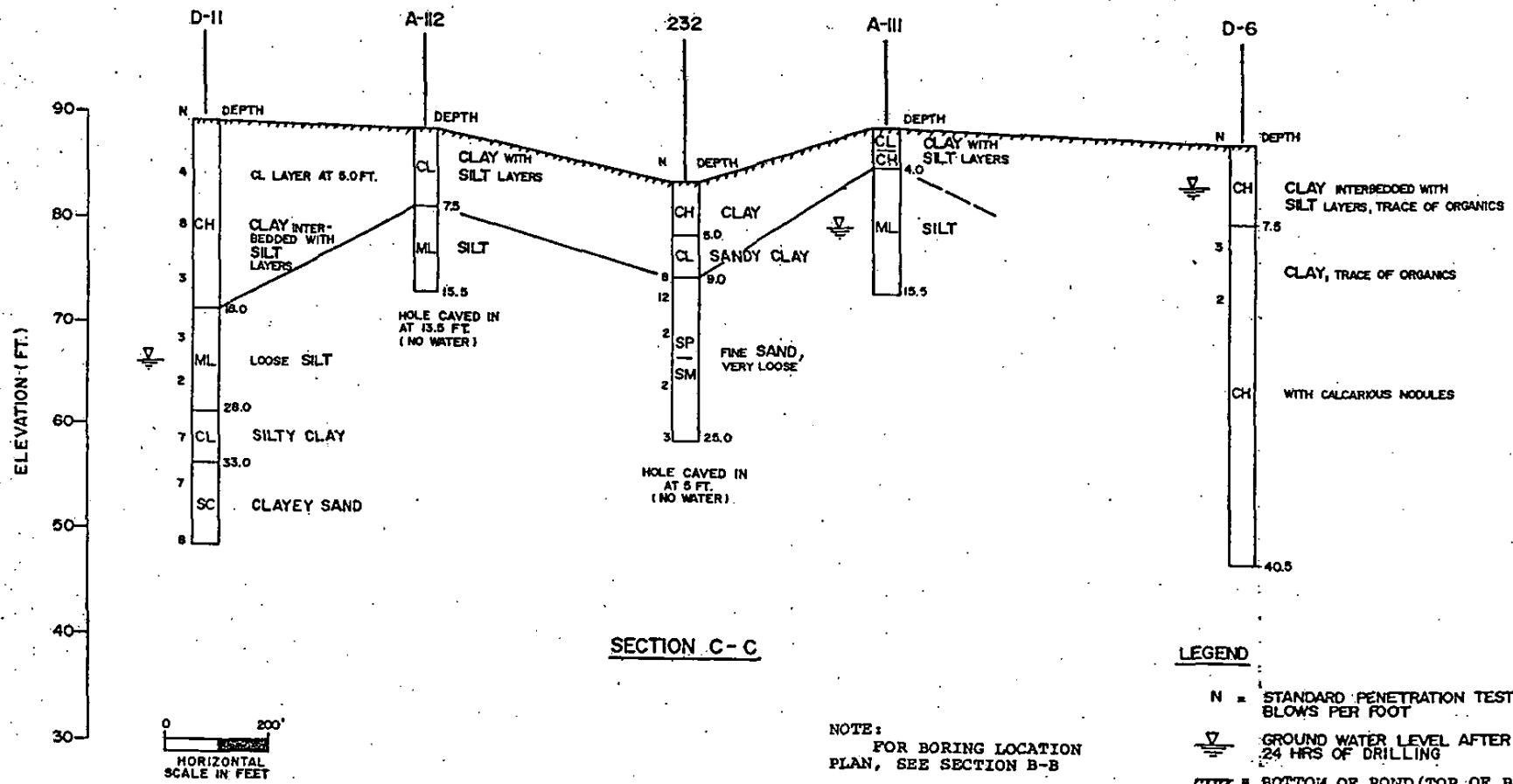


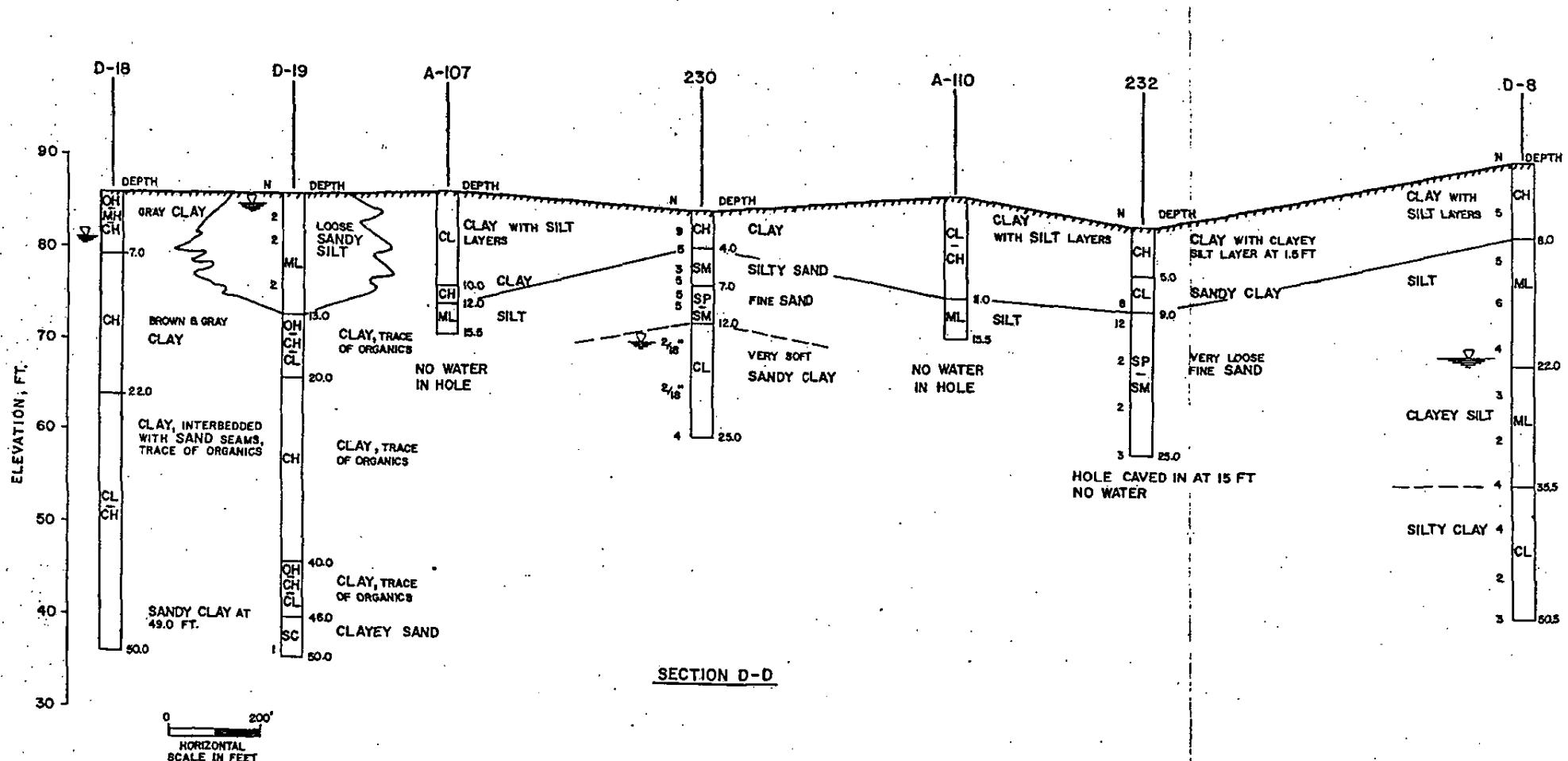




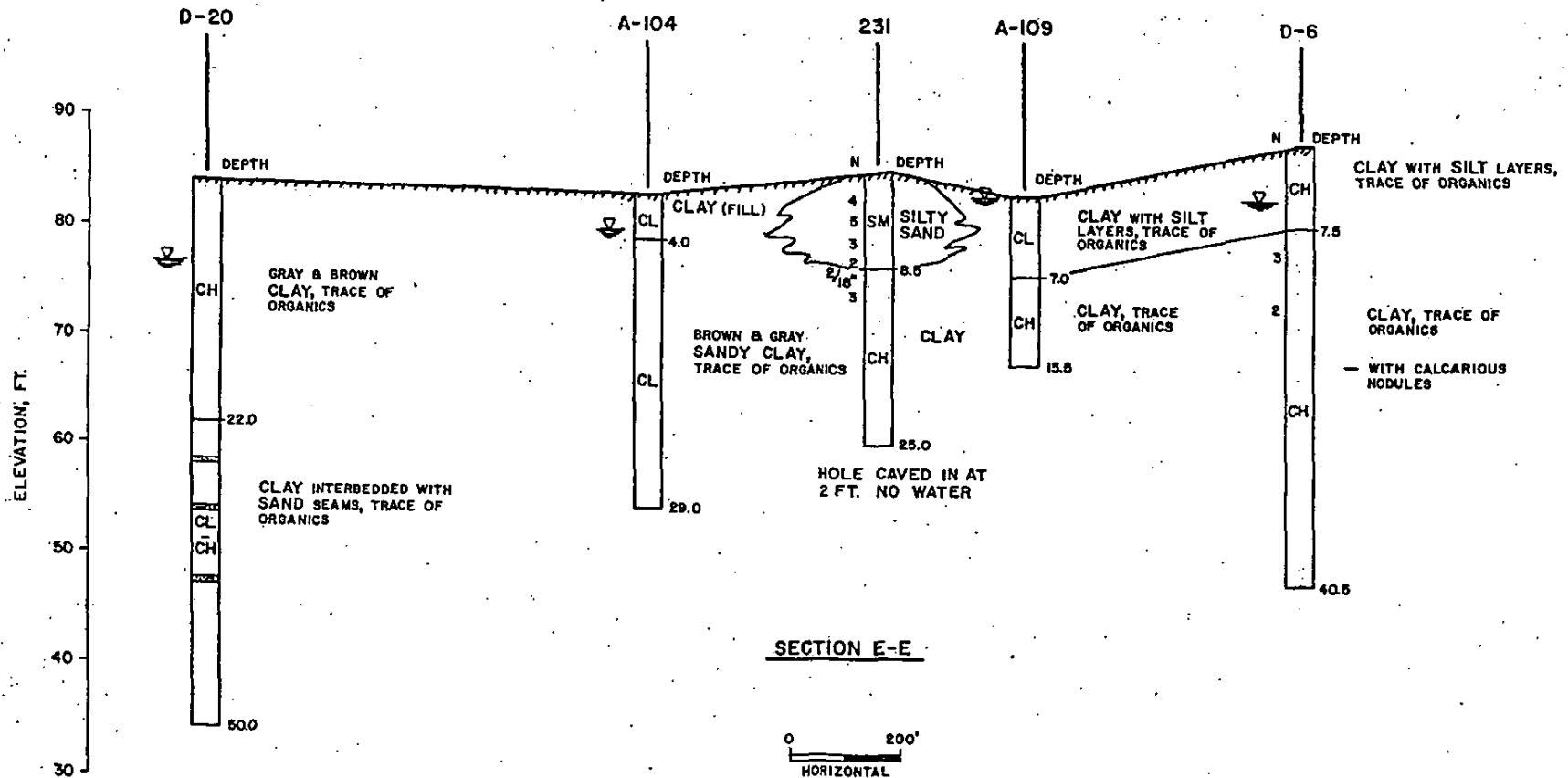








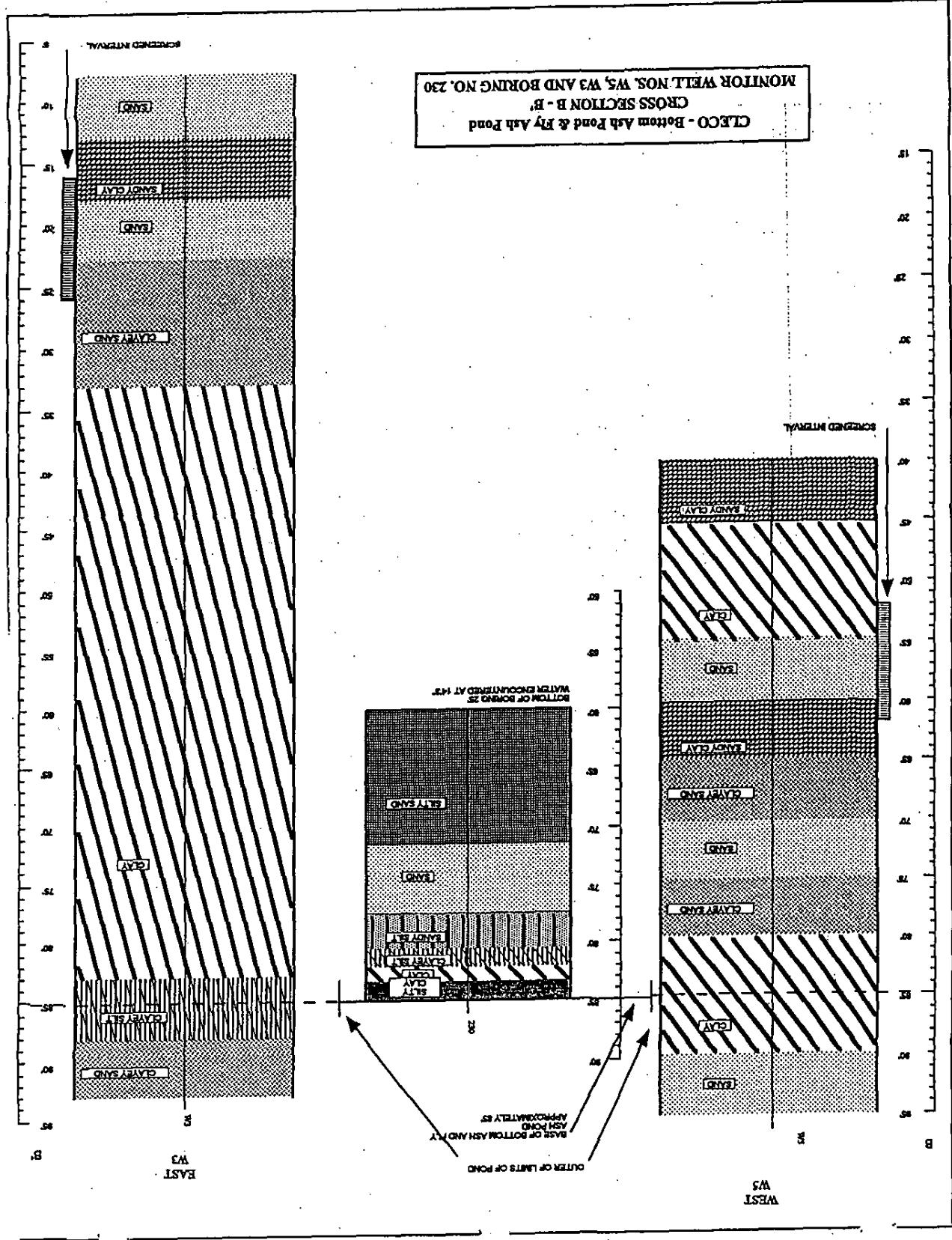
GENERALIZED SUBSURFACE DIAGRAM - SECTION D-D FLY ASH POND	
RODEMACHER POWER STATION UNIT 2 SOLID WASTE MANAGEMENT PLAN PERMIT APPLICATION	
EXHIBIT FA 6.4.3.B.2-5	



GENERALIZED SUBSURFACE DIAGRAM
SECTION E-E
FLY ASH POND

RODEMACHER POWER STATION UNIT 2
SOLID WASTE MANAGEMENT PLAN
PERMIT APPLICATION

EXHIBIT PA 6.4.3.B.2-6



APPENDIX D – DESIGN DOCUMENTS

521.F. FACILITY PLANS AND SPECIFICATIONS

1. CERTIFICATION

Appendix D contains a certification by the person who prepared the permit application in accordance with 33.VII.521.F.1

2. INFORMATION REQUIRED FOR TYPE I AND II FACILITIES

Exhibit 19 illustrates the Fly Ash Pond with original and final topographic contours. The Fly Ash Pond is located predominately in alluvial deposits of the Red River Valley. The pond was constructed by building a 20-foot wide dike around the area to be used. At elevation 103, the surface area of the pond will be 109 acres. The slope of the dikes are 3 horizontal to 1 vertical. The interior side of the dikes have a minimum 3-foot thick layer of compacted clay. There is an effective horizontal clay layer of about 10 feet.

2.a. Topography

Typical cross sections of the Fly Ash Pond are similar to those of the adjacent Bottom Ash Pond as shown in Exhibit 16. The dry fly ash trucked to the pond will be spread and sprayed for dust control as well as for compaction by dozers spreading the material. Rain water which will pond on the lowest sections of the Fly Ash Pond will be used for spraying the ash. The bottom of the Fly Ash Pond and completed dikes will have a minimum 3-foot thickness of relatively impermeable clay. This should protect the underlying ground water. furthermore, the waste characteristics of the ash are such that no harmful effects should occur to the quality of ground water.

2.b. Levee Construction

The levees were constructed using on-site materials. Since additional construction is not anticipated, calculations are not applicable.

2.c. Construction Materials

Very plastic clay soils underlay the Fly Ash Pond and excavation of these soils is underway to form a 3-foot thick layer for the diked Fly Ash Pond. The soil liner is a CH material. Cross sections showing soil profiles are illustrated in Exhibits 20 through 24.

3. INFORMATION REQUIRED FOR TYPE I, II, AND III LANDFILLS

3.a. Daily Fill and Cover

This section is not applicable to surface impoundments.

3.b. Cover Material

This section is not applicable to surface impoundments (see above).

S21.F (cont'd)

4. PREVENTION OF GROUNDWATER CONTAMINATION

4.a. Representative Cross-Sections

Cross sections of the subsurface conditions are shown in Exhibits 20 through 24.

4.b. Liner System

An engineered liner system was not included as part of the original design of the facility, however, as shown in the geotechnical information, the soils in the area of the pond are primarily clays and silty clays.

4.c. Leachate Collection and Removal System

A leachate collection and removal system was not included as part of the original design of the facility.


No leachate collection and/or treatment system is planned for the Fly Ash Pond since it is designed to contain liquids.

5. GROUNDWATER MONITORING

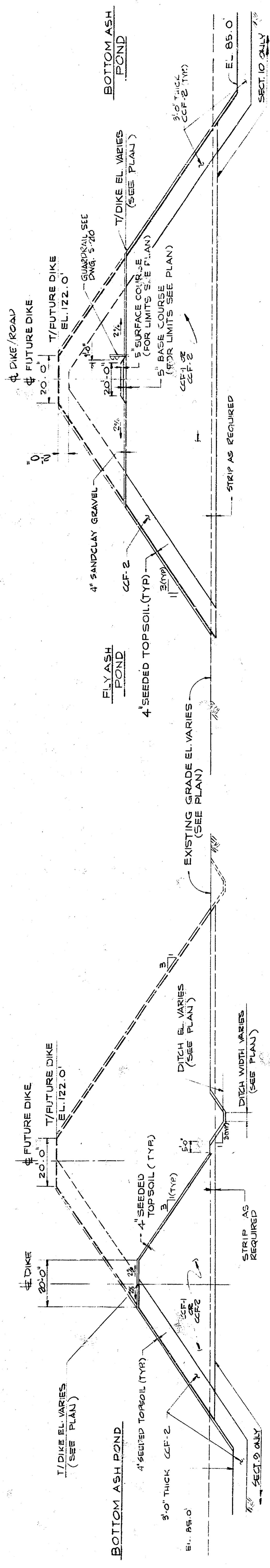
Appendix E contains a comprehensive groundwater monitoring program.

6. GAS COLLECTION AND TREATMENT SYSTEM

A gas collection and treatment system is not provided because surface impoundments do not have a potential to produce any gases.

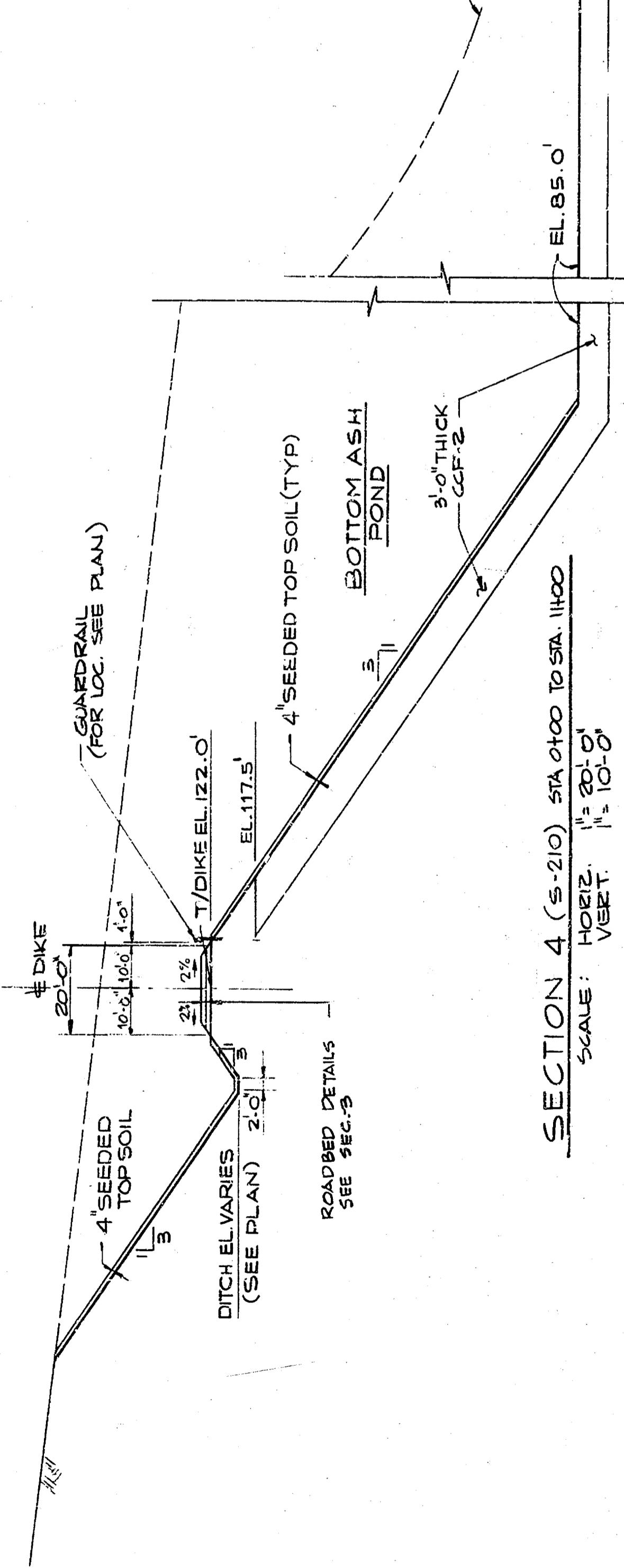


S-21



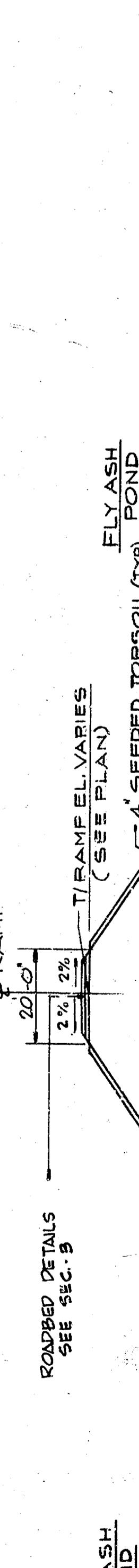
SECTION 2 AS NOTED (S-210) STA 8150 TO STA 27+92.07

SECTION 2 AS NOTED (S-210) STA 1100 TO STA 8150
SCALE : HORIZ. 1"-20'-0" VERT. 1"-10'-0"



SECTION 4 (S-210) STA 8150 TO STA 1100

SCALE : HORIZ. 1"-20'-0" VERT. 1"-10'-0"



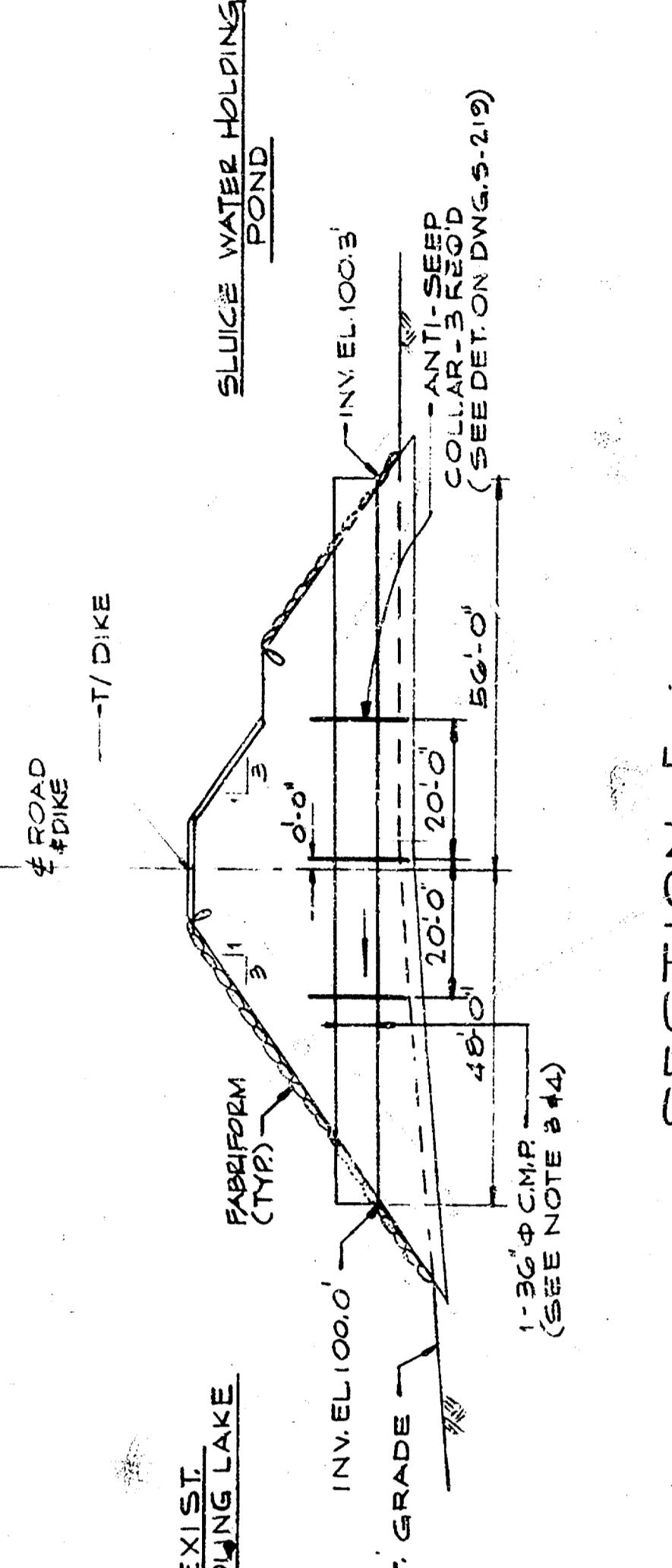
SECTION 6 (S-210) STA 8150 TO STA 8151

SCALE : HORIZ. 1"-20'-0" VERT. 1"-10'-0"



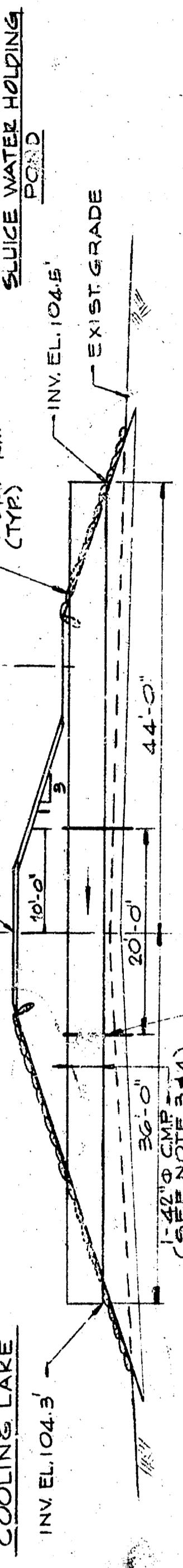
SECTION 3 (S-210) CENTER DIKE STA 9150 TO STA 24+19.46

SECTION 10 AS NOTED (S-210) CENTER DIKE STA. 9150 TO STA. 9150
SCALE : HORIZ. 1"-20'-0" VERT. 1"-10'-0"



SECTION 5 (S-210)

SCALE : HORIZ. 1"-20'-0" VERT. 1"-10'-0"



SECTION 7 (S-210)

SCALE : 1"-10'-0"

NOTES

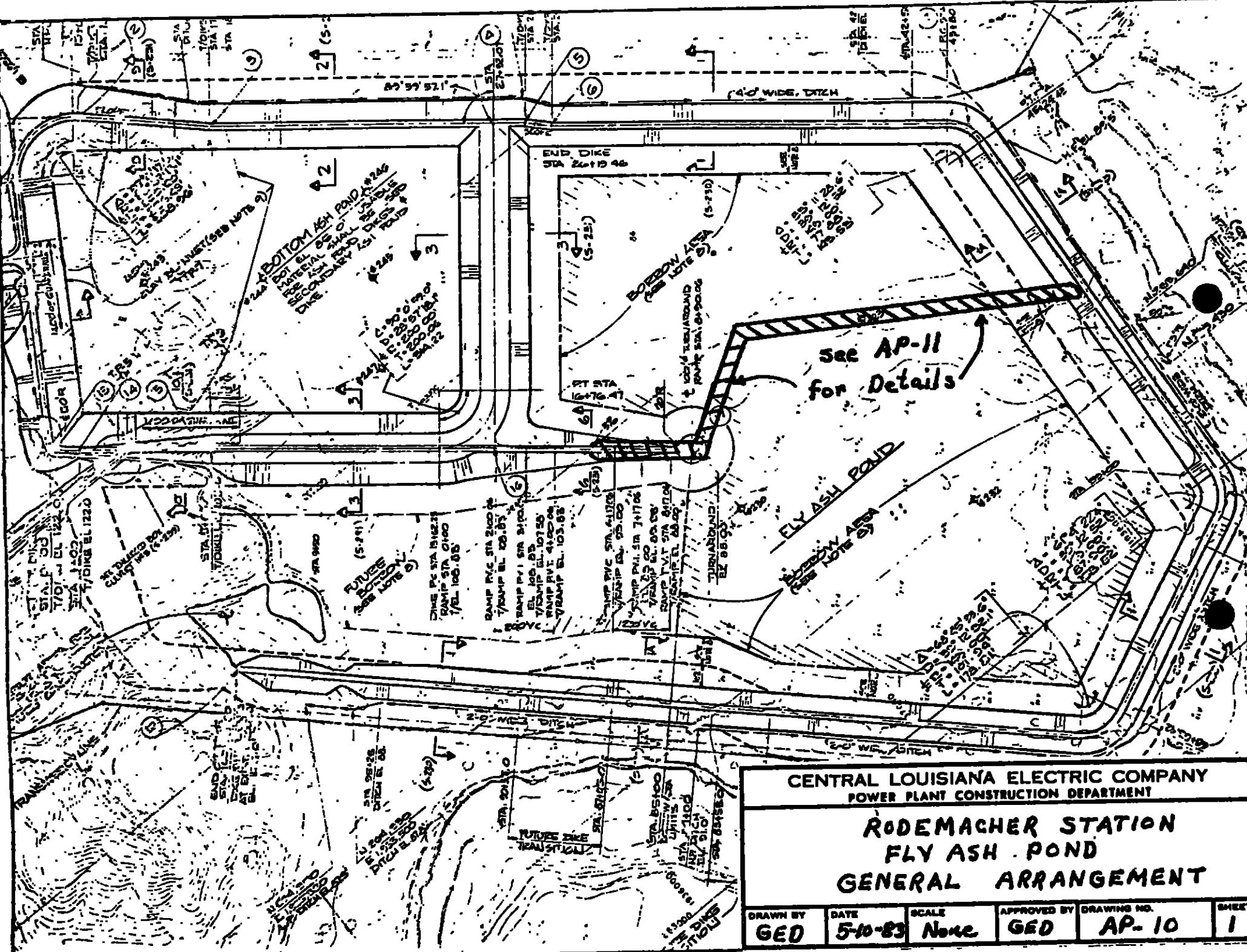
1. FOR GENERAL NOTES SEE DRAWING S-206.
2. ALL NEW WORK SHOWN ON THIS DRAWING SHALL BE BY THE GENERAL WORK CONTRACTOR IN ACCORDANCE WITH SPECIFICATION R-3810 UNLESS PROVIDED OTHERWISE.
3. GUARDRAILS AND GUARDRAIL POSTS, BARRIERS AND PROTECTIONS SHALL BE PROVIDED WITH TRANSFERABLE STANDARDS DOUBLE BUTT WELDED OR CORDED, 30 MILS EACH FACE, THICKNESSES OF ONE INCHES (SO MILS) EACH FACE.
4. IN COULD BE USED AS A FIELD CHECKERED PIPE APPROVED EQUIVALENT NO. 1005G-A OR APPROVED EQUIVALENT.

REFERENCE DRAWINGS

S-206 SITE PLAN - GENERAL ARRANGEMENT SHEET 1
S-210 SITE PLAN - GENERAL ARRANGEMENT SHEET 2
S-211 GUARDRAIL SEC. DNG. S-210
S-212 GUARDRAIL SEC. DNG. S-210
S-213 GUARDRAIL SEC. DNG. S-210
S-214 GUARDRAIL SEC. DNG. S-210
S-215 GUARDRAIL SEC. DNG. S-210
S-216 GUARDRAIL SEC. DNG. S-210
S-217 GUARDRAIL SEC. DNG. S-210
S-218 GUARDRAIL SEC. DNG. S-210
S-219 GUARDRAIL SEC. DNG. S-210
S-220 ASH PIPE CORRIDOR - SECTION 1
S-221 ASH PIPE CORRIDOR - SECTION 2
S-222 ASH PIPE CORRIDOR - SECTION 3
S-223 ASH PIPE CORRIDOR - SECTION 4

DRAWING RELEASE RECORD		PURPOSE		DRAWING RELEASE RECORD	
FILE	REV.	DATE RECD.	PREPARED	REVIEWED	APPROVED
A	04-01-78	W. L. [Signature]	FOR ELLIS - SPEC. R-3810 ADDENDUM 1		
B	04-14-78	[Signature]	GENERAL REVISION FOR ASH POND DIKE CONSTRUCTION, SPEC. R-3810		
C	04-22-78	[Signature]	GENERAL REVISION FOR ASH POND DIKE CONSTRUCTION, SPEC. R-3810		
D	05-10-78	[Signature]	GENERAL REVISION FOR ASH POND DIKE CONSTRUCTION, SPEC. R-3810		
E	4-27-81	[Signature]	GENERAL REVISION FOR ASH POND DIKE CONSTRUCTION, SPEC. R-3810		
F	02-03-82	[Signature]	REvised Drawing, Permit Application		
G	02-03-82	[Signature]	Revised Drawing, Permit Application		
H	02-03-82	[Signature]	Revised Drawing, Permit Application		

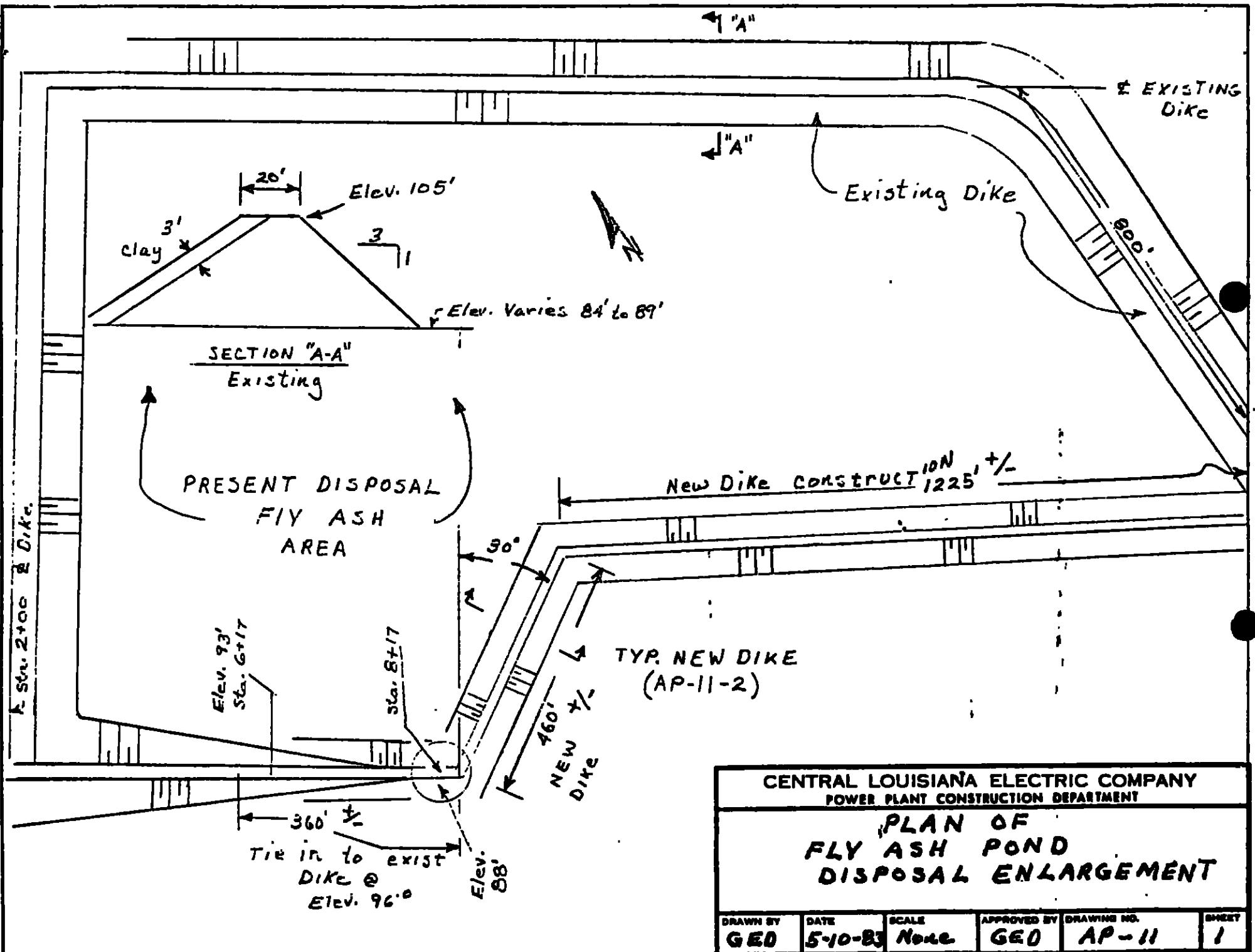
SARGENT & LUNDY	
ENGINEERS	CHICAGO
DR. NO.	REV.
S-231	
SHEET	OF



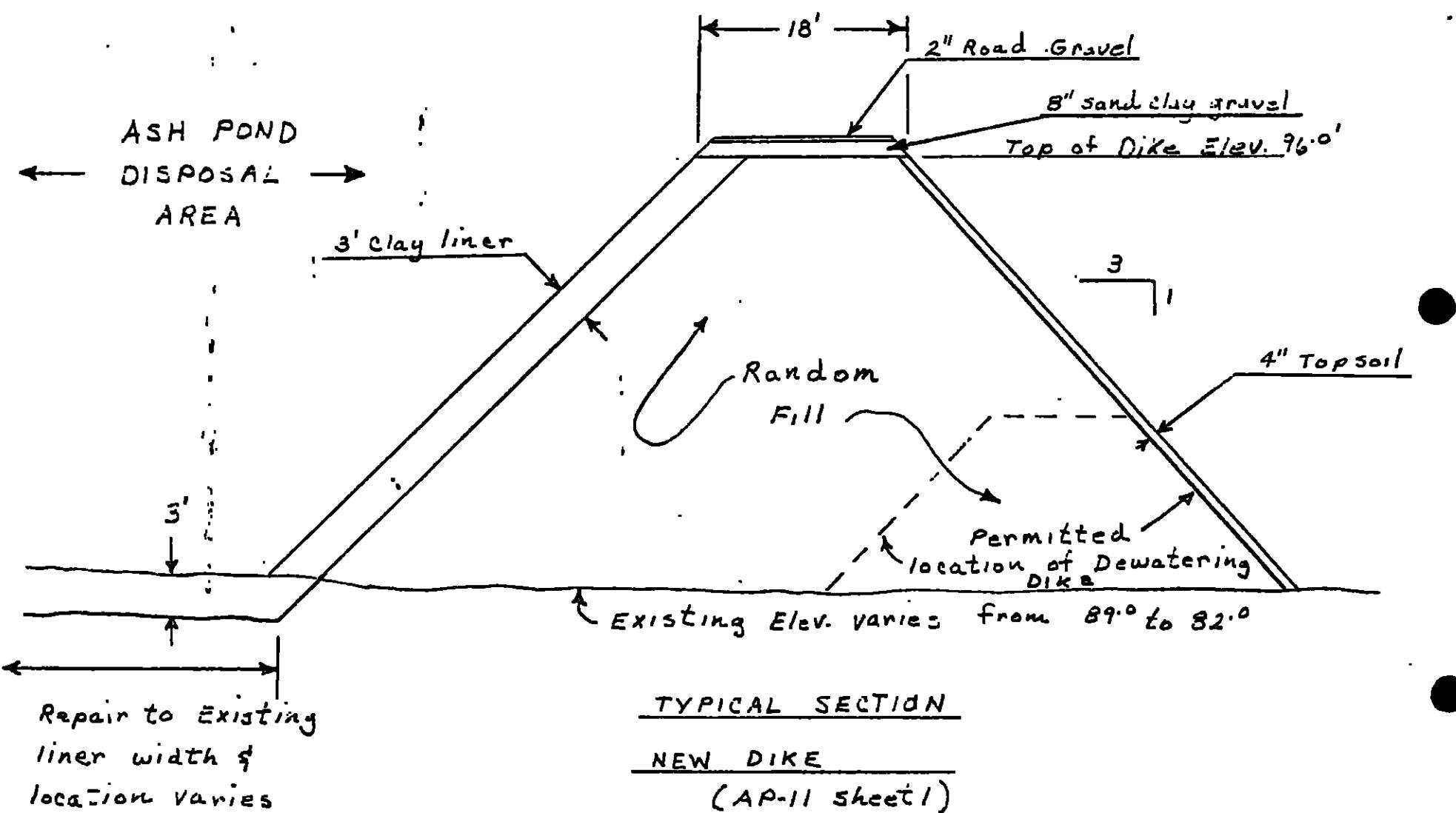
**CENTRAL LOUISIANA ELECTRIC COMPANY
POWER PLANT CONSTRUCTION DEPARTMENT**

**RODEMACHER STATION
FLY ASH POND
GENERAL ARRANGEMENT**

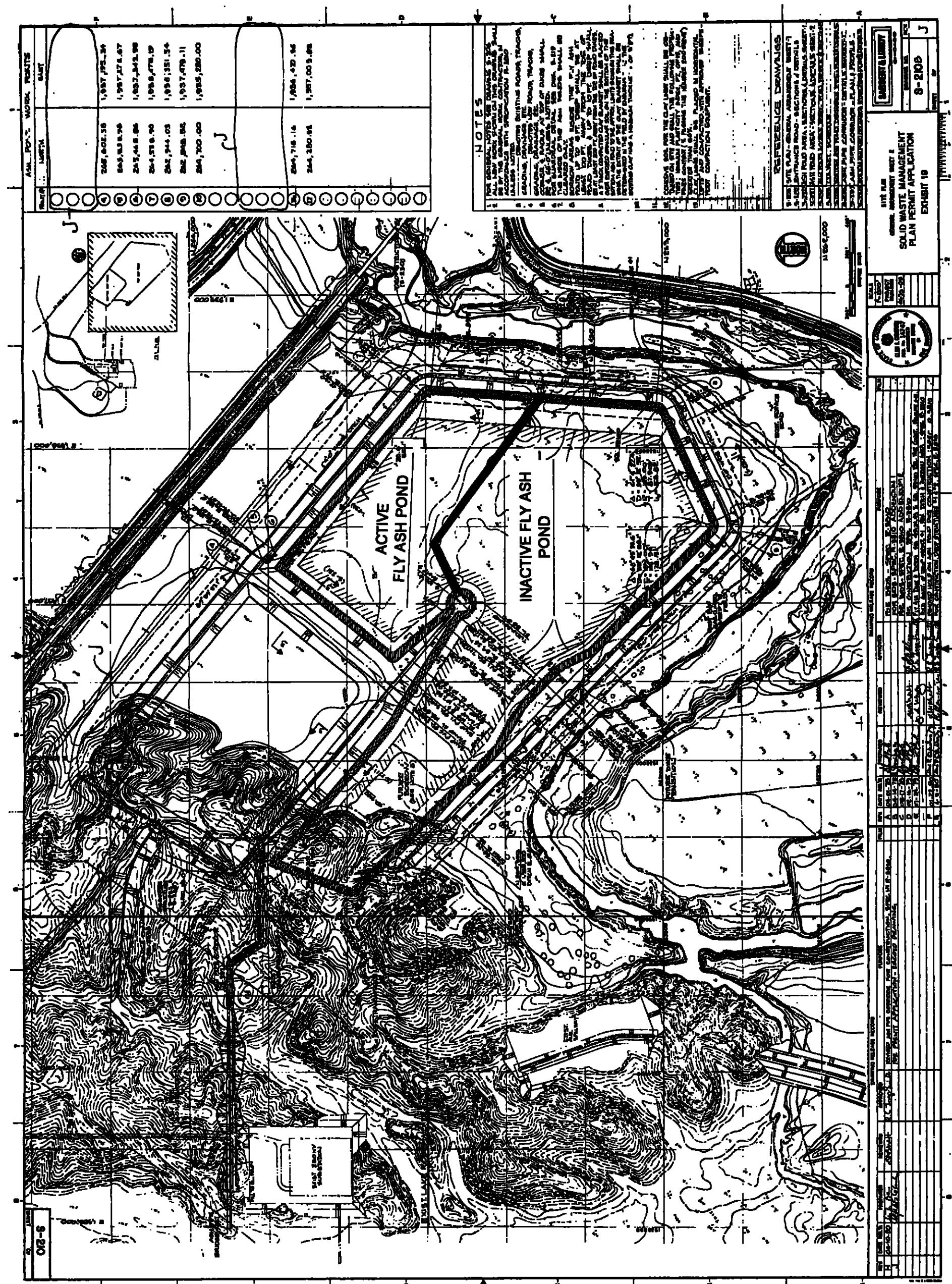
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GED	5-10-83	None	GED	AP- 10	1



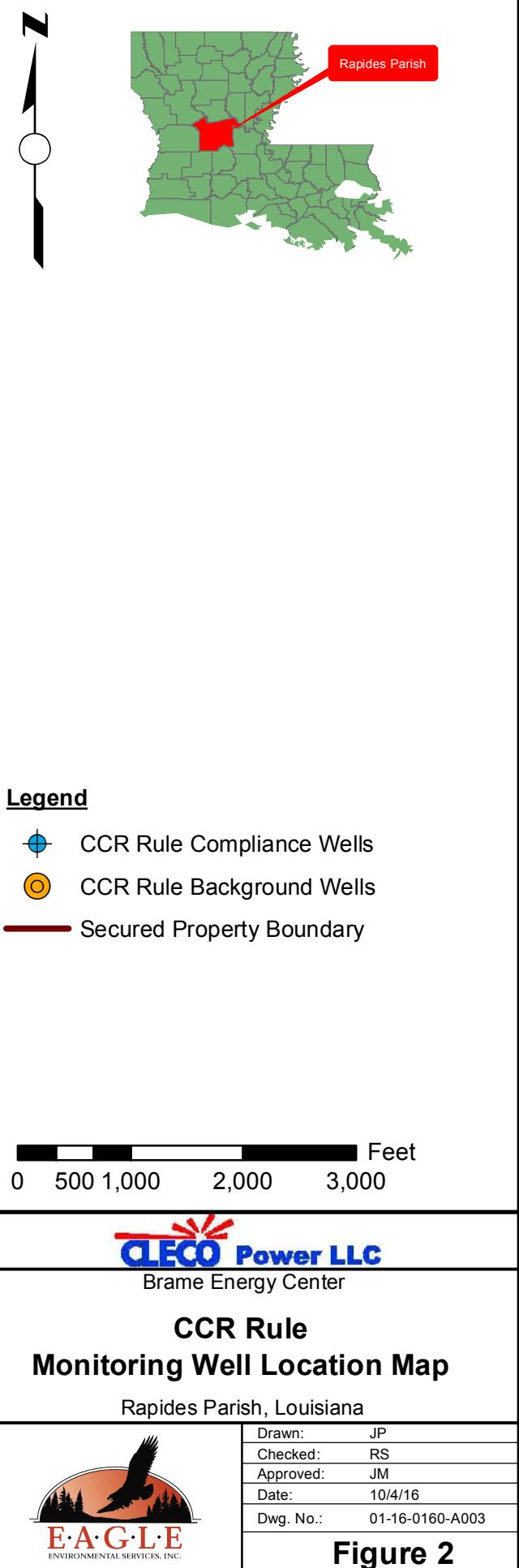
CENTRAL LOUISIANA ELECTRIC COMPANY POWER PLANT CONSTRUCTION DEPARTMENT					
PLAN OF FLY ASH POND DISPOSAL ENLARGEMENT					
DRAWN BY GEO	DATE 5-10-83	SCALE None	APPROVED BY GEO	DRAWING NO. AP-11	SHEET 1



CENTRAL LOUISIANA ELECTRIC COMPANY POWER PLANT CONSTRUCTION DEPARTMENT					
DIKE Section					
DRAWN BY GEO	DATE 5-10-83	SCALE None	APPROVED BY GEO	DRAWING NO. AP-11	SHEET 2



APPENDIX E - INSTRUMENTATION





CREATE AMAZING.

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